

SUB-APPENDIX G-1
SBEACH ANALYSIS REPORT

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**SOUTHERN PALM BEACH ISLAND COMPREHENSIVE
SHORELINE STABILIZATION PROJECT
DRAFT SBEACH ANALYSIS REPORT**

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1.0 INTRODUCTION

Under direction of the U.S. Army Corps of Engineers (USACE), CB&I Coastal Planning & Engineering, Inc. (CB&I) assisted in the development of the Southern Palm Beach Island Comprehensive Shoreline Stabilization Project Environmental Impact Statement (EIS). The initial tasks associated with the effort included public scoping and agency coordination to determine what data was necessary to develop the EIS. After review of the data and previous work, the USACE has determined that the level of storm protection needed to be analyzed using the Storm Induced Beach Change Model (SBEACH).

The Project Area for the Southern Palm Beach Island Comprehensive Shoreline Stabilization Project (the Project) includes approximately 2.07 miles of shoreline and nearshore environment. The north and south limits are Florida Department of Environmental Protection (FDEP) range monuments (R-monuments) R-129-210 (south end of Lake Worth Municipal Beach) and R-138+551 (south of the Eau Palm Beach Resort and Spa in Manalapan), respectively (Figure 2-1). The Project Area's beaches provide storm protection to residential and public infrastructure and serve as nesting areas for marine turtles. The Project Area has been designated as "critically eroded" (FDEP, 2014). The active hurricane tropical storm activity that occurred between 2004 and 2008 has resulted in a narrow, low profile beach along the majority of its shoreline. Over the past 8 years, the annual shoreline change has averaged a loss of 2.25 feet per year (CPE, 2013). Previous attempts to rebuild dunes in the Project Area have not resulted in a stable dune system or a stable beach. The Applicants' Proposed Project under evaluation in the EIS intends to address the current erosion rates by stabilizing and widening the shoreline through periodic sand nourishments.

2.0 OBJECTIVE OF SBEACH MODEL STUDY

The objectives of this beach profile storm response study using the SBEACH model are as follows:

- To verify the need for a project along all sections of the Project Area
- Determine the level of storm protection provided by the existing conditions
- Preliminarily evaluate the storm protection benefits of two proposed fill alternatives

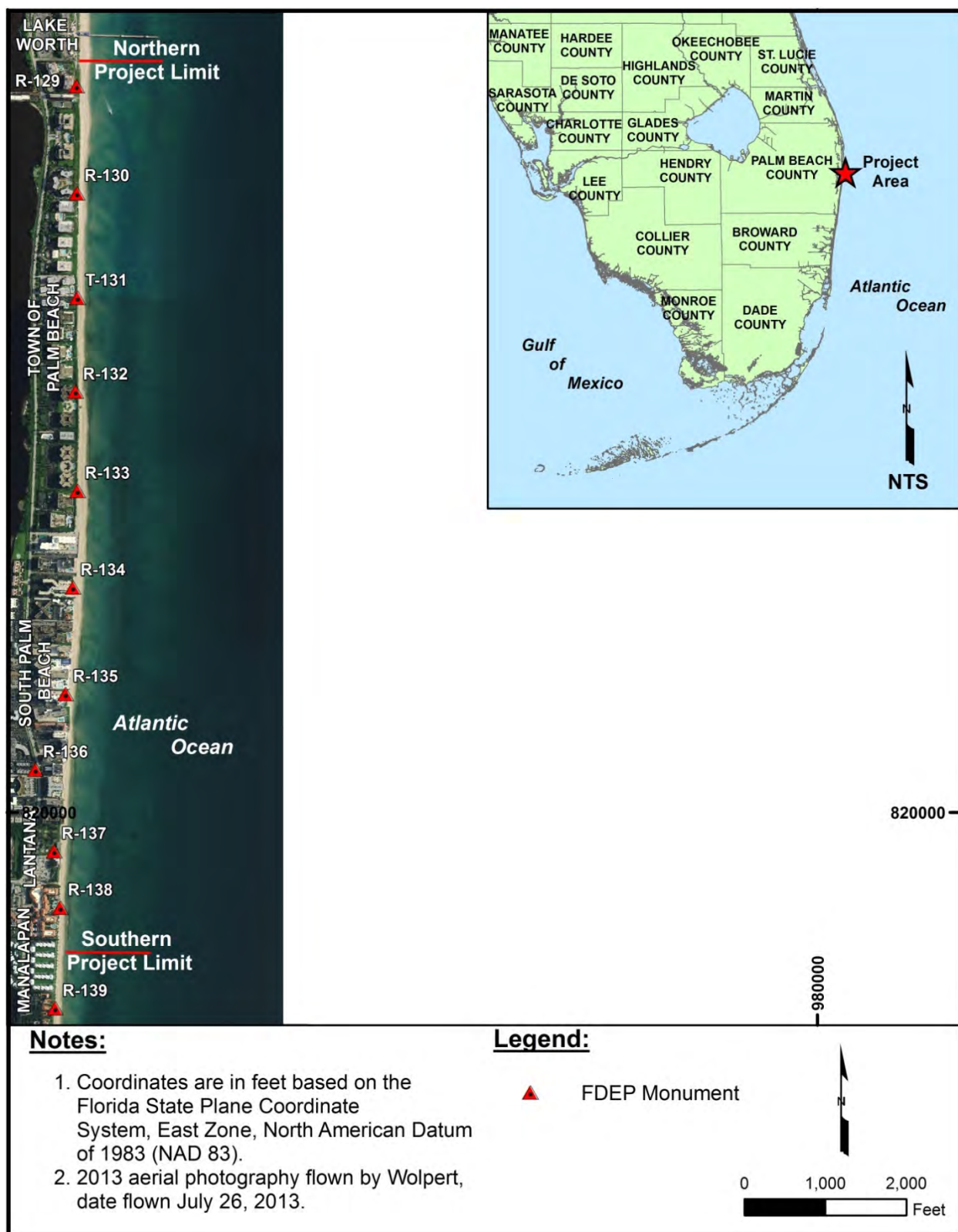


Figure 2-1. Southern Palm Beach Island Comprehensive Shoreline Stabilization Project Location.

3.0 METHODOLOGY

Cross-shore storm impact evaluations for the Project Area were conducted using the Storm Induced Beach Change Model (SBEACH) (Larson and Kraus, 1989). SBEACH is a numerical model that simulates changes to beach and dune profiles due to storm-driven erosion. Inputs to the SBEACH model include the initial profile, the time histories of the waves and water levels during each storm, and a set of model calibration parameters. Changes to the beach and dune profiles were simulated for storms return periods of 5, 15, 25, 50, and 100 years. The level of storm protection afforded by the existing beach and by the design beach fill and dune is defined by the return period of the storm event that causes a 0.5 foot vertical loss at the landward limit of the beach.

4.0 SBEACH MODEL SETUP

4.1. Model Background

SBEACH Version 4.03 (Larson et al., 2004) was used to model the cross-shore response of the design cross-section to the 5, 15, 25, 50 and 100 year storms. SBEACH is a one-dimensional model that simulates beach profile changes resulting from varying storm waves and water levels. These profile changes include the formation and movement of morphological features such as longshore bars, troughs, berms, and dunes. SBEACH evaluates storm impacts through simulated profile changes produced by cross-shore processes.

SBEACH is an empirically based numerical model, formulated using both field data and the results of large-scale physical model tests. Input data required by SBEACH includes the beach cross-section, the median sediment grain size, several calibration parameters, and the waves, wind velocities, and water surface elevations over the duration of the storm. SBEACH calculates the cross-shore variation in wave height and wave setup at discrete points along the profile from the offshore zone to the landward survey limit.

The following basic assumptions underlie the SBEACH model:

- Breaking waves and variations in water level are the major causes of sand transport and profile change.
- The influence of structures blocking longshore transport is small, and the shoreline is straight (i.e., longshore effects are negligible during the term of simulation).
- Linear wave theory is applicable everywhere along the beach profile.

4.2. Model Calibration

The model calibration was conducted using Hurricanes Frances (Category 2) and Jeanne (Category 3) because of the availability of beach profile survey data before and after the storms. These storms made landfall approximately 54 miles north of the Project Area near Hutchinson Island between August 25, 2004 and September 30, 2004.

The following wave, water level, and wind data collected during Hurricanes Frances and Jeanne was used in the SBEACH model setup:

- Waves were primarily based on the NOAA WAVEWATCH hindcast for the Western North Atlantic for the period from August 25, 2004 through September 30, 2004. Wave heights, wave periods, and wave directions at 3 hour intervals were taken from an observation point 12 miles northeast from the project site (Palm Beach Country Club, 26°45'N, 80°W) at a depth of -126.76 feet NGVD.
- Water levels were based on hourly measurements collected during the storms at the Lake Worth Pier tide gauge (NOAA Station ID LKWF1- 8722670), located immediately north of the project site.
- Wind data from NOAA Buoy LKWF1, Lake Worth was also used for calibration. Wind speed and direction was recorded hourly throughout the storm. There were two instances in the record when the station went offline for 3 to 9 hours. The

wind statistics were linearly interpolated during these periods to generate a continuous record.

The following beach profile surveys were used for the SBEACH model setup and calibration:

- Pre-storm beach profile survey conducted by Morgan & Eklund dated August 20, 2004.
- Post-storm LIDAR survey conducted by the NOAA Coastal Services Center Coastal Remote Sensing Program between November 22, 2004 and December 3, 2004.
- Post-storm beach profile survey including R-137 conducted by Palm Beach County dated October 4, 2004

The following LIDAR surveys were used to extend the SBEACH profiles landward where necessary:

- U.S. Army Corps of Engineers (USACE) Joint Airborne Lidar Bathymetry Technical Center of Expertise (JALBTCX) survey data collected by the Compact Hydrographic Airborne Rapid Total Survey (CHARTS) system along the coast of Florida from August 31 - October 3, 2009.
- Airborne Topographic Mapper LIDAR data collected in partnership with the National Oceanic and Atmospheric Administration (NOAA) Coastal Services Center along the coast of Florida in 1990.

4.3. Model Parameters

The observed changes due to Hurricanes Frances and Jeanne were used as the basis for determining the calibration settings. The initial calibration run utilized the default parameters. In the following runs, a range of values for each calibration parameter were considered until the settings with the best agreement between observed and simulated conditions were identified. Varying calibration parameters to correct the agreement at a

specific profile resulted in greater discrepancies at other profiles; therefore, the final calibration parameters were selected based on the agreement across the Project Area as a whole.

The final calibration parameters used in the production runs were as follows:

- The transport rate coefficient, which was equal to the ratio between the cross-shore transport rate and the wave energy dissipation rate was set to $K = 2.5 \times 10^{-7} \text{ m}^4/\text{N}$.
- The slope dependent coefficient, which governed the influence of the profile slope on the cross-shore transport, was set to $\varepsilon = 0.001 \text{ m}^2/\text{s}$.
- The transport rate decay coefficient, which governed the reduction in the wave height over the beach profile due to wave breaking, was set to $\lambda = 0.5$.
- The assumed depth at landward end of the surf zone was set to $D_{fs} = 1 \text{ foot}$.

In addition to the parameters above, the following assumptions were made for parameters required in the most recent version of SBEACH (4.03):

- A median grain size of 0.3 mm for the existing conditions. Samples collected in 2006 confirm the native grain size to be 0.3 mm (CPE, 2007). As an additional note, dune nourishments constructed in 2011 placed a small amount of coarser sand along the dune measuring 0.45 mm from an upland sand source (ATM, 2012).
- A grain size of 0.3 mm for the beach and dune fill. The grain size of sand in the borrow areas included in the Beach Management Agreement range from 0.25 to 0.29 mm with a compliance range of 0.25 mm to 0.6 mm for the region containing the Project Area (FDEP, 2013). Additionally, using the same grain size sediment for the various alternatives during production runs as was used in calibration allows the results to be comparable and eliminates a potential source of error.
- Average water temperature of 28.5°C (83°F) (NOAA, 2013).

- A default avalanche slope of 45°.
- The beach profiles were represented in the model with grid cell spacing of 6 feet.
- The time step used in simulations was 1 minute.
- An overwash coefficient of 0.008. The overwash coefficient is a relatively recent addition to the SBEACH model (see Larson, et al, 2004). The default value of this parameter is 0.005 for an unreinforced dune. No significant difference is noticed between simulations with varying overwash parameters for the 5, 15, 25, and 50 year storms. During the 100 year storm, the profiles are sensitive to the overwash coefficient and the magnitude of overwash increases as the coefficient increases (Figure 4-1).

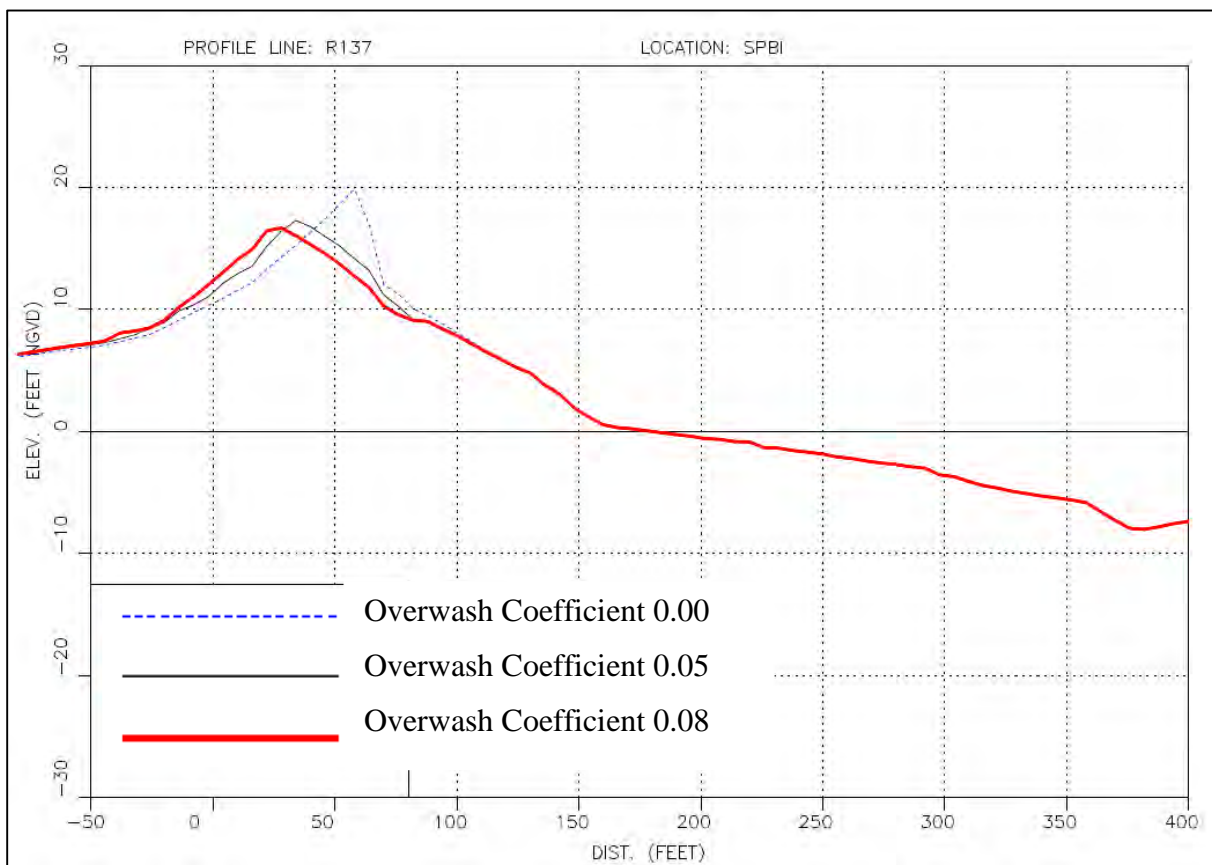


Figure 4-1. Sensitivity of overwash coefficient for R-137 profile, 100 year storm.

4.3.1. Final Calibration Results

The simulated beach profile responses with the final calibration settings agree well with the observed conditions within the Project Area. A comparison of the observed and calibrated shoreline changes, volume changes and landward limits of erosion is presented in Table 4-1. The average difference between the observed and calibrated shoreline changes was 6 feet. The average difference between the observed and calibrated volume change above mean low water (MLW) was 4 cubic yards per foot (cy/ft). The average difference between the observed and calibrated landward limit of storm recession, where at least 0.5 feet of elevation was lost, was 5 feet. On average, the calibration slightly overpredicted the erosion resulting from Hurricanes Frances and Jeanne along most profiles. This overprediction rather than underprediction of erosion is expected to positively affect the reliability of the results of the production runs. Unlike the calibration storms, the storms used in the production runs will be assumed to make landfall at the Project Area. As a result, the erosion simulated in production during an equivalent return period storm as Hurricanes Frances is expected to be more severe than what was observed in calibration.

Table 4-1. Observed vs. calibration run results for volume and shoreline changes.

| Profile ¹ | Shoreline Change (feet) | | Volume Change above Mean Low Water (-0.73 feet NGVD) (cy/ft) | | Landward Limit of Storm Erosion ² (feet from R-monument) | |
|----------------------|-------------------------|------------|--|------------|---|------------|
| | Observed | Calibrated | Observed | Calibrated | Observed | Calibrated |
| R-129 | -49 | -53 | -13 | -15 | 60 | 60 |
| R-130 | -50 | -45 | -16 | -15 | 37 | 53 |
| T-131 | -71 | -46 | -22 | -14 | 27 | 35 |
| R-132 | -16 | -21 | -8 | -13 | 33 | N/A |
| R-133 | 9 | -21 | -5 | -13 | 3 | 11 |
| R-134 | -19 | -6 | -13 | -13 | 0 | 0 |
| R-135 | -13 | -35 | -8 | -19 | 15 | N/A |
| R-136 | -20 | -32 | -7 | -17 | 11 | N/A |
| R-137 | -27 | -48 | -4 | -19 | 80 | 80 |
| Average ³ | -28 | -34 | -11 | -15 | 35 | 40 |
| Difference | | -6 | | -4 | | -5 |

¹Survey data was not available at R-138

²Survey data near the landward limit of the active profile was not available at profiles R-132, R-135 and R-136.

³Averages only include profiles where data was available.

4.4. Seawalls

Seawalls are present along 78% of the Project Area (CPE, 2007) and serve as an important component of storm protection for upland properties. The seawalls are non-homogeneous in that the quality and age of construction materials used and design criteria utilized varies by property. The information available about these seawalls is limited to the elevation of the top of the wall. Despite the limited information available, including seawalls in SBEACH is critical for simulating the beach profile response to storms.

In SBEACH, location and seawall failure criteria can be included in the model setup. The locations of the seawalls as included in the model are shown on the figures in Appendix A. The SBEACH model has three modes of failure 1) scour at the toe of the structure, 2) direct wave attack and 3) inundation. The seawall is assumed to fail and erosion occurs landward of the seawall if one or more of these criteria are met during a time step. Detailed information about the construction and stability of each seawall within the Project Area was not available. The following assumptions were made to

incorporate seawalls into the SBEACH model setup. These assumptions were intended to conservatively represent the conditions of the seawalls.

- Toe scour failure was assumed to occur when the beach profile elevation at the seawall lowered to -3 feet NGVD. Based on an average seawall height of +17 feet NGVD, the depths of the seawalls were anticipated to extend to at least -3 feet NGVD.
- The wave height at the seawall which causes failure was computed for each design storm based on the maximum water level that occurred during each storm and the overtopping failure criteria of 0.015 cubic meters per second per meter (Allsop et al, 2005; USACE, 2000).
- The water level at the seawall which was expected to cause inundation failure was assumed to be equal to the top elevation of the seawall.

Recent storms have provided evidence of the likelihood of seawall failure along the Project Area. Along the southern portion of the Project Area, many of the seawalls are exposed directly to wave action during storms (Figure 4-2 and Figure 4-3). The seawalls along the Project Area vary in age, stability and degree of exposure, leaving them more or less vulnerable to the modes of failure discussed previously. As an example, wave impacts and scouring that occurred during Hurricane Sandy led to failure and undermining of walls less than one mile south of the project site resulting in significant property damage and loss (Figure 4-4). Examining the likelihood and magnitude of toe scour using SBEACH will assist in understanding the risk of seawall failure along the Project Area and determining the overall need for the project.



Figure 4-2. Impacts of Hurricane Sandy near R-136, Town of South Palm Beach (October 26, 2012).



Figure 4-3. Impacts of Hurricane Sandy near R-137, Town of South Palm Beach (October 26, 2012).



Figure 4-4. Failure of seawall in Manalapan after Hurricane Sandy (1 mile south of the Project Area, R-143.5) (Coastal Star, 2013).

4.4.1. Seawall Replacement Cost

The estimated cost per mile to replace a seawall in Palm Beach County is approximately \$30.6 million based on the 2009 seawall construction that occurred near R-136. Therefore, the cost to replace all of the seawalls (78% of shoreline) along the 2.07-mile long Project Area after catastrophic failure would be approximately \$49.4 million.

4.5. Representative Profiles

Ten beach profiles were modeled using SBEACH (R-129 to R-138). To represent the most recent conditions, profile survey data collected between 2011 and 2012 was utilized. The datum used during the surveys were the Florida State Plane Coordinate System, North American Datum of 1983. The surveys were converted to the National Geodetic Vertical Datum using Corpscon (ver. 6.x) for consistency of datums throughout the calibration and production runs. The beach profile cross sections were extended landward for modeling purposes using the 1990 Survey for R-129 to R-137 and the 2009 Light Detection and Ranging (LIDAR) Survey for R-138.

The most recent survey of the Project Area which is being used for analysis and model setup was collected in November 2011 along the Town of Palm Beach (R-129-R-134) and in January 2012 for the County shoreline (R-135-R-138). Table 4-2 lists the most recent dune nourishments within the Project Area. The dune nourishments occurred approximately 9 months to 3 years prior to the survey dates for the Town of Palm Beach and County, respectively. Based on the information reviewed, neither of the surveys was an as-built survey. No major hurricanes have made a direct landfall within the Project Area since the nourishments; however, storms (including Hurricane Sandy) have occurred and have impacted the Project Area. The combinations of the storms' impacts and periods of calmer wave conditions contribute to the background erosion rate over the long term.

Table 4-2. Most recent dune nourishments.

| Date | Project | Project Extents | Volume (cy) | Sand Source |
|-------------------------------|--|------------------------|--------------------|--------------------|
| 2009 | South Palm Beach/Lantana Dune Restoration | R-135+460 to R-137+410 | 10,000 | Upland |
| December 2010 – February 2011 | Phipps Ocean Park Beach and Dune Restoration | Dune R-129 to R-133 | 56,000 | Upland |

4.5.1. Design Cross-Sections

The alternatives being considered for the Southern Palm Beach Island Comprehensive Shoreline Stabilization Project at the time of the modeling study include the following:

- 1) No Action Alternative (Status Quo), which includes periodic dune nourishment of the dry beach.
- 2) The Applicants' Preferred Alternative (Proposed Action): Beach and Dune Fill with Shoreline Protection Structures
- 3) The Applicants' Preferred Project without Shoreline Protection Structures

4) The Town of Palm Beach Preferred Project and County Increased Sand Volume Project without Shoreline Protection Structures

5) The Town of Palm Beach Increased Sand Volume Project and County Preferred Project

6) The Town of Palm Beach Increased Sand Volume Project and County Increased Sand Volume without Shoreline Protection Structures Project

SBEACH modeling was conducted for Alternatives 1, 3 and 6 (Table 4-3). Alternative 2 was not modeled since the fill design is the same as Alternative 3. SBEACH is a cross-shore transport model and does not include the option of including groins (shoreline protection structures) as present in Alternative 2. Additional details describing the alternatives are provided below.

- Alternative 1 utilized the 2011/2012 surveys without modification to represent the existing conditions or No Action (Status Quo) Alternative. No Action Alternative includes periodic dune nourishments with fill volume placements of approximately 11 cubic yards per foot from R-129 to R-133 and 5 cubic yards per foot from R-135-460 to R-137+410.
- Alternative 2 was not simulated in SBEACH. The results from Alternative 3 are applicable to Alternative 2. Alternative 2 has 7 low-profile pile and panel groins as part of the design. SBEACH cannot consider the effects of groins in simulating the cross-shore storm response of beach profiles.
- Alternative 3 utilized the Applicants' Preferred fill design which consisted of dune fill only from R-129-210 to R-129+150, dune and beach fill from R-129+150 to T-131, dune fill only from T-131 to R-134+135 (Town of Palm Beach southern limit), and beach fill from R-134+135 to R-138+551 (Towns of South Palm Beach, Lantana and Manalapan). This alternative was originally designed to require approximately 150,000 cubic yards of fill for the entire project based on 2009 surveyed profiles along the Town of Palm Beach and 2008 surveyed profiles for the remainder of the Project Area. The design was updated based on the

available winter 2011/2012 profiles for use in the SBEACH model setup requiring approximately 117,300 cubic yards of fill. The seaward crests of the dune and berm from the original design remained at the same range and elevation in the updated design with two exceptions 1) if the 2011/2012 dune was located seaward of the original design, no fill was added to the dune and 2) no fill was placed landward of the edge of vegetation as shown in the 2011/2012 aerials.

- Alternative 4 utilized the same Applicants' Preferred design as Alternative 2 for the Town of Palm Beach portion of the project area (R-129-210 to R-134+135) and a larger design along the County portion (R-134+135 to R-138+551). The fill volume within the County's portion of the project was increased to 172,100 cubic yards from 63,500 cubic yards. The total fill volume required to construct the template was estimated at approximately 225,900 cubic yards based on the winter 2011/2012 beach profiles.
- Alternative 5 utilized a modified design for the Town of Palm Beach portion (R-129-210 to R-134+135) and the Applicants' Preferred design along the County portion of the project area (R-134+135 to R-138+551). The modified design consisted of placing additional fill on the dry beach (R-129-210 to R-134+135) where feasible thereby increasing the fill volume to 100,900 cubic yards from 53,900 cubic yards. The total fill volume required to construct the template was estimated at approximately 164,400 cubic yards based on the winter 2011/2012 beach profiles.
- Alternative 6 utilized the same larger design used in Alternative 5 along the Town's portion of the project (R-129-210 to R-134+135; 100,900 cubic yards) and the same larger design used in Alternative 4 along the County portion (R-134+135 to R-138+551; 172,100 cubic yards). The total fill volume required to construct the template was estimated at approximately 273,000 cubic yards based on the winter 2011/2012 beach profiles.

Table 4-3. Cross-sections simulated in the SBEACH model.

| Profile | Dune/Berm Width (feet) | Dune/Berm Slope | Seawall Included |
|--------------------|------------------------|-----------------|------------------|
| Alternatives 2 & 3 | | | |
| R-129 | No fill added | | No |
| R-130 | 17 | 1V:10H | Yes |
| T-131 | 18 | 1V:3H | No |
| R-132 | 10 | 1V:3H | Yes |
| R-133 | No Fill added | | No |
| R-134 | 35.2 | 1V:3H | Yes |
| R-135 | 22.2 | 1V:10H | No |
| R-136 | 75.6 | 1V:10H | Yes |
| R-137 | 52.7 | 1V:10H | Yes |
| R-138 | 18.5 | 1V:10H | Yes |
| Alternative 4 | | | |
| R-129 | No fill added | | No |
| R-130 | 17 | 1V:10H | Yes |
| T-131 | 18 | 1V:3H | No |
| R-132 | 10 | 1V:3H | Yes |
| R-133 | No Fill added | | No |
| R-134 | 35.2 | 1V:3H | Yes |
| R-135 | 66.2 | 1V:10H | No |
| R-136 | 130.2 | 1V:10H | Yes |
| R-137 | 98.4 | 1V:10H | Yes |
| R-138 | 58.5 | 1V:10H | Yes |

Table 4-3 (cont.). Cross-sections simulated in the SBEACH model.

| Profile | Dune/Berm Width (feet) | Dune/Berm Slope | Seawall Included |
|---------------|------------------------|-----------------|------------------|
| Alternative 5 | | | |
| R-129 | 65.2 | 1V:5H | No |
| R-130 | 17 | 1V:10H | Yes |
| T-131 | 18 | 1V:3H | No |
| R-132 | 0 | 1V:3H | Yes |
| R-133 | 46.9 | 1V:3H | No |
| R-134 | 72.8 | 1V:3H | Yes |
| R-135 | 22.2 | 1V:10H | No |
| R-136 | 75.6 | 1V:10H | Yes |
| R-137 | 52.7 | 1V:10H | Yes |
| R-138 | 18.5 | 1V:10H | Yes |
| Alternative 6 | | | |

| | | | |
|-------|-------|--------|-----|
| R-129 | 65.2 | 1V:5H | No |
| R-130 | 17 | 1V:10H | Yes |
| T-131 | 18 | 1V:3H | No |
| R-132 | 10 | 1V:3H | Yes |
| R-133 | 46.9 | 1V:3H | No |
| R-134 | 72.8 | 1V:3H | Yes |
| R-135 | 66.2 | 1V:10H | No |
| R-136 | 130.2 | 1V:10H | Yes |
| R-137 | 98.4 | 1V:10H | Yes |
| R-138 | 58.5 | 1V:10H | Yes |

4.6. Storm Data

Five specific return interval storm events were used in the SBEACH cross-shore analyses, 5 year, 15 year, 25 year, 50 year and 100 year. Wind, water level and wave data from Hurricane Frances observed during the time period from August 25, 2004 to September 9, 2004 was used as the basis for the design of the return interval storms. The Hurricane Frances data was scaled accordingly to match the maximum values listed in Table 4-4 for each storm. Maximum wave heights, wave periods, and water levels during each storm appear in Table 4-4. Plots of the wave height, wave period, and water level versus time appear in Appendix B.

Table 4-4. Design Storm Summary.

| Return Period (years) | Maximum Values | | | |
|--------------------------|--|----------------------------------|---|----------------------------------|
| | Offshore Significant Wave Height ¹ (feet) | Peak Wave Period (seconds) | Water Level ² (feet NGVD) | Wind Speed ³ (mph) |
| 5 | 20.8 | 9.7 | 3.7 | 69 |
| 15 | 26.4 | 11.0 | 5.0 | 85 |
| 25 | 29.1 | 11.5 | 5.5 | 93 |
| 50 | 32.6 | 12.2 | 6.3 | 103 |
| 100 | 36.2 | 12.8 | 7.0 | 111 |

NOTES: 1. Wave heights are given at a depth of 356 meters (USACE, 2012).

2. Values in italics are interpolated or extrapolated from FEMA (1982). These values do not include wave setup as it is calculated and included by SBEACH during the simulations.

3. Values in italics are interpolated or extrapolated from USACE (1985).

FEMA return period water level accounts for tidal effects. FEMA used a numerical hydrodynamic model of the region to simulate the coastal surge generated by different return period storms. The astronomical tide for the region was statistically combined

with the computed storm tide to yield recurrence intervals of total water level shown in the published water levels (FEMA, 1982).

5.0 MODEL RESULTS

5.1. General

SBEACH model results appear in Appendices C and D and include the post-storm profiles for all design storms in Table 4-4.

5.2. Existing Conditions (2011/2012 Beach Profiles) / No Action Status Quo Scenario

The existing conditions along the Project Area shoreline consist of eroded dunes, exposed seawalls and steep gradient berms. Along the Town of Palm Beach, there is a continuous dune feature and line of vegetation separating the beach from the residential infrastructure. There are several buried seawalls along this section of shoreline (R-129-210 to R-134+135). Along the Towns of South Palm Beach, Lantana and Manalapan, there is no dune feature and the majority of the beach profiles consist of partially exposed seawalls.

The degree of erosion during a storm will vary spatially due to the characteristics of the beach profiles (Table 5-1; Appendix C). Profiles T-131 through R-134 will experience the most erosion. Profile T-131 is not protected by seawalls. This profile also has the steepest existing beach face which leads to higher breaking waves in the surf zone and increases the potential for runoff and erosion. Profiles R-132, R-134 and R-137 will experience similar erosion. The exposed seawalls present on these profiles leads to scouring and volume loss at the base of the wall. The other profiles have similar but slightly lower erosion rates. The average volume change above MLW during a 5, 10, 25, 50 and 100-year return interval storm along the Project Area was -6.0 cy/ft, -7.3 cy/ft, -7.7 cy/ft, -8.4 cy/ft and -9.1 cy/ft, respectively (Table 5-2).

Under existing conditions, the seawalls and revetments at monuments R-130, R-132, R-136 are exposed. Scouring at the toe of the seawalls occurs at these locations in all of

the simulated return interval storms (Appendix C). Scouring increases incrementally with magnitude of storm. No seawall failures were observed during the simulations.

The landward limit of erosion was quantified to determine the potential impacts to infrastructure and property landward of the Project Area (Table 5-3). The landward limit of erosion was defined as the landward position where at least 0.5 feet of elevation was lost as a result of the storm. The values in Table 5-3 are referenced to the FDEP R-monuments since the monuments are at a fixed location. As the profiles erode landward towards the R-monuments, the values in the table decrease until they retreat landward of the monument and then the values are negative. The table values in red signify that recession landward of the improved or maintained property has occurred. Maintained property refers to landscaped areas or paved/ gravel areas. While a seawall is operational, the landward limit of recession is the same for different return interval storms because the seawalls prevent further landward recession as shown in the table at R-130 for the 15, 25, 50, 100-year storms. In general, profiles without seawalls, T-131 and R-135 are certainly at risk of damage during the occurrence of a 25-year return interval storm or stronger storm. Damage is possible adjacent to profile R-133 as a result of a 50-year return interval or stronger storm. The critical storm return interval for damage to property to occur is between a 15-year and 25-year storm.

Table 5-1. SBEACH shoreline and volume changes under existing conditions (2011/2012) and a 15 year storm.

| Profile | MLW Change (feet) | Volume Change above MLW (cy/foot) |
|----------------|--------------------------|--|
| R-129 | -17 | -5.6 |
| R-130 | 0 | -6.4 |
| T-131 | 2 | -8.1 |
| R-132 | 4 | -8.1 |
| R-133 | -23 | -9.2 |
| R-134 | -22 | -8.7 |
| R-135 | -17 | -7.1 |
| R-136 | -22 | -5.8 |
| R-137 | -24 | -7.4 |
| R-138 | -40 | -6.1 |

NOTE: Mean Low Water (MLW) = -0.73' NGVD.

Table 5-2. SBEACH shoreline retreat and erosion, existing conditions (2011/2012).

| | 5 Year Storm | 15 Year Storm | 25 Year Storm | 50 Year Storm | 100 Year Storm |
|----------------|--|--|--|--|--|
| Profile | Volume Change above MLW (cy/ft) | Volume Change above MLW (cy/ft) | Volume Change above MLW (cy/ft) | Volume Change above MLW (cy/ft) | Volume Change above MLW (cy/ft) |
| R-129 | -4.6 | -5.6 | -6.0 | -6.6 | -7.1 |
| R-130 | -5 | -6.4 | -6.8 | -7.4 | -8 |
| T-131 | -6.5 | -8.1 | -8.8 | -9.9 | -10.7 |
| R-132 | -6.3 | -8.1 | -8.8 | -9.8 | -10.6 |
| R-133 | -7.6 | -9.2 | -9.9 | -10.7 | -11.4 |
| R-134 | -7.4 | -8.7 | -9.3 | -10 | -10.5 |
| R-135 | -5.9 | -7.1 | -7.6 | -8.2 | -8.7 |
| R-136 | -5.0 | -5.8 | -6.0 | -6.5 | -6.7 |
| R-137 | -6.5 | -7.4 | -7.5 | -8 | -10.3 |
| R-138 | -5.3 | -6.1 | -6.3 | -6.7 | -7.2 |

Table 5-3. SBEACH landward limit of storm erosion.

| FDEP R-Monument ¹ | Simulation ID | Landward Limit of Storm Erosion ² (feet from seaward edge of maintained property) Given Return Period in Years: | | | | |
|------------------------------|------------------------------|--|-----|-----|------|------|
| | | 5 | 15 | 25 | 50 | 100 |
| R-129 | Existing Conditions | 97 | 66 | 52 | 33 | 31 |
| | Alternative 3 | 97 | 66 | 52 | 36 | 31 |
| | Alternative 6 | 111 | 93 | 85 | 50 | 31 |
| R-130 | Existing Conditions | 55 | 37 | 32 | -2 | -7 |
| | Seawall Failure ³ | 55 | 37 | 32 | -14 | -24 |
| | Alternative 3 | 80 | 49 | 47 | 40 | -6 |
| | Alternative 6 | 88 | 61 | 59 | 56 | 49 |
| T-131 | Existing Conditions | 19 | 9 | -1 | -13 | -42 |
| | Alternative 3 | 21 | 13 | -2 | -12 | -56 |
| | Alternative 6 | 21 | 13 | -2 | -11 | -56 |
| R-132 | Existing Conditions | 24 | 18 | 16 | 11 | 10 |
| | Seawall Failure ³ | 24 | 18 | 8 | -20 | -38 |
| | Alternative 3 | 45 | 34 | 23 | 18 | 16 |
| | Alternative 6 | 48 | 34 | 23 | 18 | 16 |
| R-133 | Existing Conditions | 30 | 12 | 10 | -6 | -8 |
| | Alternative 3 | 29 | 12 | 10 | -4 | -8 |
| | Alternative 6 | 55 | 39 | 35 | 26 | 13 |
| R-134 | Existing Conditions | 54 | 30 | 23 | 11 | 0 |
| | Seawall Failure ³ | -17 | -17 | -17 | -17 | -17 |
| | Alternative 3 | 59 | 43 | 34 | 28 | 18 |
| | Alternative 6 | 68 | 59 | 55 | 44 | 40 |
| R-135 | Existing Conditions | 48 | -1 | -71 | -96 | -133 |
| | Alternative 3 | 81 | 50 | -55 | -88 | -119 |
| | Alternative 6 | 81 | 14 | 12 | 2 | -93 |
| R-136 | Existing Conditions | 8 | 2 | 0 | 0 | 0 |
| | Seawall Failure ³ | -14 | -19 | -20 | -30 | -42 |
| | Alternative 3 | 54 | 36 | 31 | 26 | 24 |
| | Alternative 6 | 110 | 71 | 66 | 54 | 50 |
| R-137 | Existing Conditions | -15 | -27 | -29 | -29 | -29 |
| | Seawall Failure ³ | -15 | -27 | -29 | -54 | -77 |
| | Alternative 3 | 13 | 22 | -10 | -16 | -22 |
| | Alternative 6 | 73 | 61 | 47 | 43 | -16 |
| R-138 | Existing Conditions | 0 | 0 | 0 | 0 | 0 |
| | Seawall Failure ³ | -21 | -51 | -88 | -144 | -142 |
| | Alternative 3 | 3 | 0 | 0 | 0 | 0 |
| | Alternative 6 | 28 | 18 | 13 | 8 | 1 |

¹Profiles R-129, T-131 and R-135 do not have a seawall. ²Values bolded in red represent erosion landward of the edge of maintained or improved property or infrastructure. Cells shaded yellow represent exposed seawalls. ³Simulations run assuming seawall had failed.

5.3. Future scenario without project conditions

Evaluating the existing conditions alone does not provide a complete perspective of the beach response to storms without a project. Based on the erosional trend along the Project Area, the beach profile is likely to continue recessing and lowering in elevation. To represent future scenarios without a project, 10-year and 50-year projections of beach profiles were developed and simulated with SBEACH. The existing condition profiles were translated landward based on the background erosion rate of 2.25 feet per year (CPE, 2013). Seawalls were included in the future scenarios as they were in the existing conditions simulations.

The landward limits of erosion for the future scenarios are presented in Table 5-4. Based on the future scenario simulations, all storm protection provided by the dune between R-130 and R-134 is lost. Seawalls that were buried within the dune have become exposed and are subject to wave action. The seawalls along the shoreline between R-136 and R-138 fail due to toe scour, allowing erosion of upland property and damage to infrastructure (Figure 5-1).

Table 5-4. SBEACH landward limit of storm erosion future scenario.

| FDEP R-Monument ¹ | Future Scenario (years in the future) | Landward Limit of Storm Erosion ² (feet from seaward edge of maintained property) Given Return Period in Years: | | | | |
|------------------------------|--|---|-------|------|------|------|
| | | 5 | 15 | 25 | 50 | 100 |
| R-129 | 10 | 91 | 59 | 39 | 29 | -9 |
| | 50 | 1 | -31 | -51 | -61 | -99 |
| R-130 | 10 | 13 | 5 | -2 | -11 | -16 |
| | 50 | -21 | -32 | -36 | -43 | -43 |
| T-131 | 10 | -11 | -34 | -40 | -67 | -83 |
| | 50 | -101 | -124 | -130 | -180 | -188 |
| R-132 | 10 | 29 | 16 | -7 | -15 | -34 |
| | 50 | -25 | -26 | -26 | -26 | -26 |
| R-133 ³ | 10 | 0 | -19.5 | -25 | -56 | -72 |
| | 50 | -37 | -37 | -37 | -37 | -37 |
| R-134 | 10 | 21 | 1 | -5 | -5 | -5 |
| | 50 | -5 | -5 | -5 | -5 | -5 |
| R-135 | 10 | -246 | -246 | -246 | -246 | -246 |
| | 50 | -236 | -236 | -236 | -236 | -236 |
| R-136 | 10 | 354 | 354 | 353 | 353 | 353 |
| | 50 | 354 | 354 | 354 | 354 | 355 |
| R-137 | 10 | 95 | 95 | 95 | 24 | -8 |
| | 50 | 77 | 77 | 77 | 77 | -51 |
| R-138 | 10 | -11 | -11 | -11 | -11 | -11 |
| | 50 | -11 | -11 | -11 | -11 | -11 |

¹Profiles R-129, T-131 and R-135 do not have a seawall.

²Values bolded in red represent erosion landward of the edge of maintained or improved property or infrastructure.

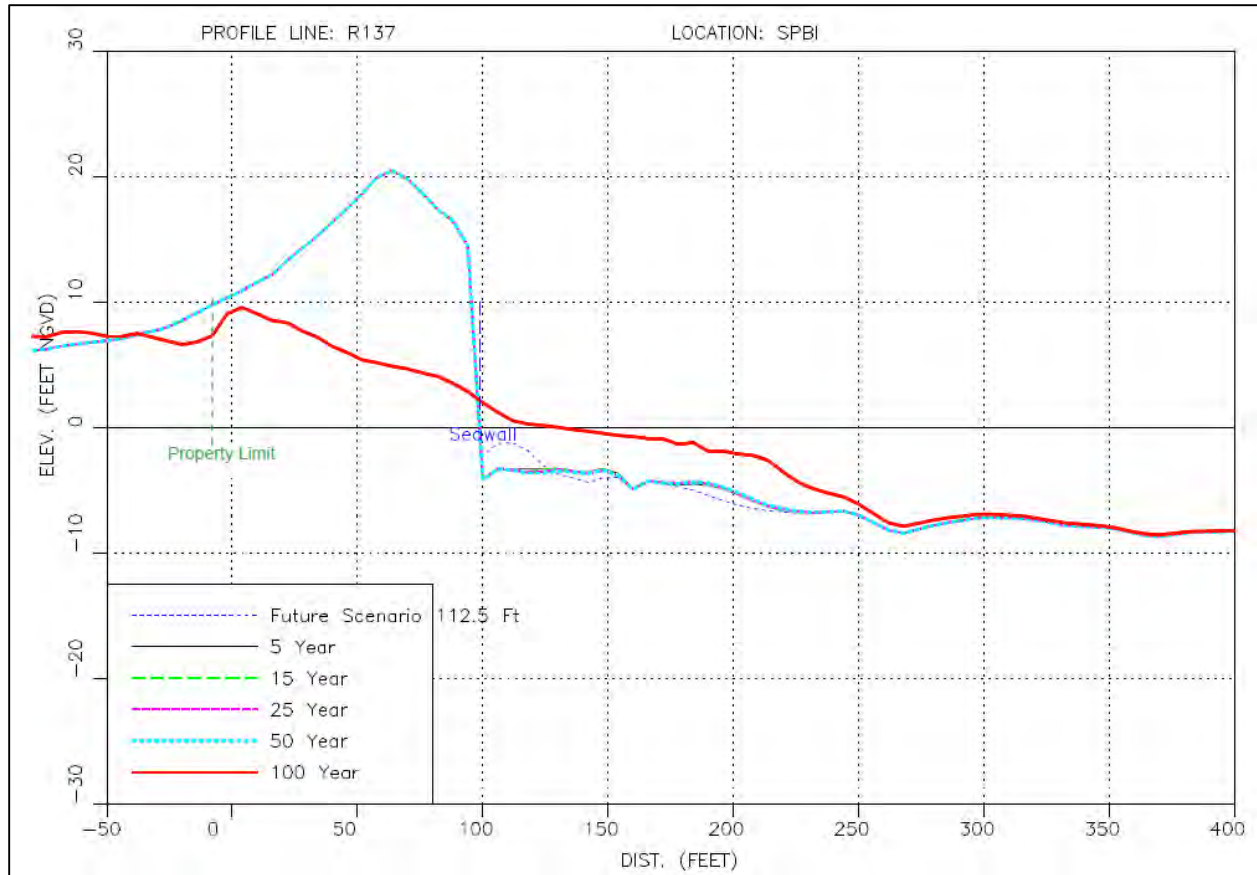


Figure 5-1. Seawall failure profile R-137 Future Scenario (50 years into the future).

5.4. Alternative 3: Applicants' Preferred Project without Shoreline Protection Structures

The Applicants' Preferred Alternative fill design consists of dune only and dune and berm fill from R-129-210 to R-134+135 and berm fill only from R-134+135 through R-138+551. No fill was simulated at R-129 since the existing conditions met the design criteria for the seaward dune extent. The placement of berm fill only from R-134+135 to R-138+551 allows the seawalls to remain partially exposed.

The project prevents scouring at the toe of the seawalls at all locations simulated except R-136 and R-138 (Appendix C). At these two locations, scouring increases incrementally with magnitude of storm. Furthermore, none of the buried seawalls were exposed as a result of the return interval storms. No seawall failures were observed during the simulations.

In general, the project provides storm protection against a 15-year storm with little to no impact to the pre-construction profile (Table 5-3). Under the occurrence of a 5, 15 and 25-year storm, the frontal dunes present at profiles R-129 through R-133 retained their shape but lost volume. Recession into the pre-construction profile increases with increasing magnitude of return interval storm. The berm profile remains at a 2 to 3-foot higher elevation than the pre-construction profile even after a 100-year storm.

5.5. Alternative 6: The Town of Palm Beach Increased Sand Volume Project and County Increased Sand Volume Project without Shoreline Protection Structures

Alternative 6 consists of a wider dune fill at profiles R-129-210 through R-134+135 and a wider berm fill at profiles R-134+135 through R-138+551 than the Applicants' Preferred Alternative. Berm widths range from approximately 17 to 130 feet from the pre-construction profile (Table 4-3).

The project prevents scouring at the toe of the seawalls at all locations (Appendix C). None of the buried seawalls were exposed as a result of the return interval storms. No seawall failures were observed during the simulations.

In general, the project provides storm protection against a 15-year storm with little to no impact to the pre-construction profile from profiles R-129 to R-134 and 50-year return interval storm protection to the pre-construction profiles from R-135 through R-138. Under the occurrence of a 5, 15 and 25-year storm, the frontal dunes present at profiles R-129 through R-133 retained their shape but receded and lost volume. Recession into the pre-construction profile increases with increasing magnitude of return interval storm. The berm profile remains at a 2 to 5-foot higher elevation than the pre-construction profile even after a 100-year storm.

Based on the landward limit of erosion calculation, damage to property is possible adjacent to profile T-131 as a result of a 25-year return interval or stronger storm (Table 5-3). Property along profiles R-135 and R-137 are at risk of damage during the occurrence of a 100-year return interval storm or stronger storm.

6.0 CONCLUSIONS AND RECOMMENDATIONS

To determine the level of storm protection provided by existing and potential dunes and berms along the Project Area, the SBEACH model was applied and storm erosion given the existing (Winter 2011/2012) conditions and two alternatives of beach and dune fill cross-sections was analyzed. The following conclusions were made based on the results of the model study:

- The critical return interval storm resulting in property damage under existing conditions is between a 15-year and 25-year storm. On average, 7.3 to 7.7 cy/ft was simulated to erode from the beach above MLW during a 15-year and 25-year storm, respectively. This volumetric loss coincides with a steepening of the dune face, shoreline retreat and lowering of the beach profile elevation. Based on 2011/2012 conditions, erosion and wave impacts were simulated to extend landward damaging infrastructure and maintained (landscaped) property areas at FDEP R-monuments R-130, R-133, R-135 and R-137. These locations lack seawalls or have seawalls located further landward on the property.
- Seawalls prevent erosion into the upland property until wall failure. Scouring at the toe of exposed seawalls increases their likelihood of failure. Based on the 2011/2012 conditions response to a storm event, the berm elevation adjacent to exposed seawalls will lower increasing the likelihood of seawall failure during storms. If seawall failure is assumed to occur along the Project Area, infrastructure would be impacted from R-130 through R-138. A detailed analysis of the structural stability of the individual seawalls along the Project Area would be necessary to truly assess the vulnerability of this critical component of storm protection infrastructure.
- Based on the SBEACH simulations and background erosion rates, the status quo dune nourishments alone are not sufficient to sustain the existing conditions. The No Action Status Quo conditions for the Project Area include dune nourishments of 5 to 11 cy/ft fill between R-135+460 to R-137+410 and

R-129 to R-133, respectively, placed every 1 to 5 years. This conclusion is made based on the storm response simulation of the 2011/2012 conditions which are representative of the No Action Status Quo Scenario. The 2011/2012 conditions represent the beach 9 months to 3 years after a dune nourishment and without the impacts of a major storm. The majority if not all of this placed volume would be lost during a 15-year storm.

- Based on the simulation of two forecasted No Action scenarios 10 and 50 years from the present (not Status Quo, no dune nourishments included in simulation setup), all remaining storm protection provided by the dune between R-130 and R-134 would be lost after one major storm event. Seawalls that were buried within the dune would become exposed and subjected to wave action. The seawalls between R-136 and R-138 would possibly fail due to toe scour depending on the depth of the wall, allowing erosion of upland property and damage to infrastructure.

7.0 LITERATURE CITED

Allsop, W., Bruce, T., Pearson, J., Besley, P. 2005. Wave overtopping at vertical and steep seawalls. *Maritime Engineering*. 158. pp. 103-114.

Applied Technology & Management. 2012. Town of Palm Beach Offshore Sand Search Investigations South Area Summary. Prepared for the Town of Palm Beach.

Coastal Planning & Engineering. 2013. Central Palm Beach County Comprehensive Erosion Control Project Reformulated Shore Protection Alternatives. Prepared for Palm Beach County. 109 p.

Coastal Planning & Engineering. 2007. Town of Palm Beach Reach 8 Beach Nourishment Project Response to RAI #3 JCP File Number 0250572-001-JC; July 2007; CPE02829-03802.

Federal Emergency Management Agency. 1982. Flood Insurance Study, Wave Height Analysis, Town of South Palm Beach, Florida, Palm Beach County. 15 p.

Florida Department of Environmental Protection. 2013. Palm Beach Island Beach Management Agreement. <http://www.dep.state.fl.us/BEACHES/pb-bma/index.htm>

Florida Department of Environmental Protection. 2014. Critically Eroded Beach in Florida. Updated June, 2014.

Larson M., and Kraus, N.C., 1989. SBEACH: Numerical Model for Simulating Storm-Induced Beach Change, 2 Vols., Technical Report CERC 89-9, U.S. Army Corps of Engineers, Washington.

Larson, M., Wise, R.A., Kraus, NC., 2004. Modeling Dune Response by Overwash Transport, Proceedings of the International Conference on Coastal Engineering 2004, American Society of Civil Engineers, Reston, VA, pp. 2133-2145.

National Oceanographic and Atmospheric Administration, 2013. WAVEWATCH III Model Data Access, <http://polar.ncep.noaa.gov/waves/download.shtml?>.

National Oceanographic and Atmospheric Administration, 2003. Bench Mark Data Sheets, Lake Worth Pier, FL, Station ID: 8722670, <http://tidesandcurrents.noaa.gov/>.

U.S. Army Corps of Engineers, 2000. Coastal Engineering Manual, Engineer Manual 1110-2-1100, U.S. Army Corps of Engineers, Washington, D.C. (in 6 volumes).

U.S. Army Corps of Engineers, 1991. Measurements for Lake Worth, FL, http://sandbar.wes.army.mil/public_html/pmab2web/htdocs/florida/lakeworth/lakeworth.html.

U. S. Army Engineer Waterways Experiment Station. 1985. Estimates of Hurricane Winds for the East and Gulf Coasts of the United States. CETN-I-36. 7 p.

SUB-APPENDIX G-1
SBEACH ANALYSIS REPORT

ATTACHMENT A
PROSPECTIVE DESIGN PROFILES

SUB-APPENDIX G-1
SBEACH ANALYSIS REPORT

ATTACHMENT B
INPUT STORMS FOR SBEACH MODEL

SUB-APPENDIX G-1
SBEACH ANALYSIS REPORT

ATTACHMENT C
SBEACH MODEL RESULTS

SUB-APPENDIX G-1
SBEACH ANALYSIS REPORT

ATTACHMENT C-1
EXISTING CONDITIONS (2011/2012 SURVEY)

SUB-APPENDIX G-1
SBEACH ANALYSIS REPORT

ATTACHMENT C-2
EXISTING CONDITIONS (2011/2012 SURVEY)
NO SEAWALL/SEAWALL FAILURE

SUB-APPENDIX G-1
SBEACH ANALYSIS REPORT

ATTACHMENT C-3
ALTERNATIVE 3 (APPLICANTS' PREFERRED)

SUB-APPENDIX G-1
SBEACH ANALYSIS REPORT

ATTACHMENT C-4
ALTERNATIVE 6 (LARGER FILL DESIGN)

SUB-APPENDIX G-1
SBEACH ANALYSIS REPORT

ATTACHMENT C-5
FUTURE SCENARIO (WITHOUT PROJECT CONDITIONS)

SUB-APPENDIX G-1
SBEACH ANALYSIS REPORT

ATTACHMENT D
LANDWARD LIMIT OF RECESSION BY RETURN PERIOD STORM BASED ON
SBEACH RESULTS

SUB-APPENDIX G-1
SBEACH ANALYSIS REPORT

ATTACHMENT E
SBEACH ADDITIONAL MODELING STUDY

SUB-APPENDIX G-1
SBEACH ANALYSIS REPORT

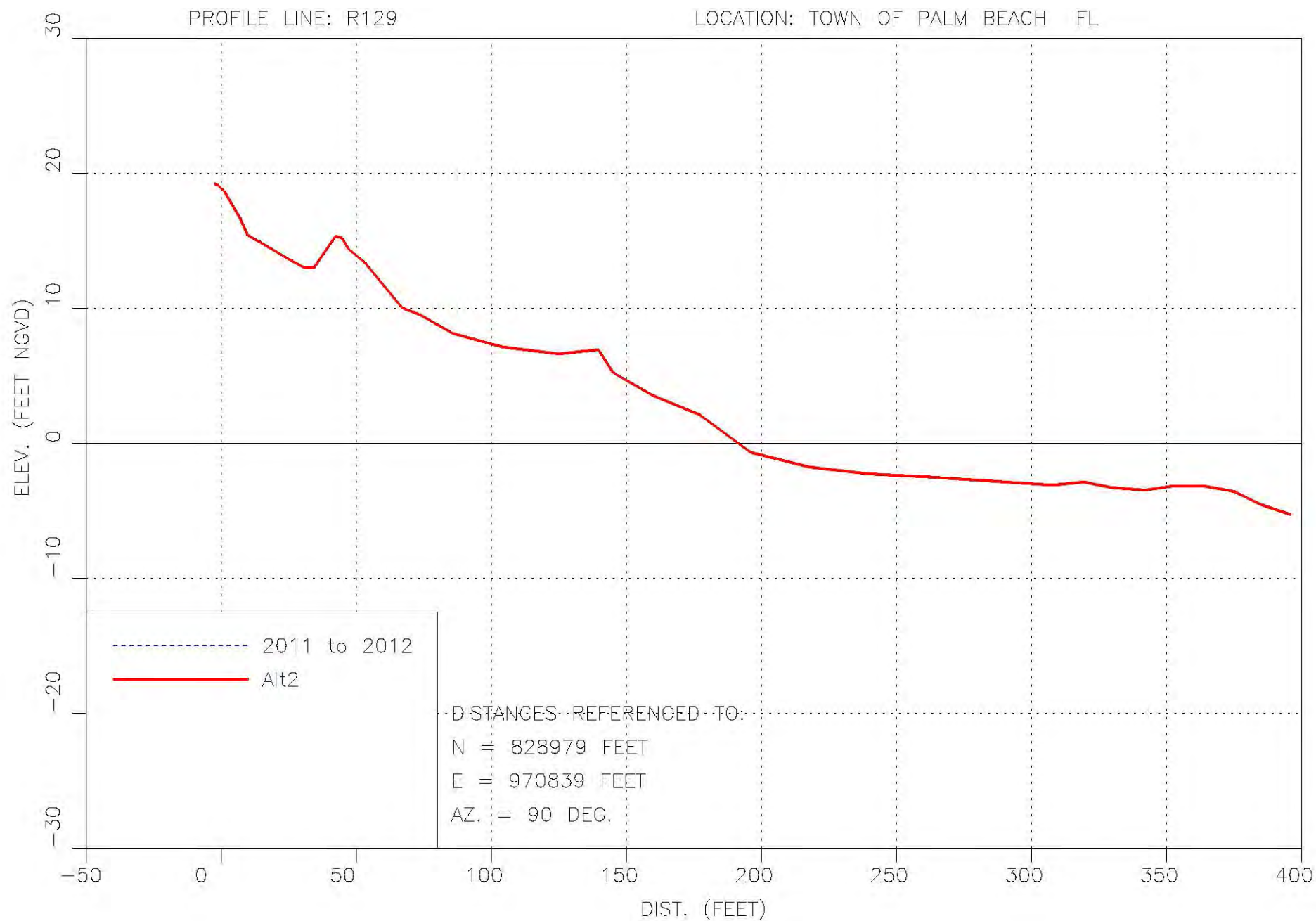
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SBEACH RESULTS FOR ADDITIONAL MODELING STUDY

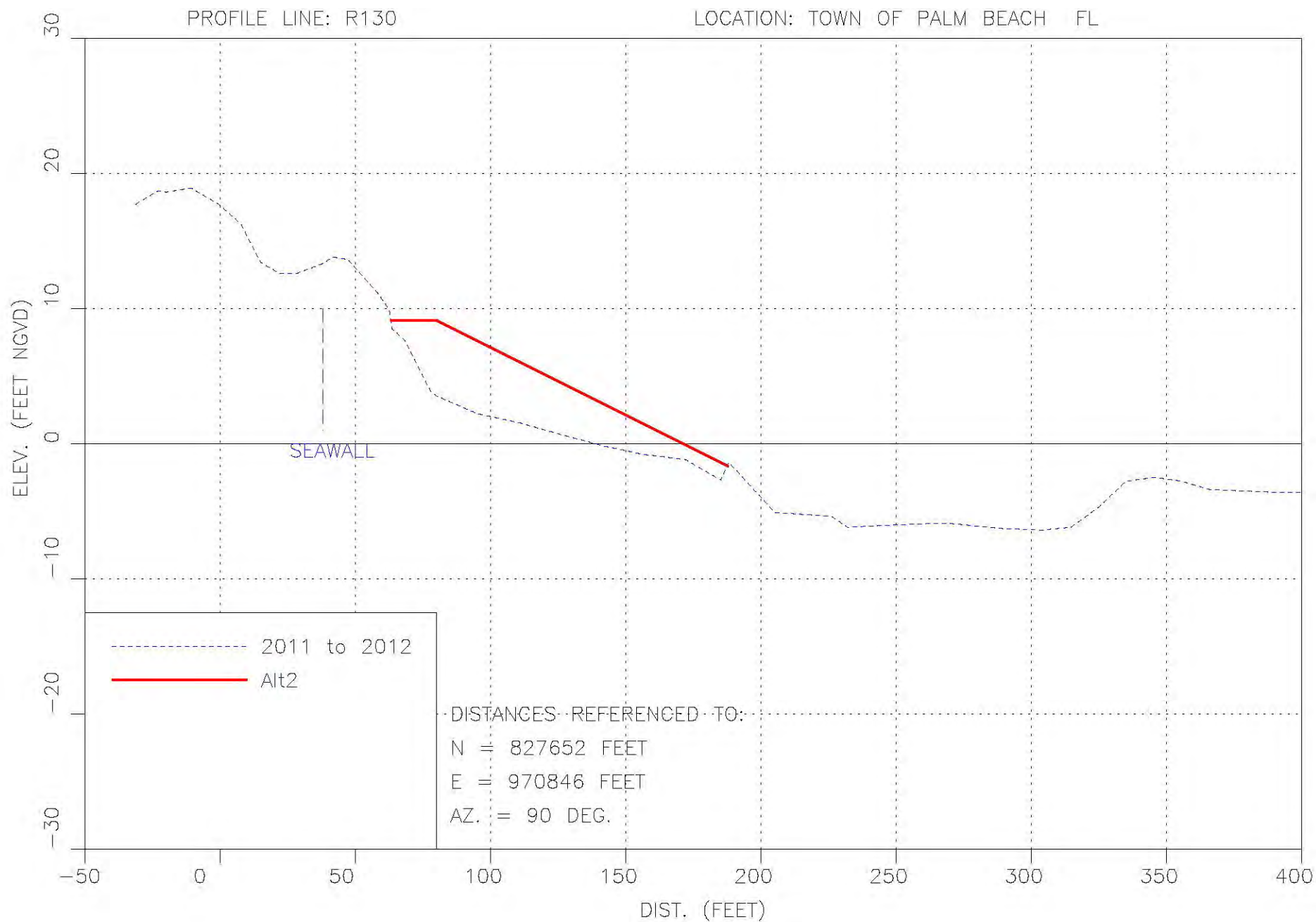
SUB-APPENDIX G-1
SBEACH ANALYSIS REPORT

ATTACHMENT A
PROSPECTIVE DESIGN PROFILES

PROFILE LINE: R129

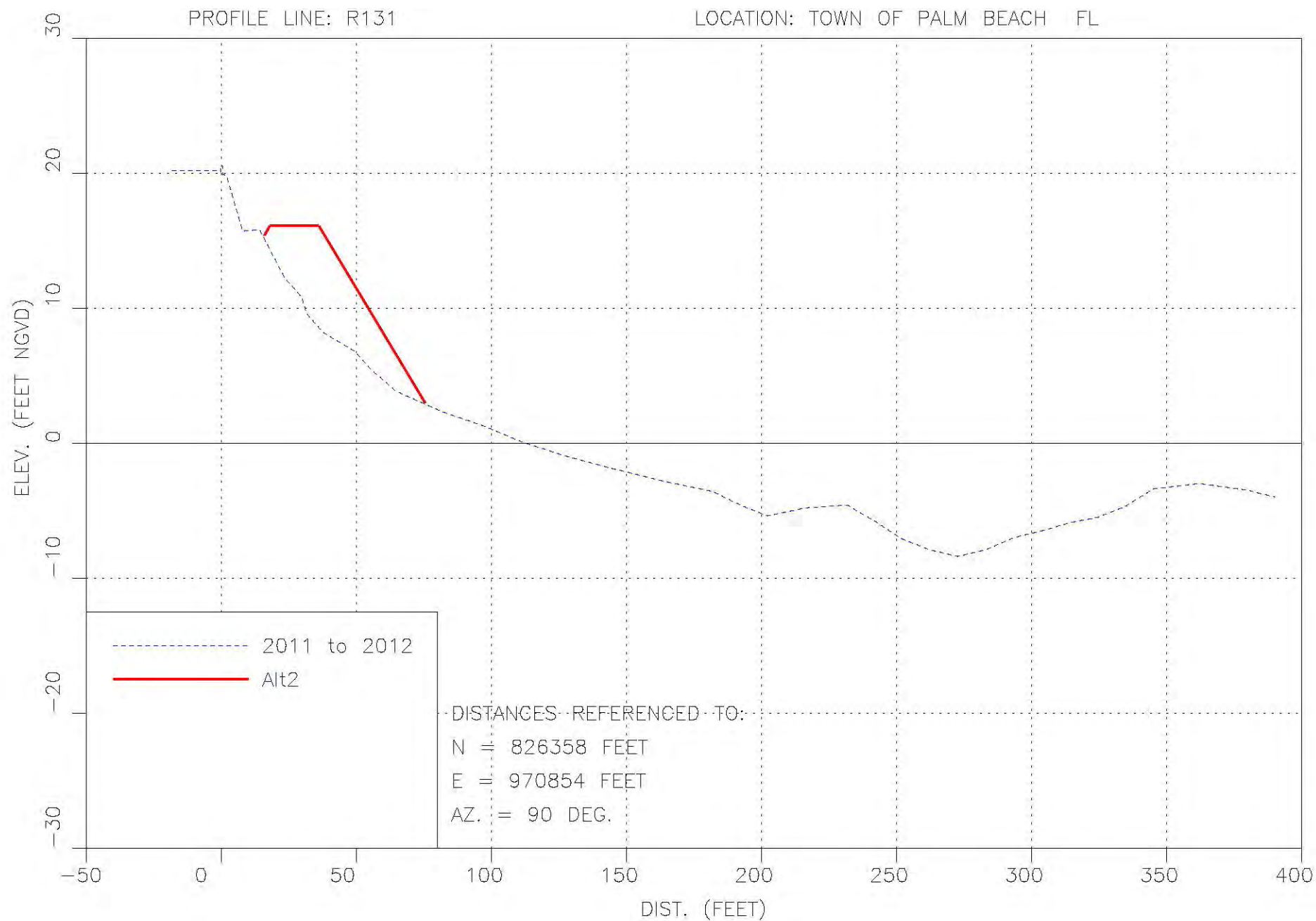
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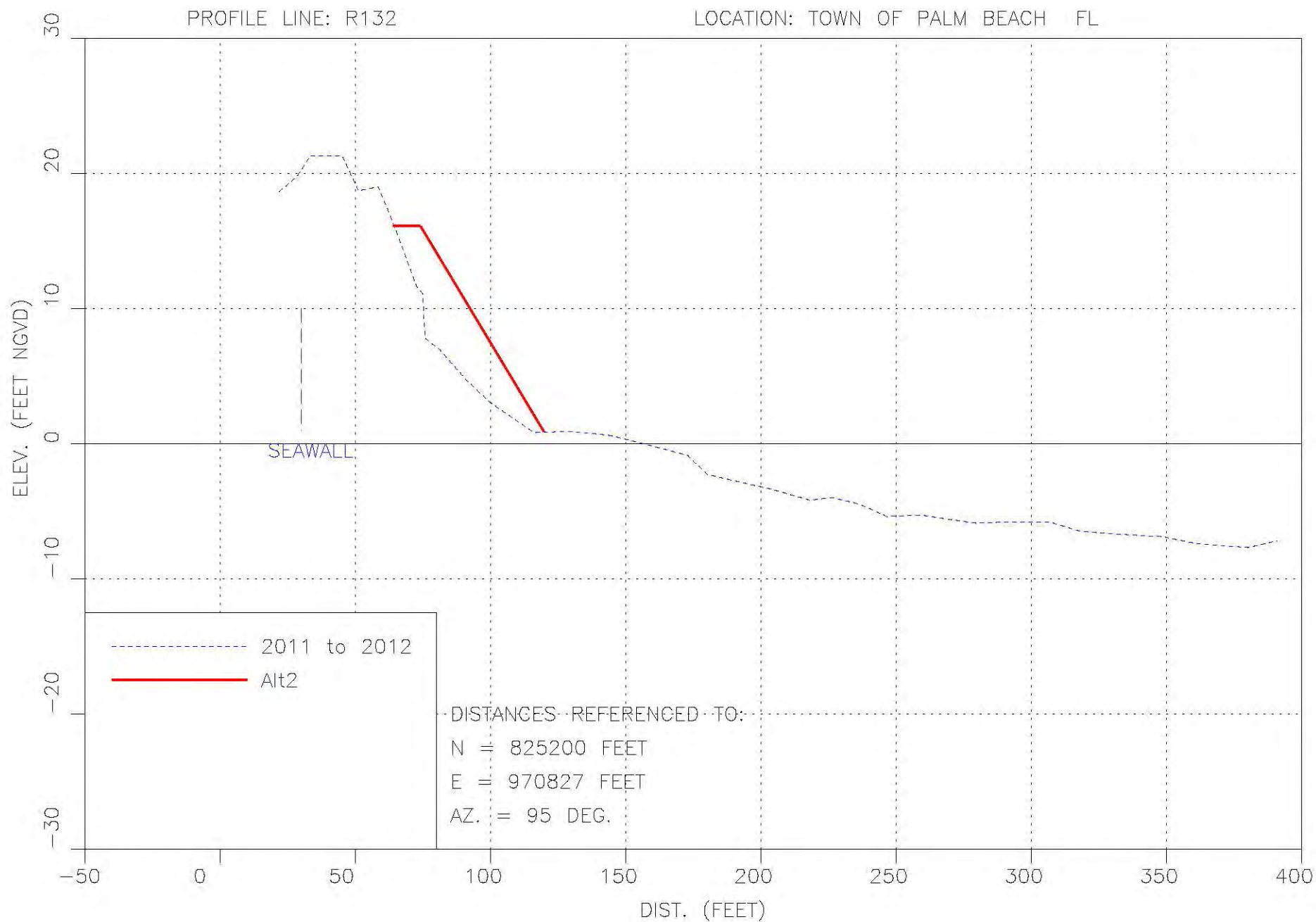




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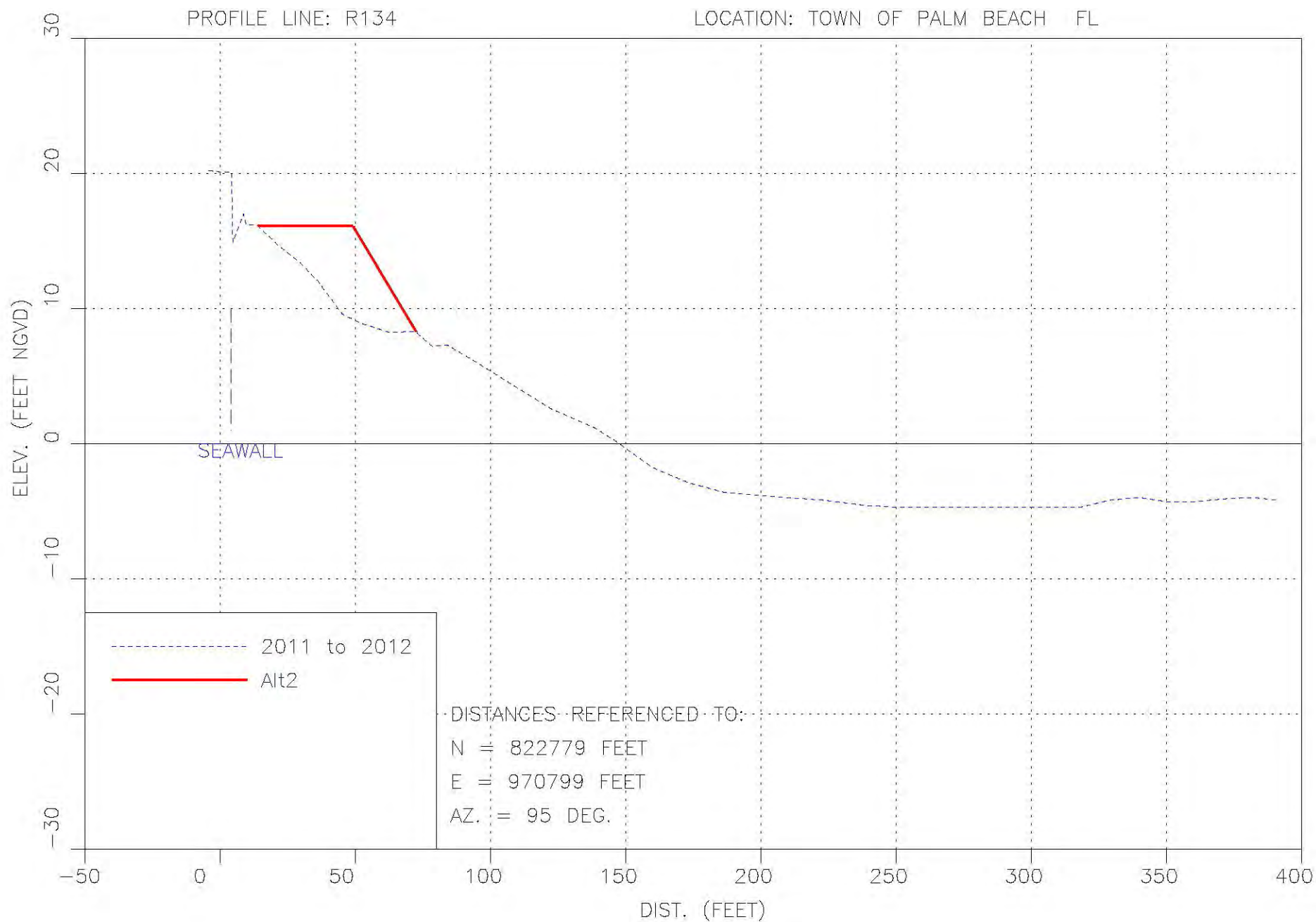




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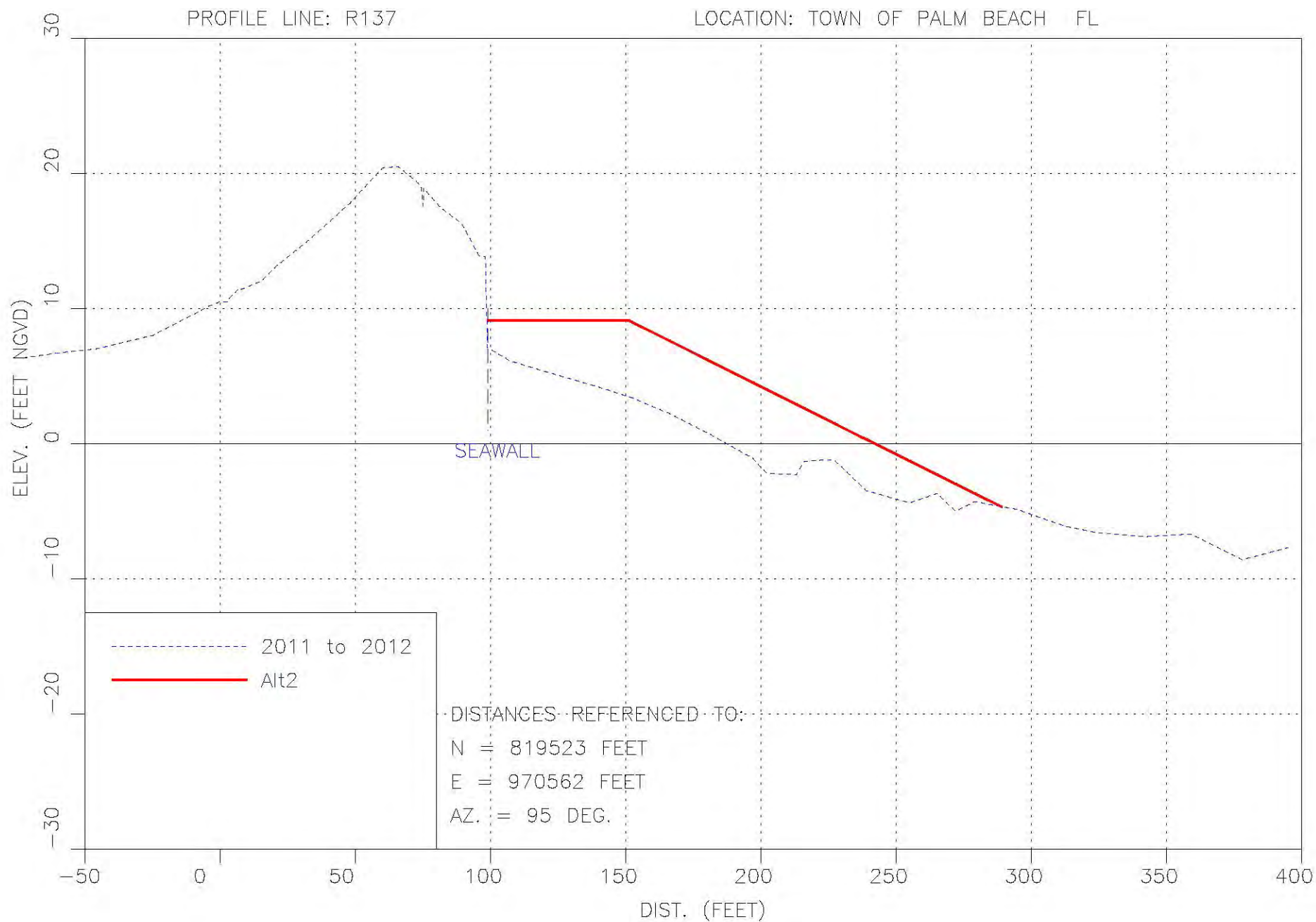


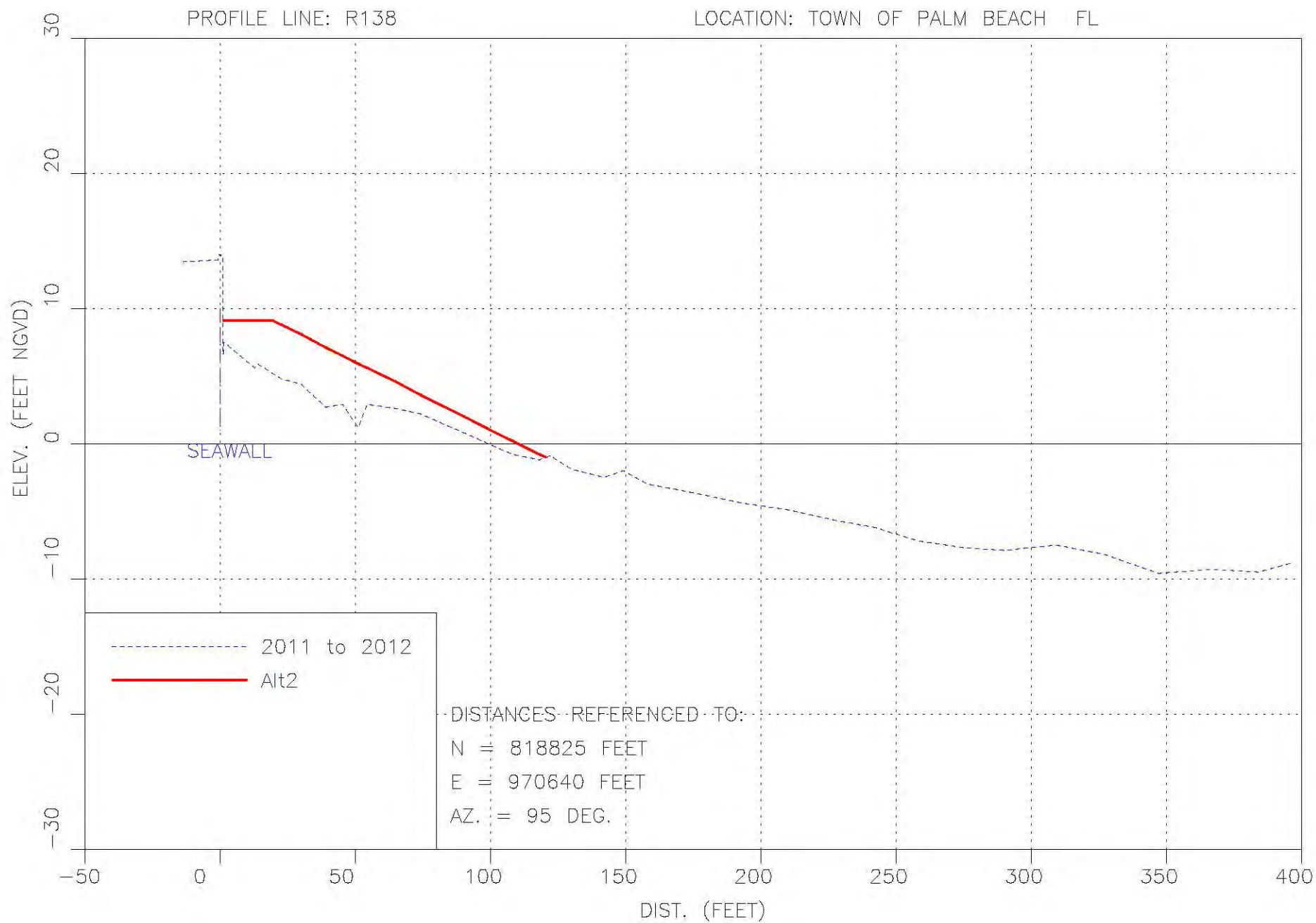
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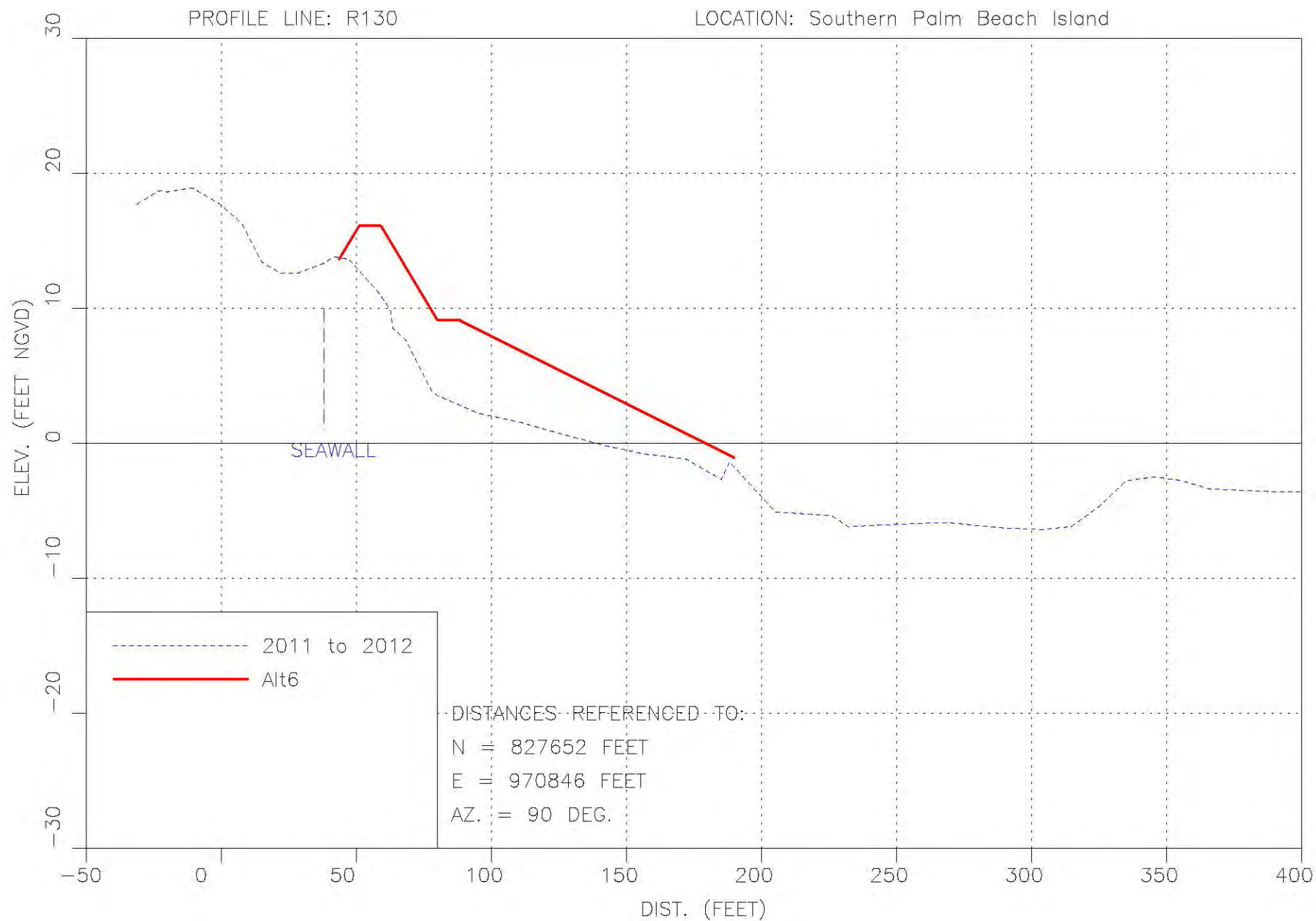


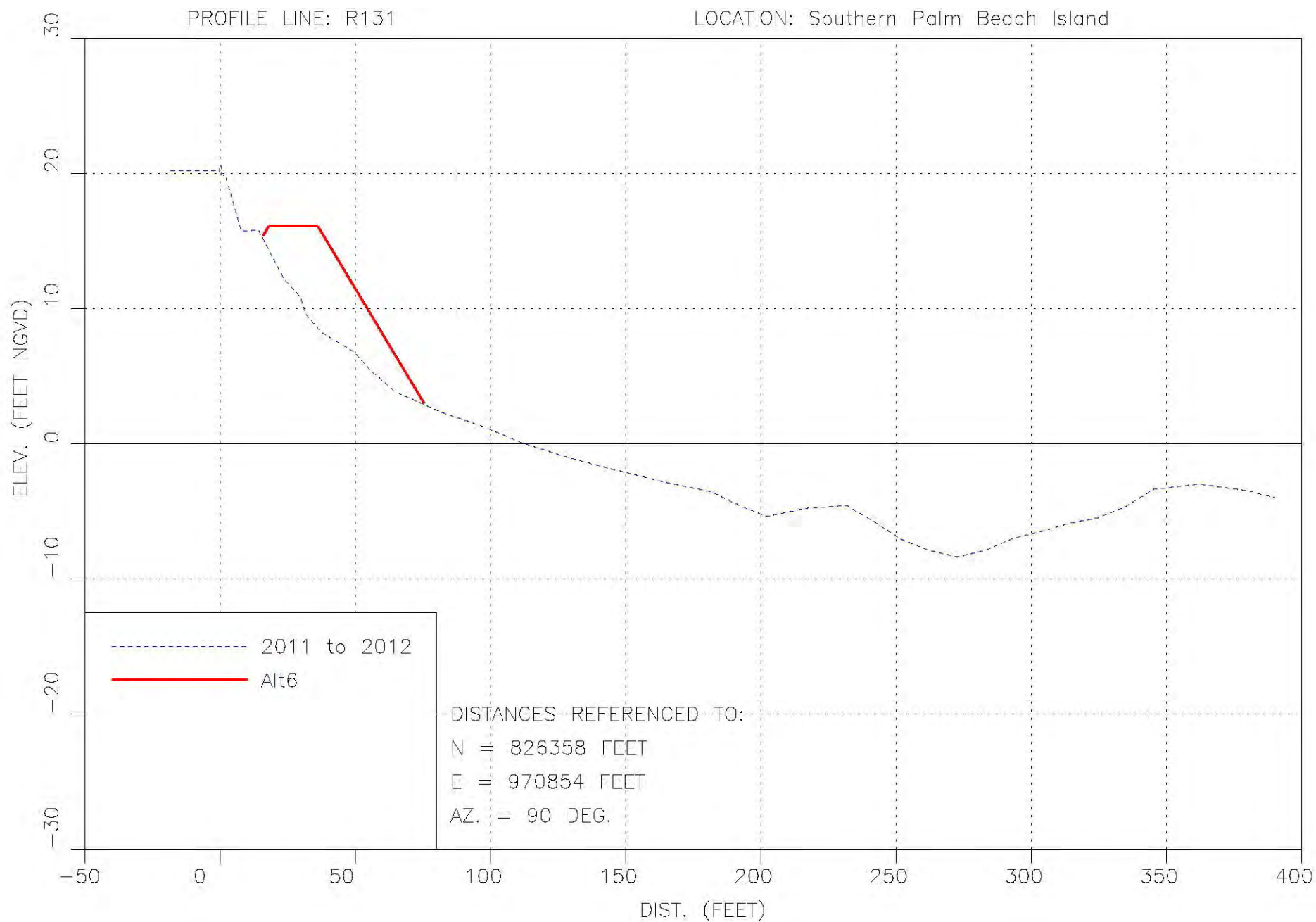


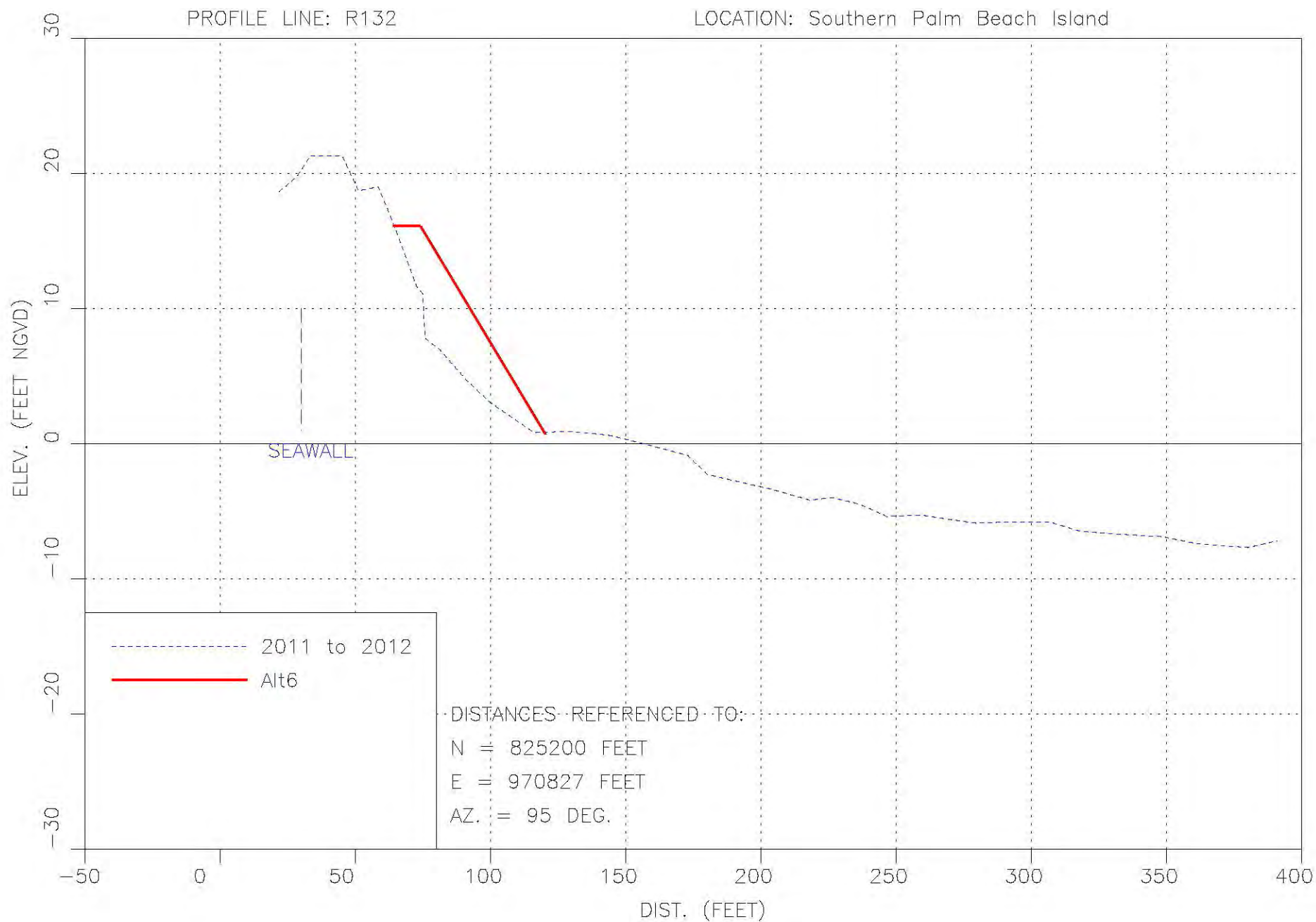






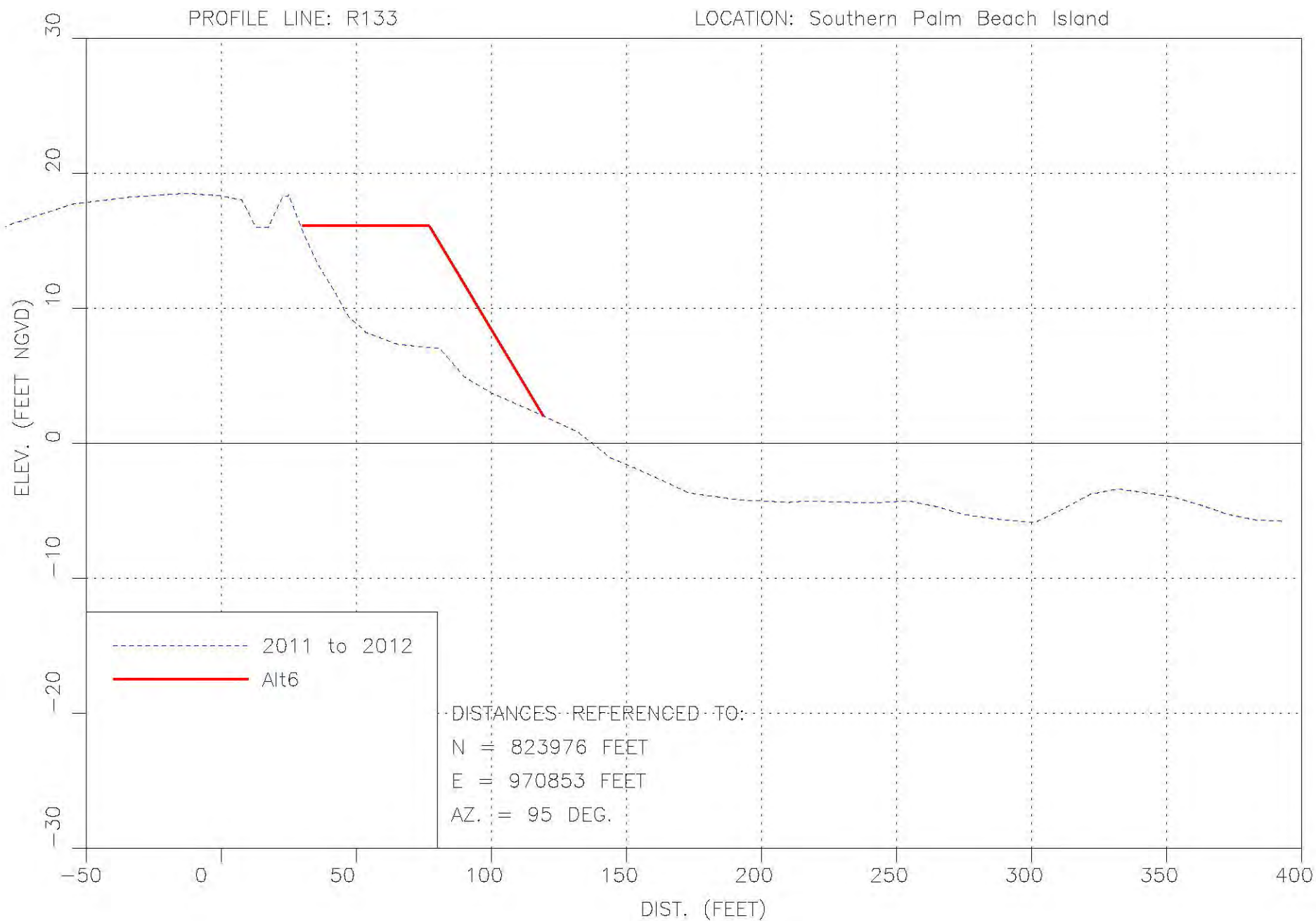


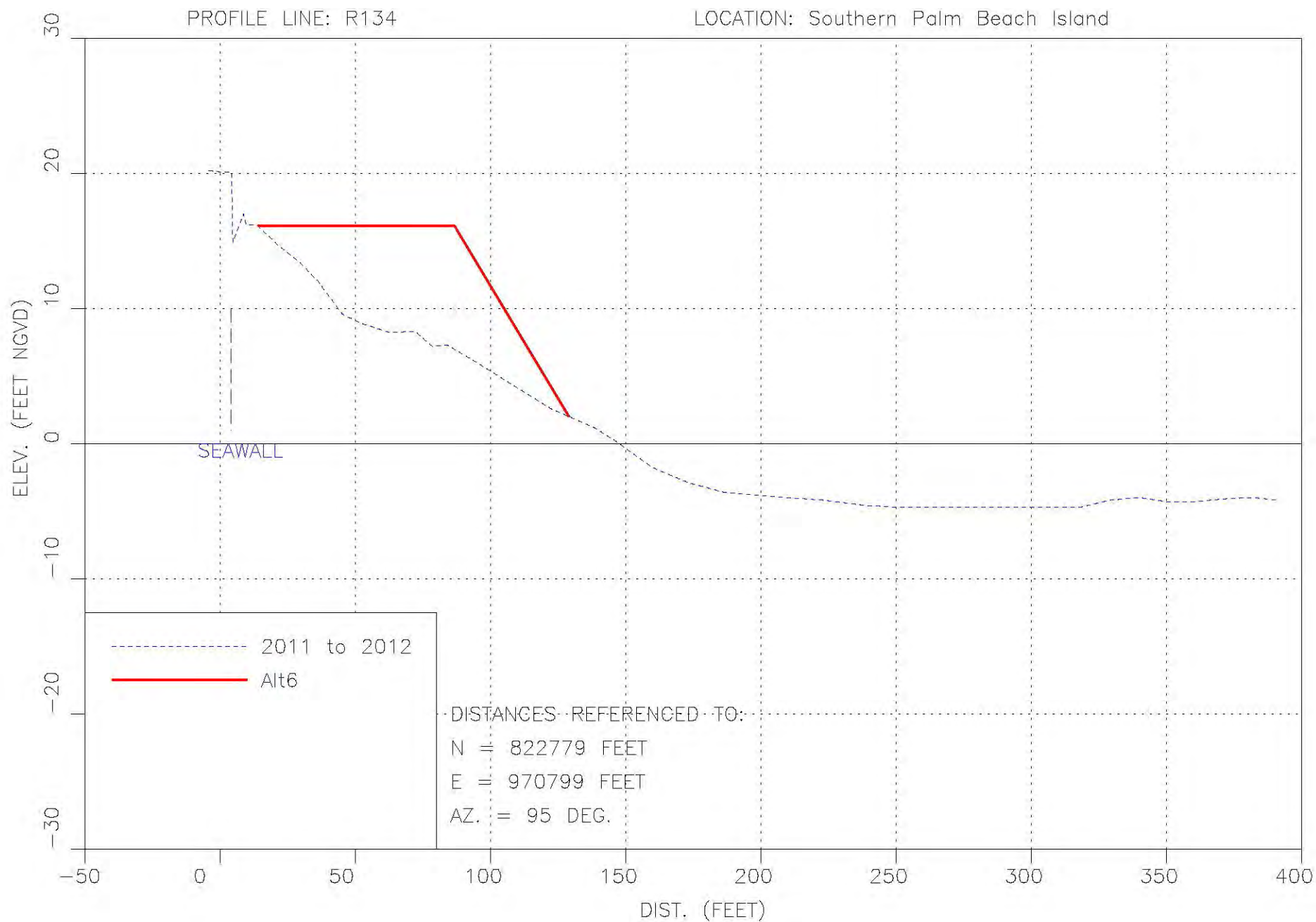




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LOCATION: Southern Palm Beach Island

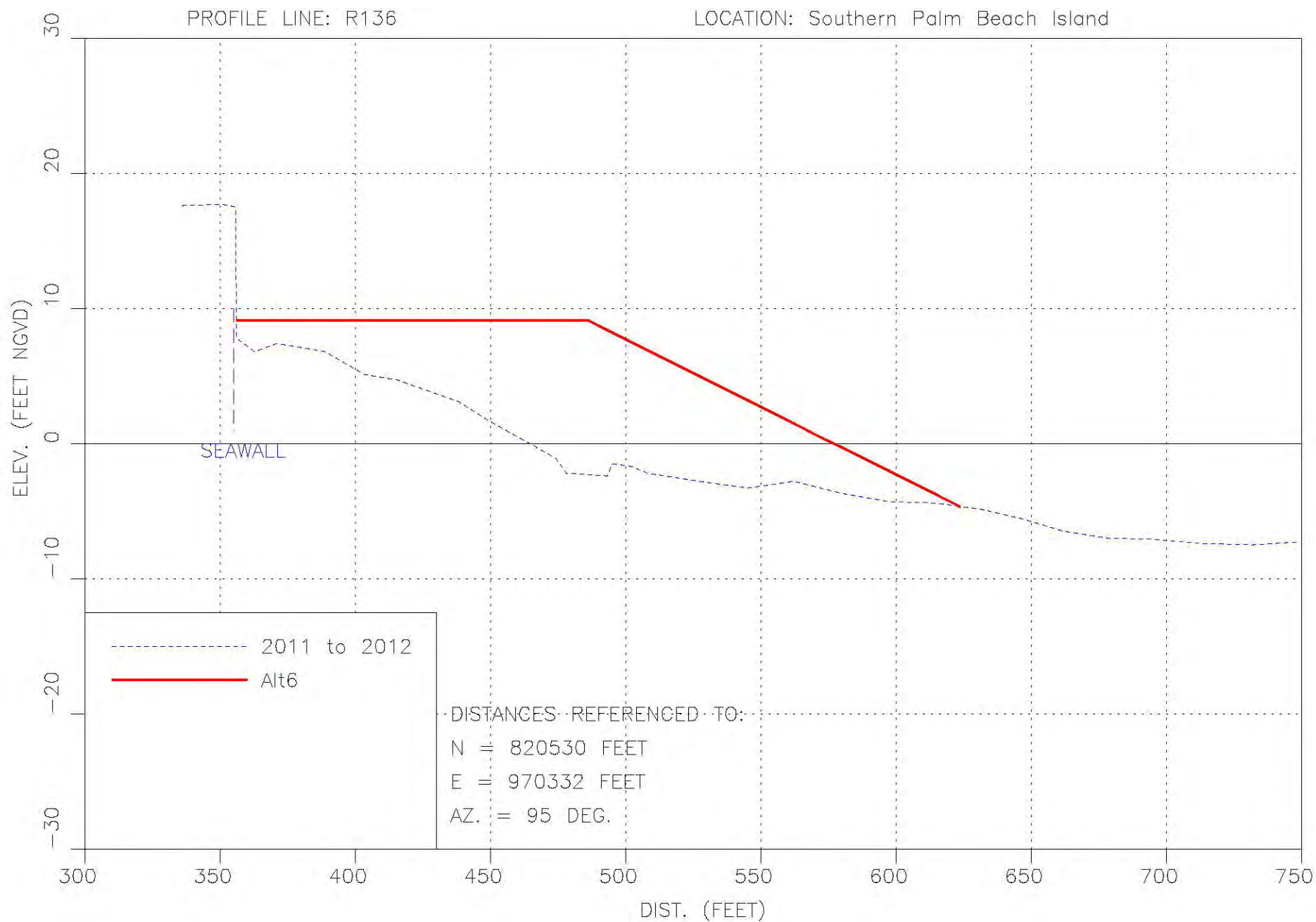




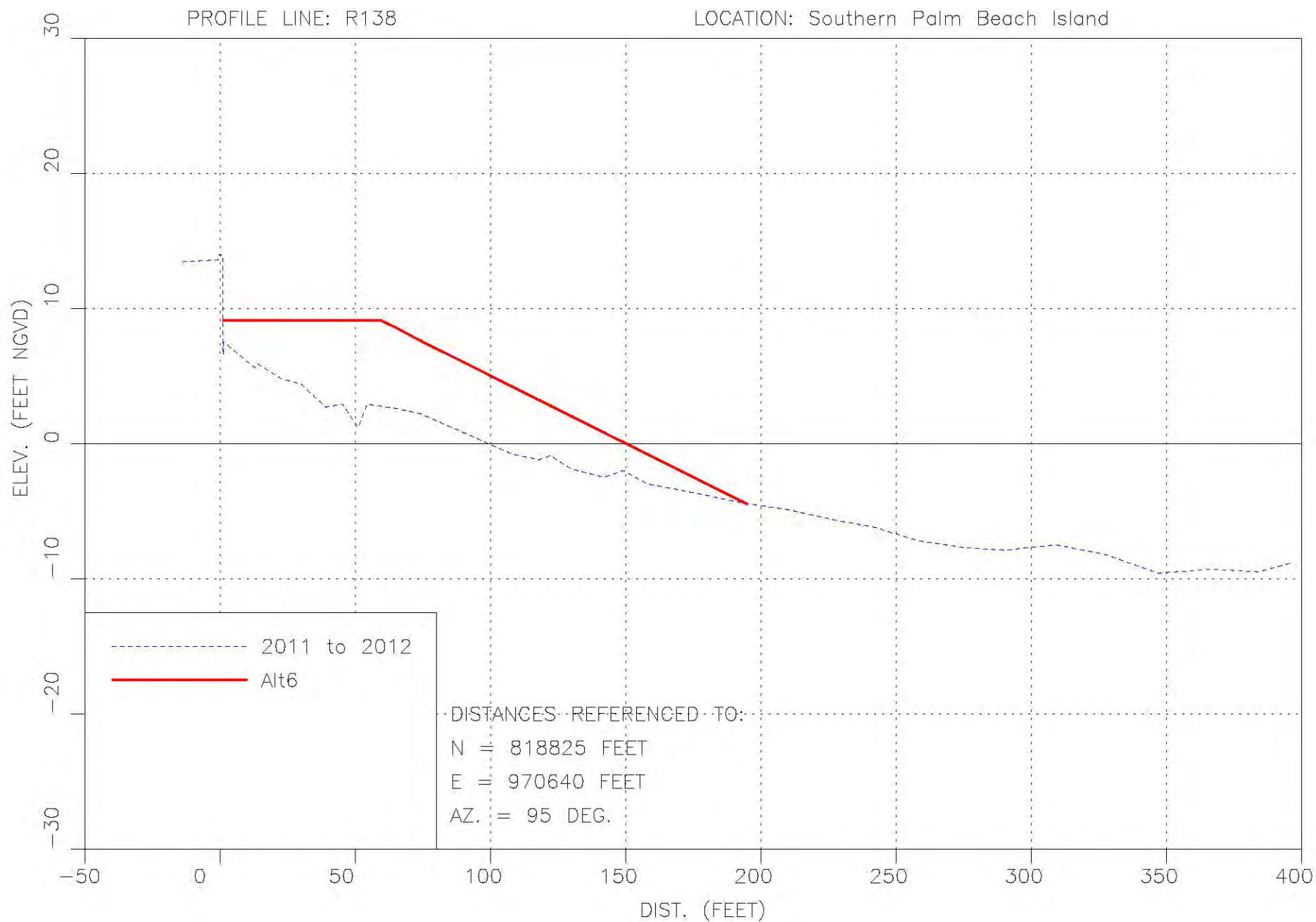
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LOCATION: Southern Palm Beach Island





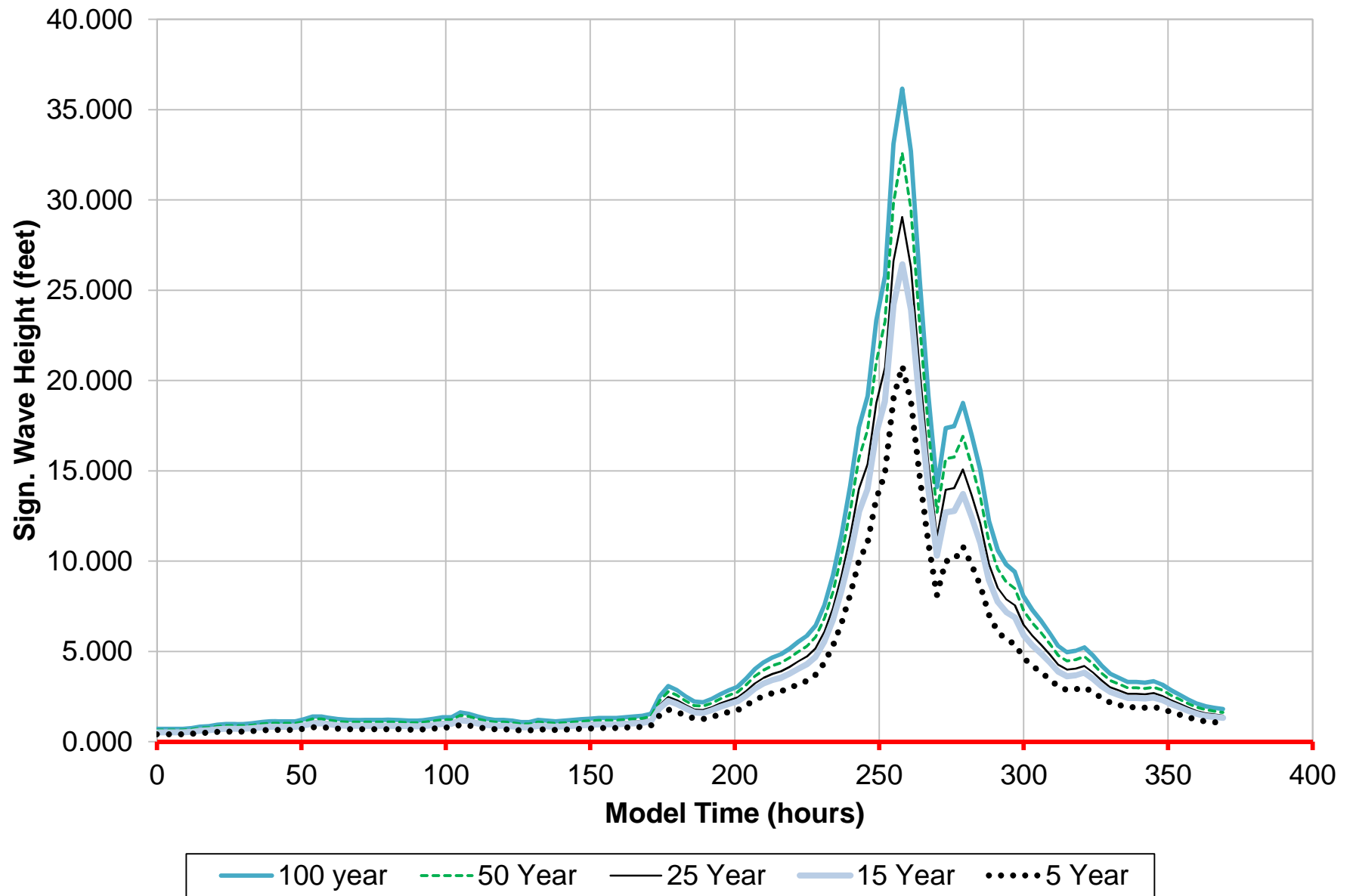




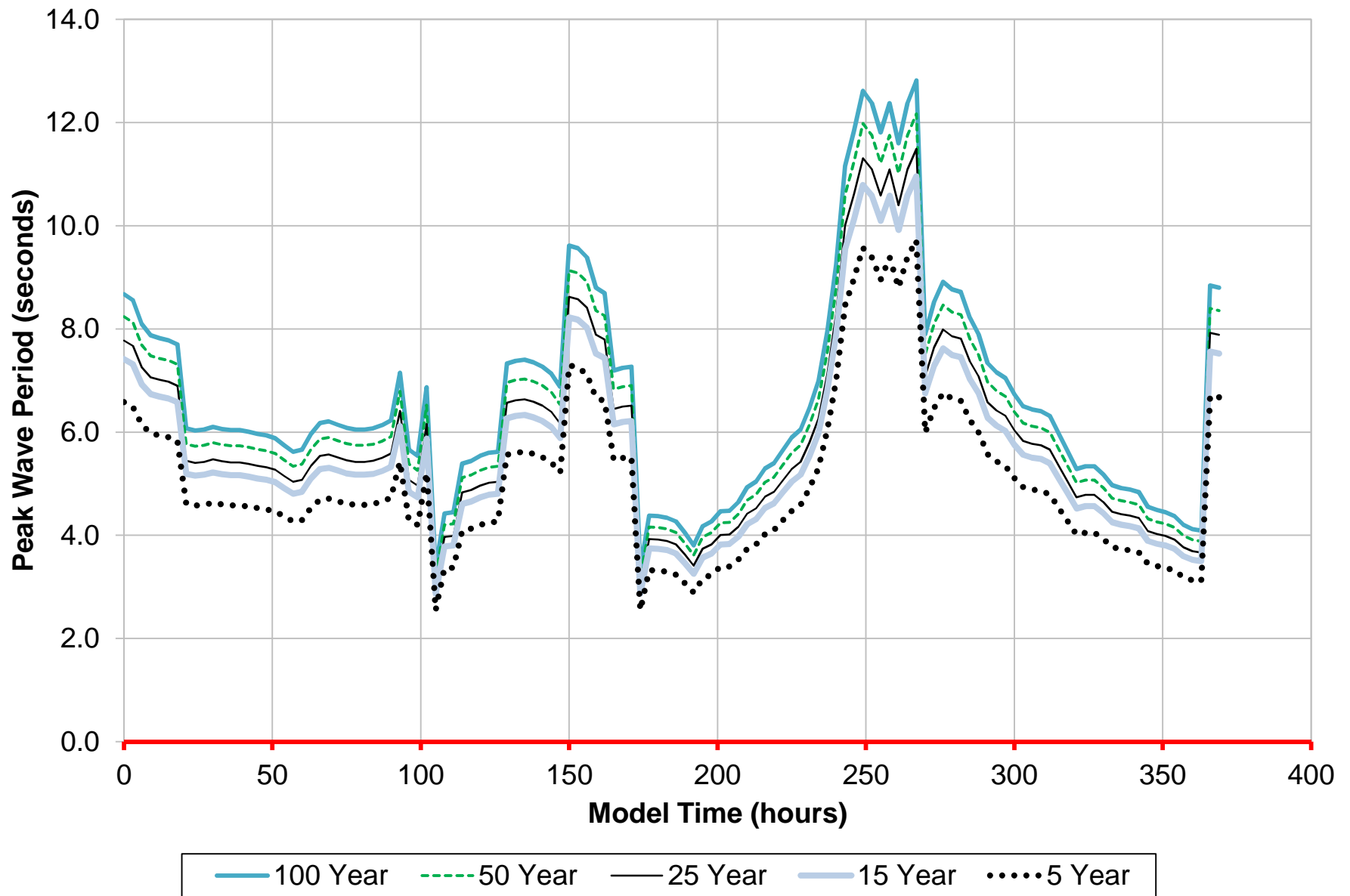
SUB-APPENDIX G-1
SBEACH ANALYSIS REPORT

ATTACHMENT B
INPUT STORMS FOR SBEACH MODEL

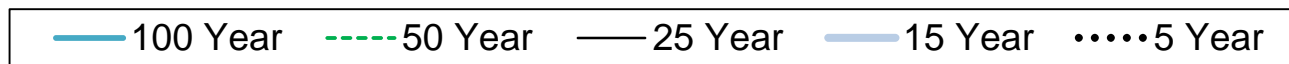
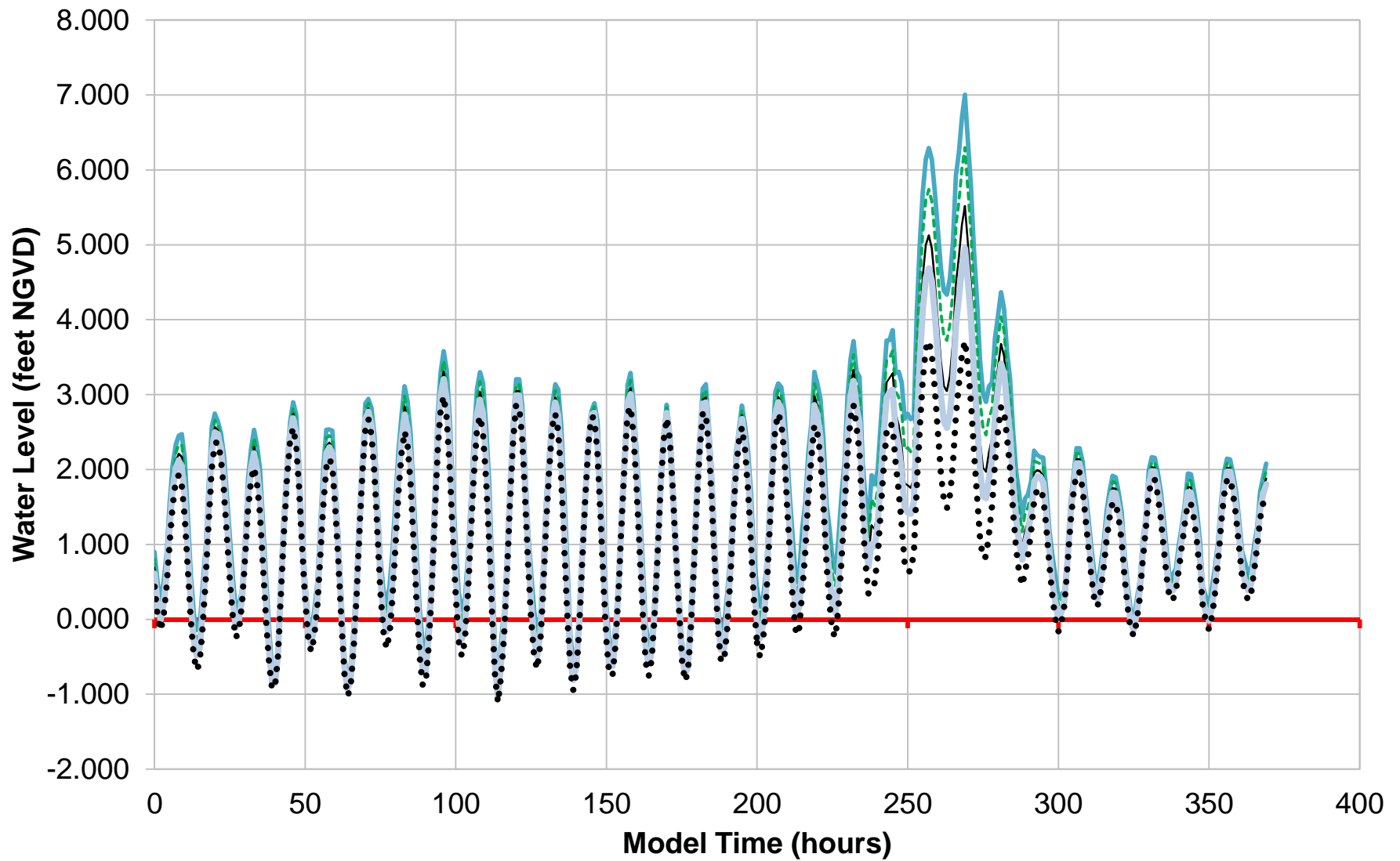
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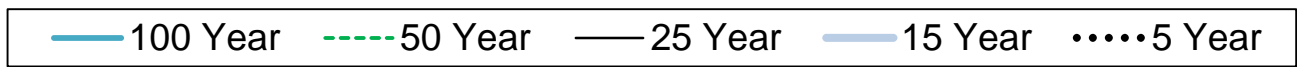
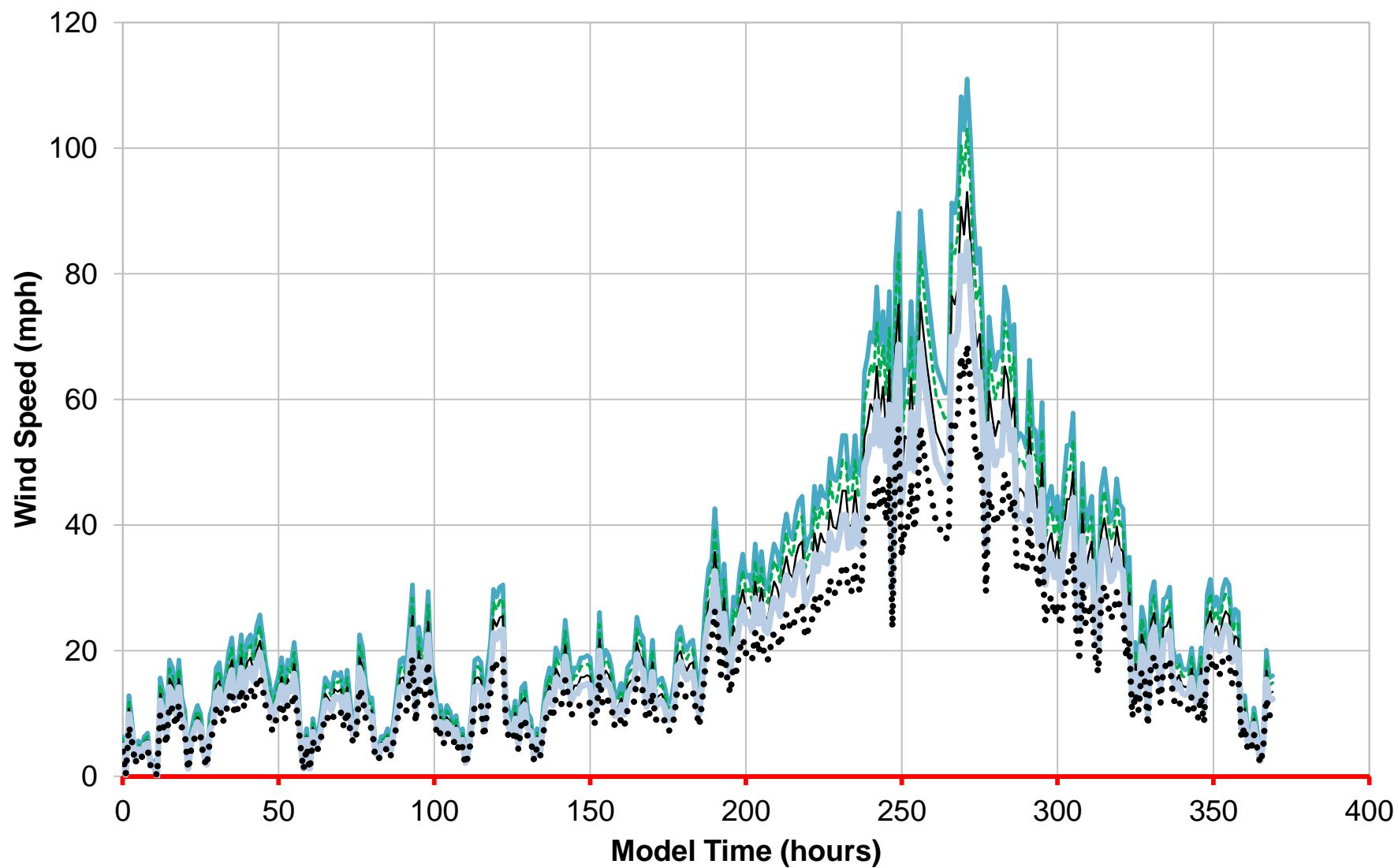
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(Input to SBEACH Model)**



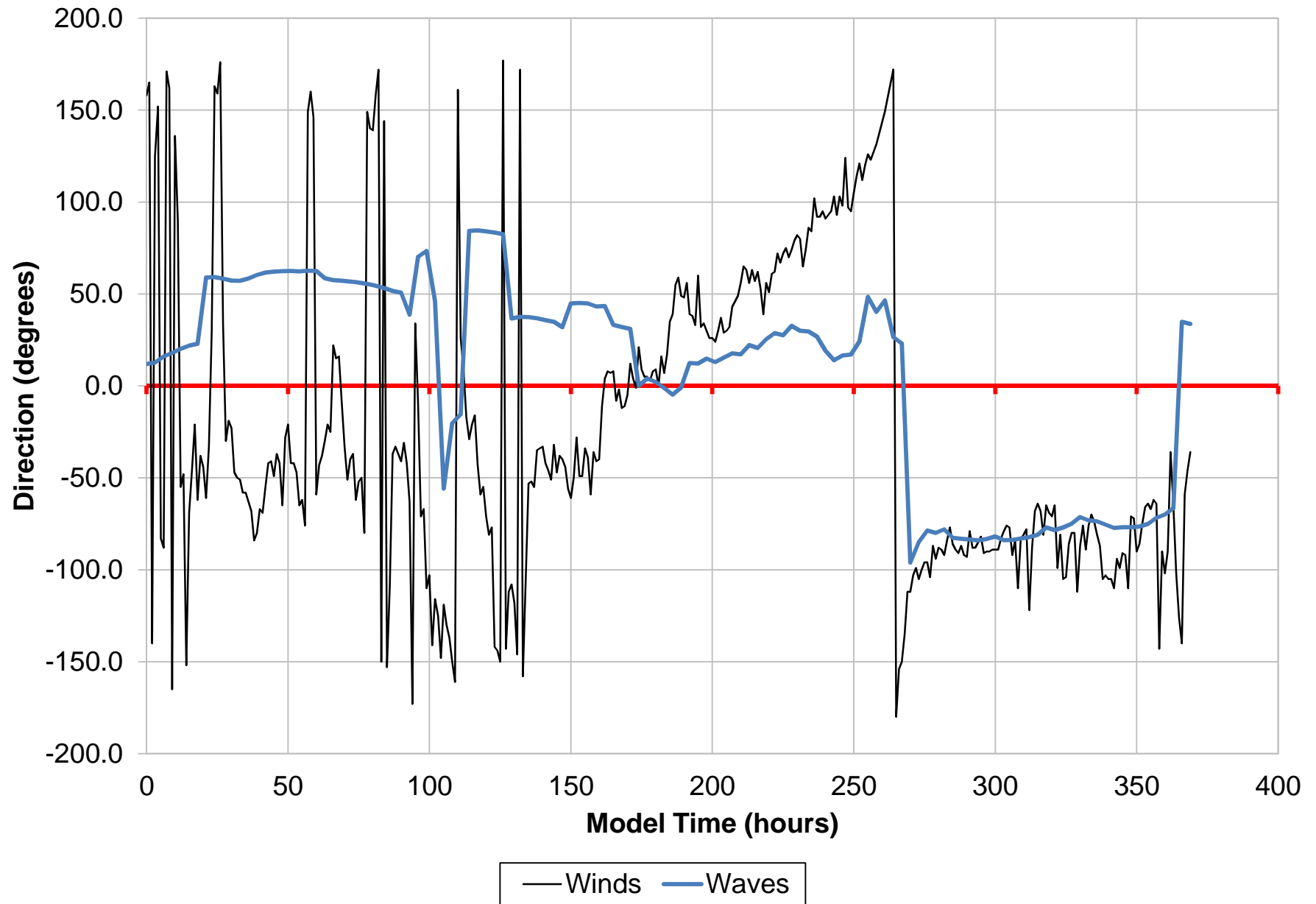
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Winds (Input to SBEACH Models)



Wind Directions & Offshore Wave Directions

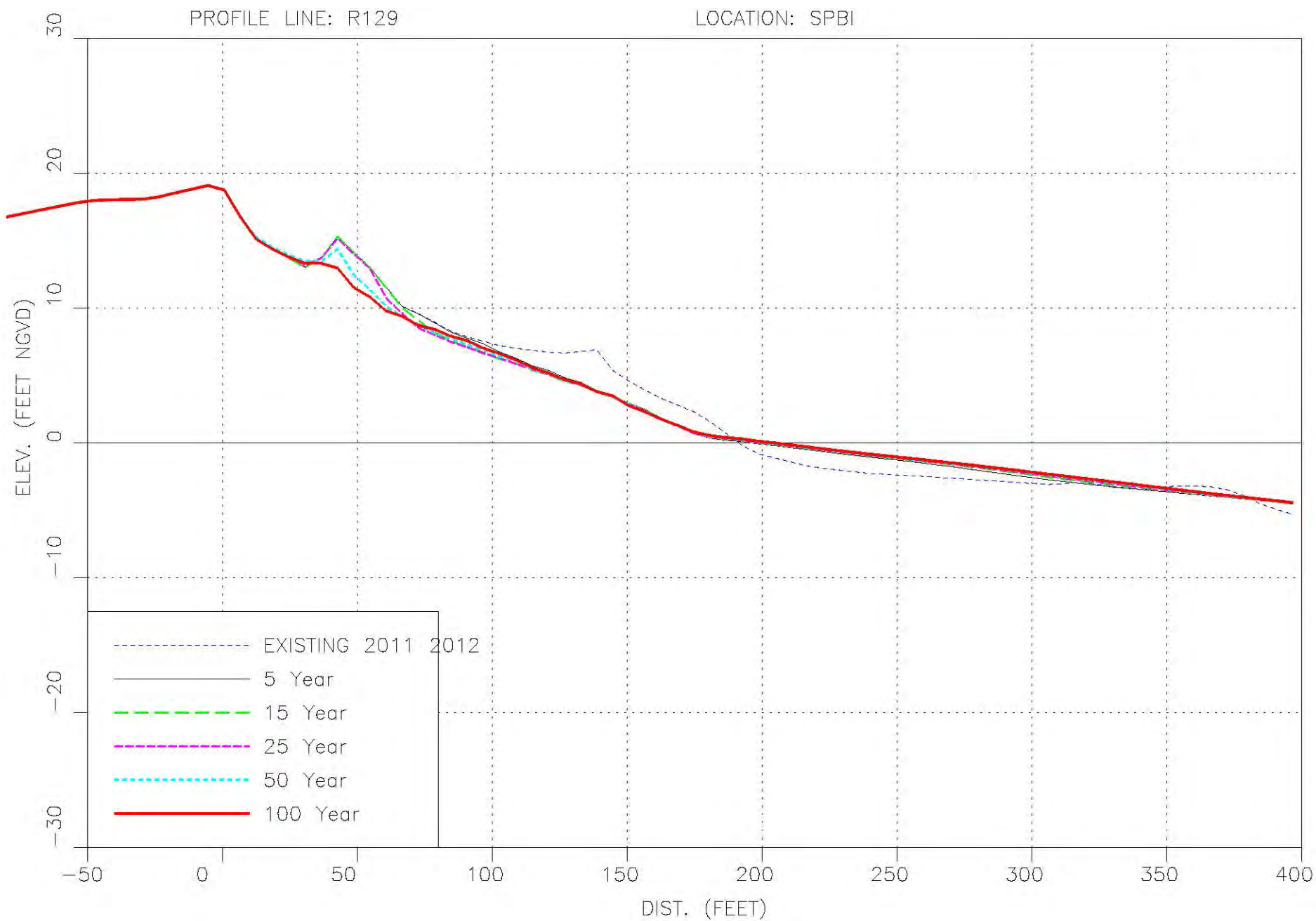


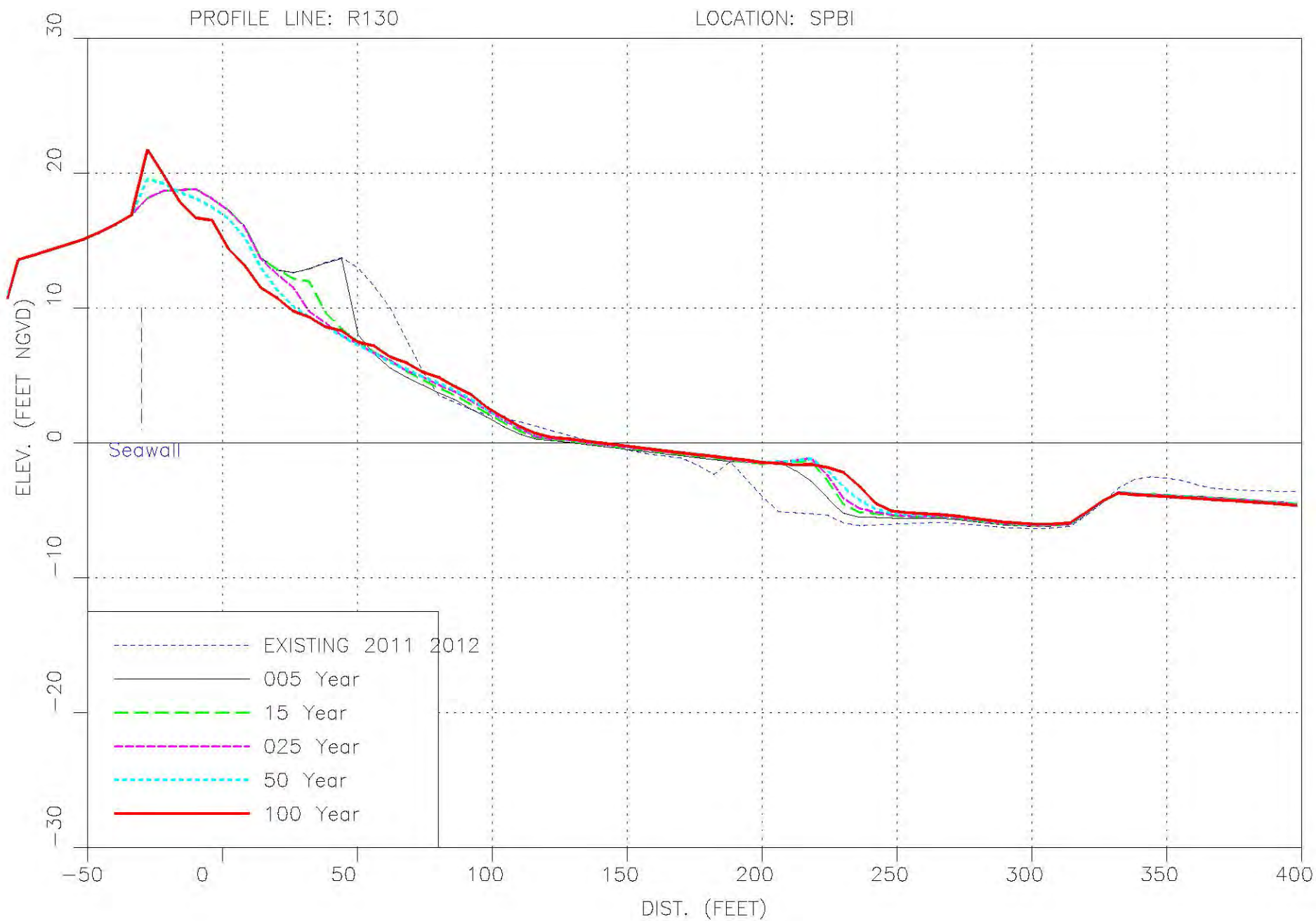
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SBEACH ANALYSIS REPORT

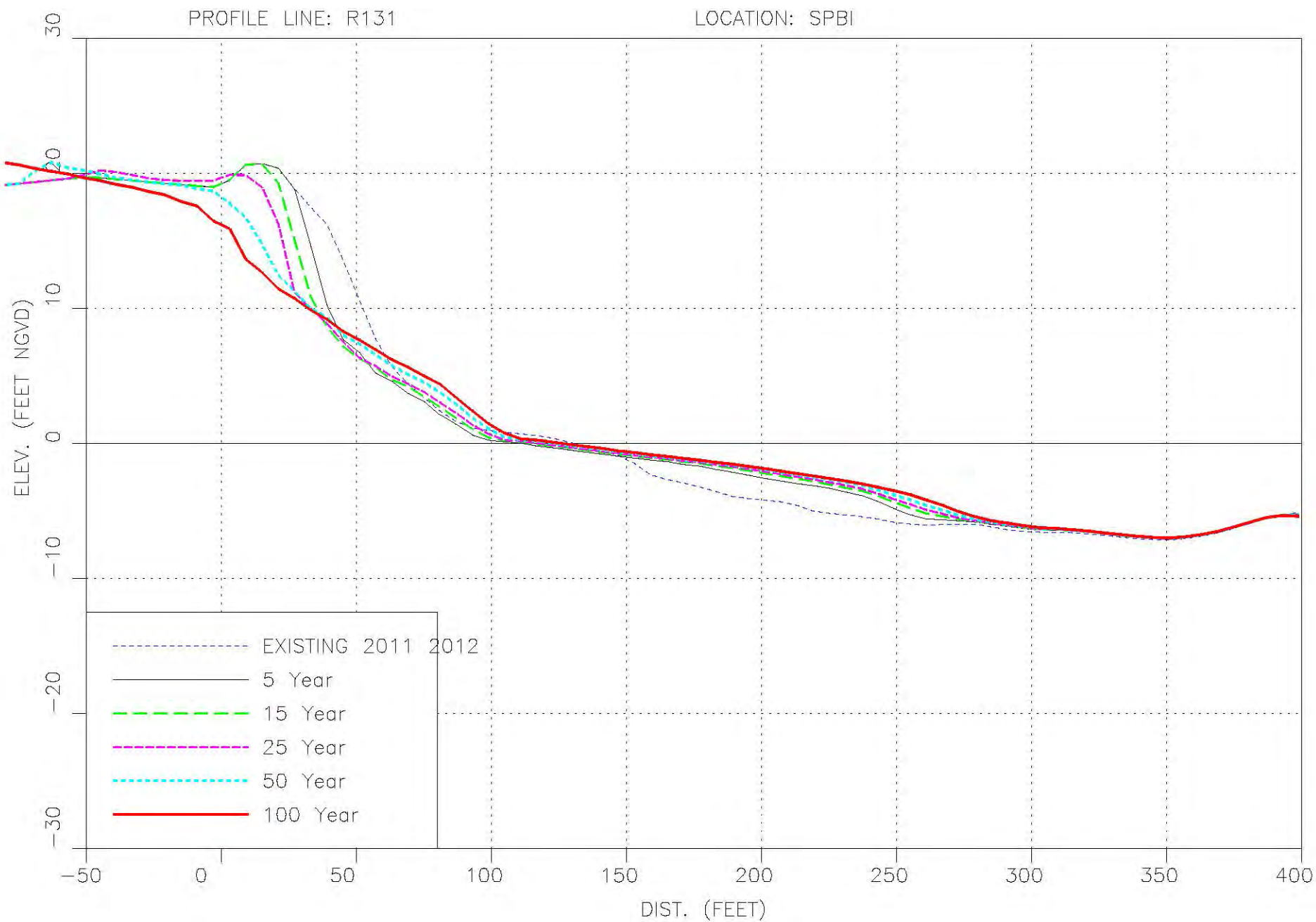
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SBEACH MODEL RESULTS

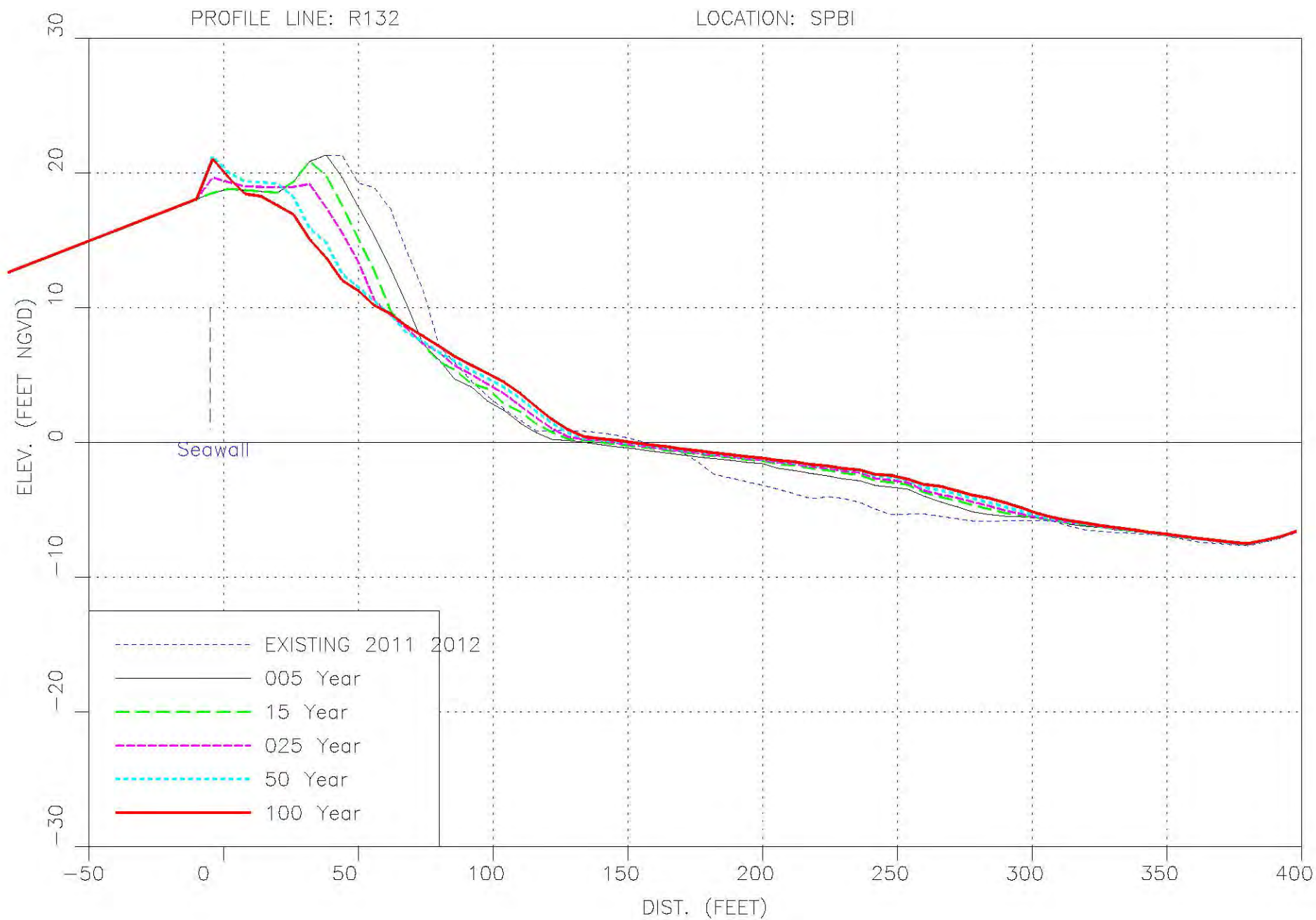
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SBEACH ANALYSIS REPORT

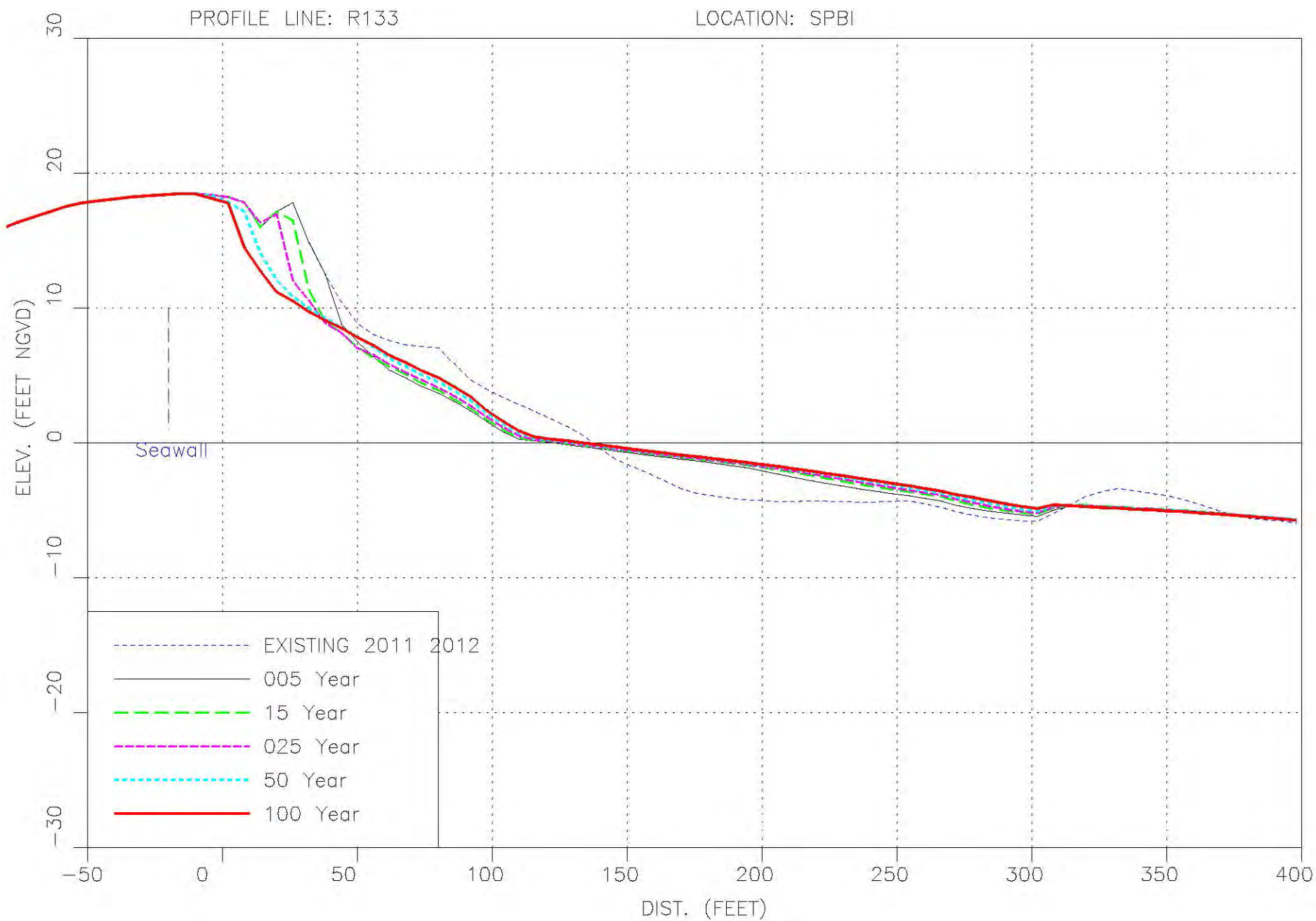
ATTACHMENT C-1
EXISTING CONDITIONS (2011/2012 SURVEY)

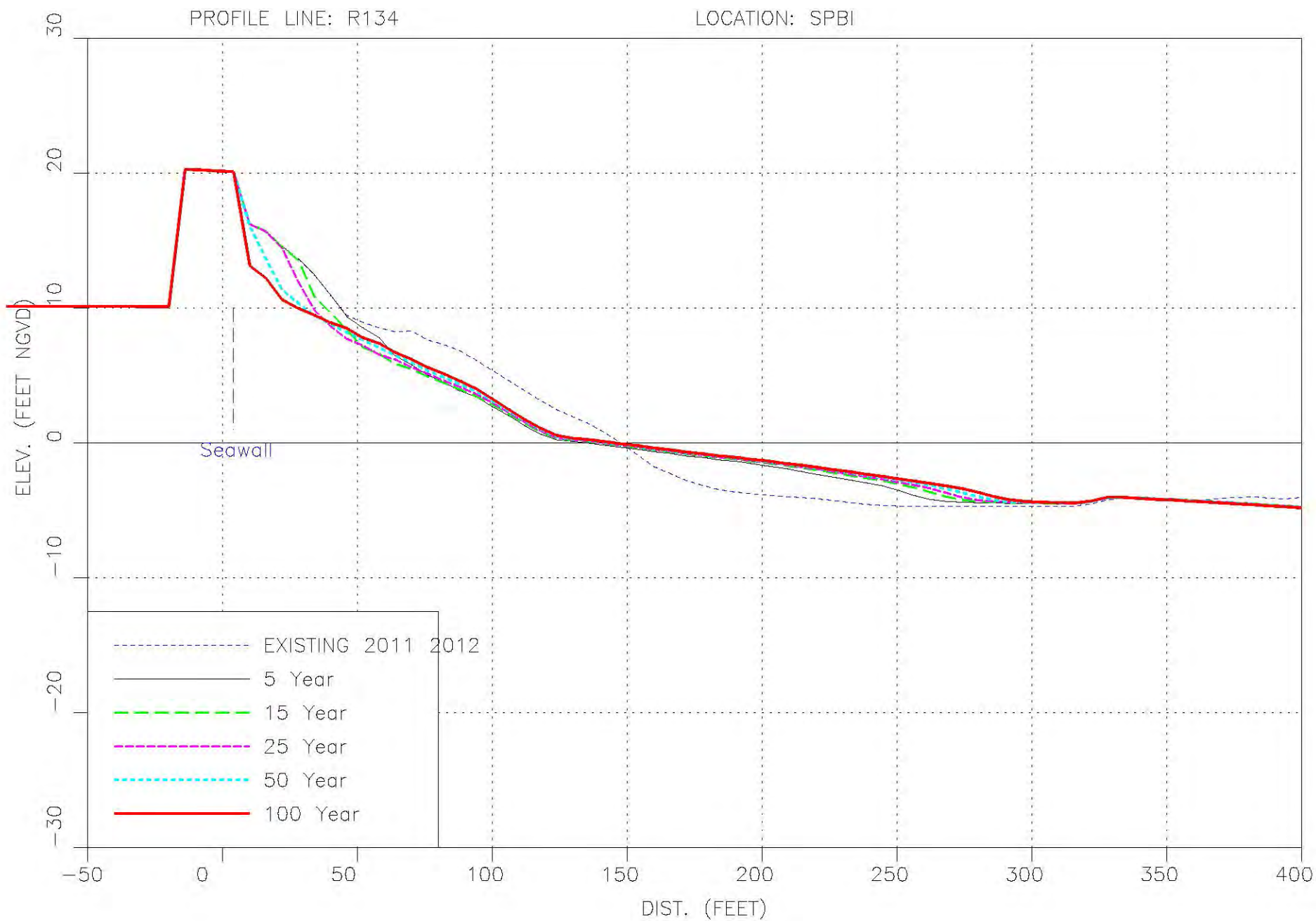


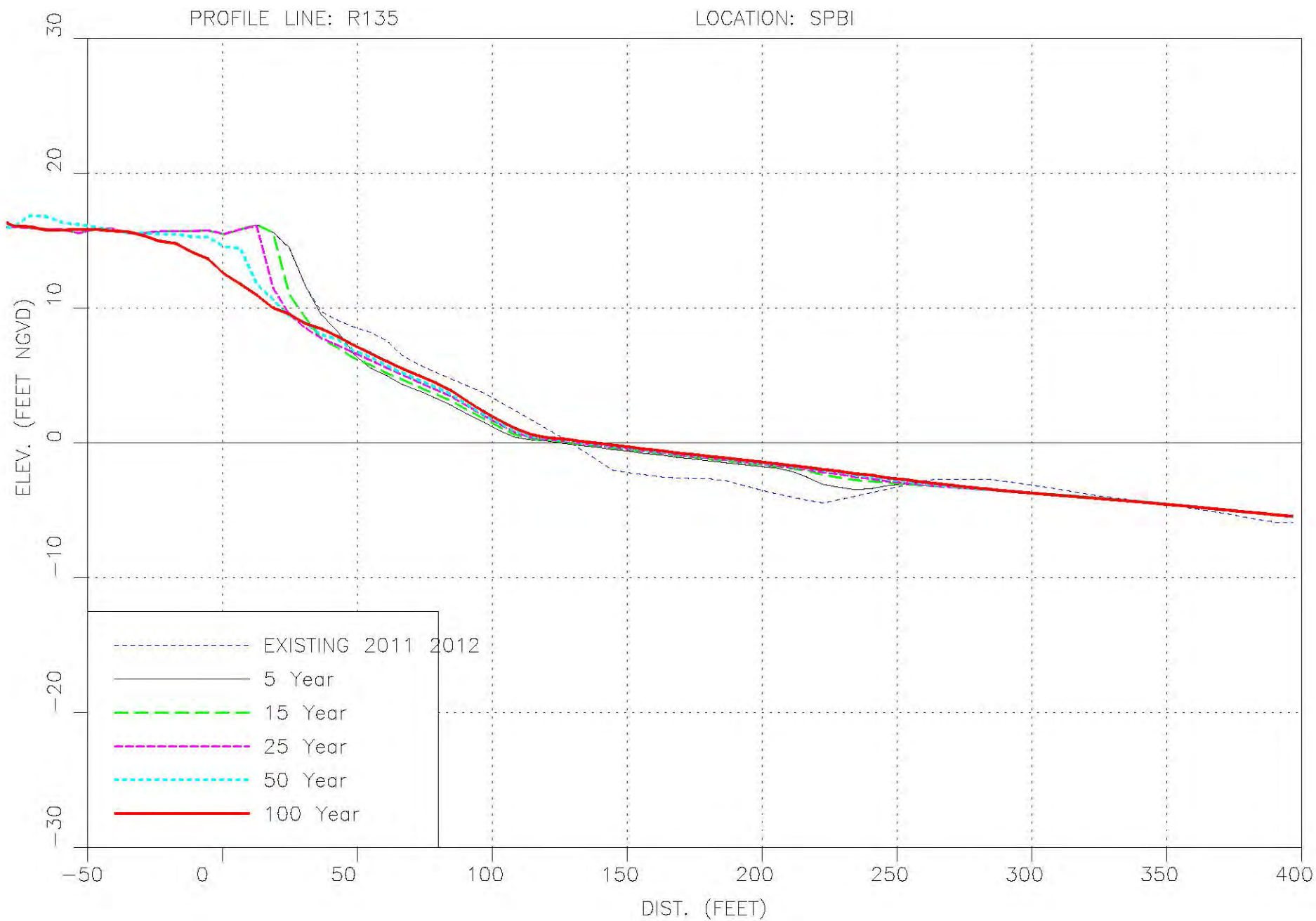


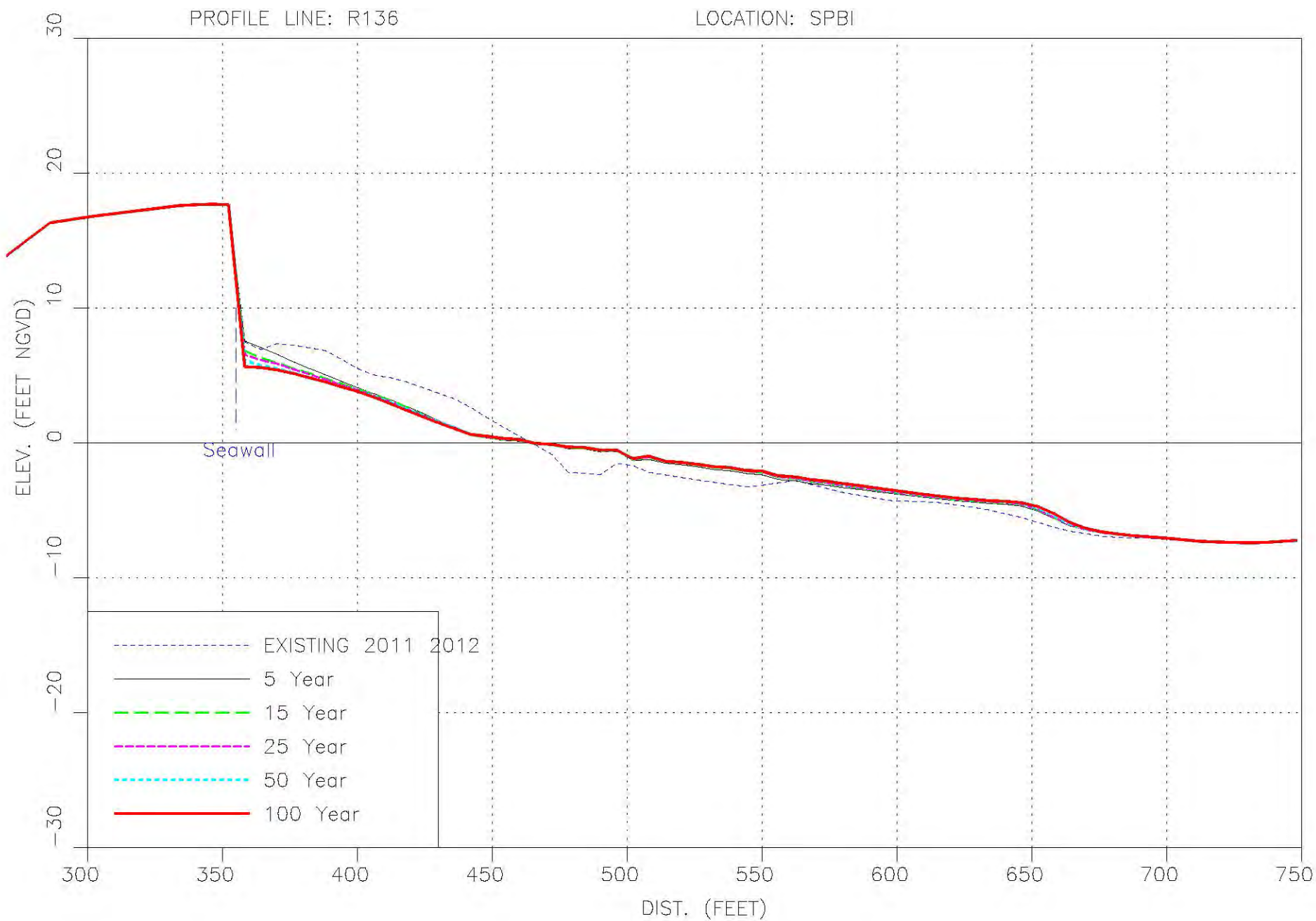


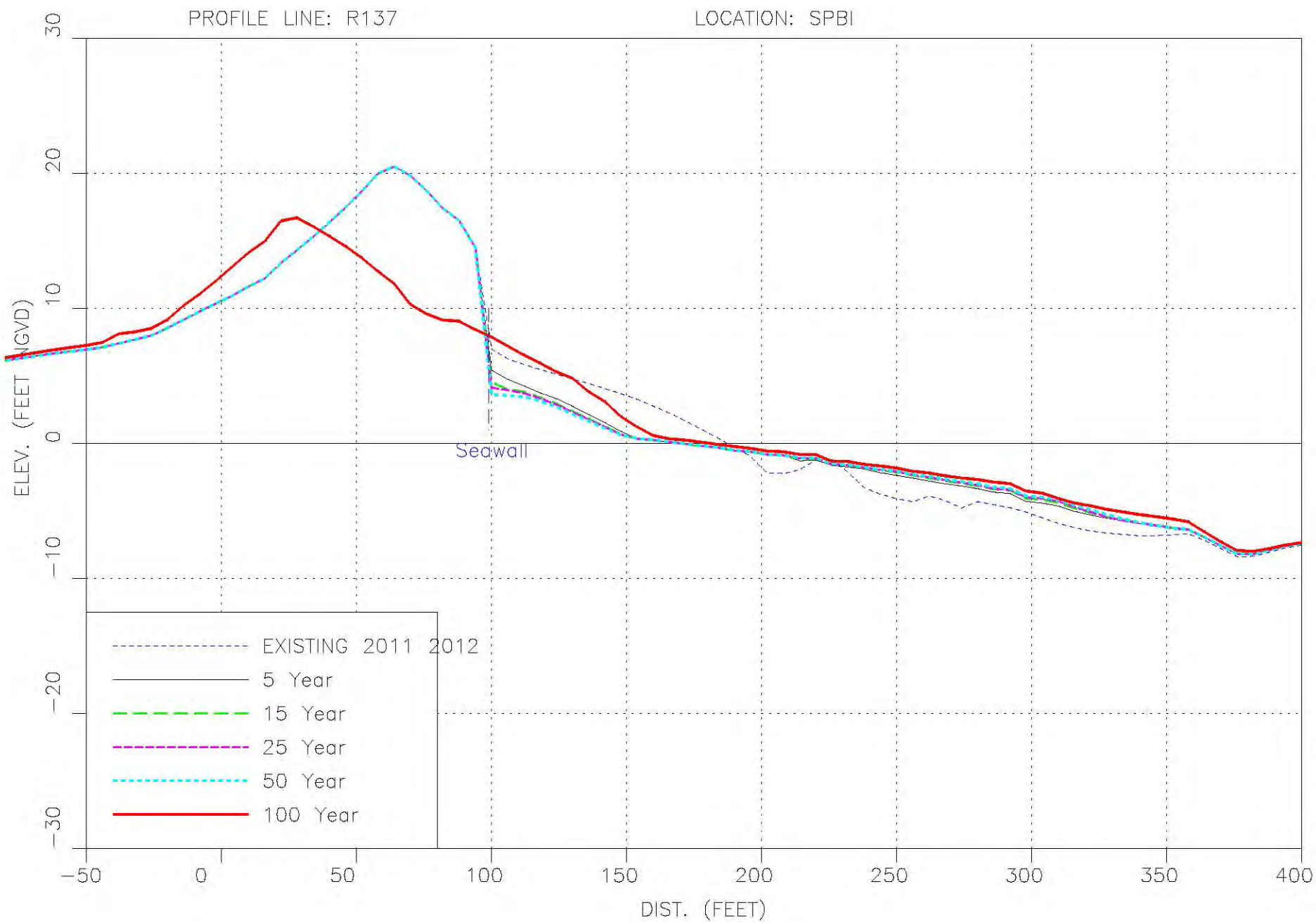


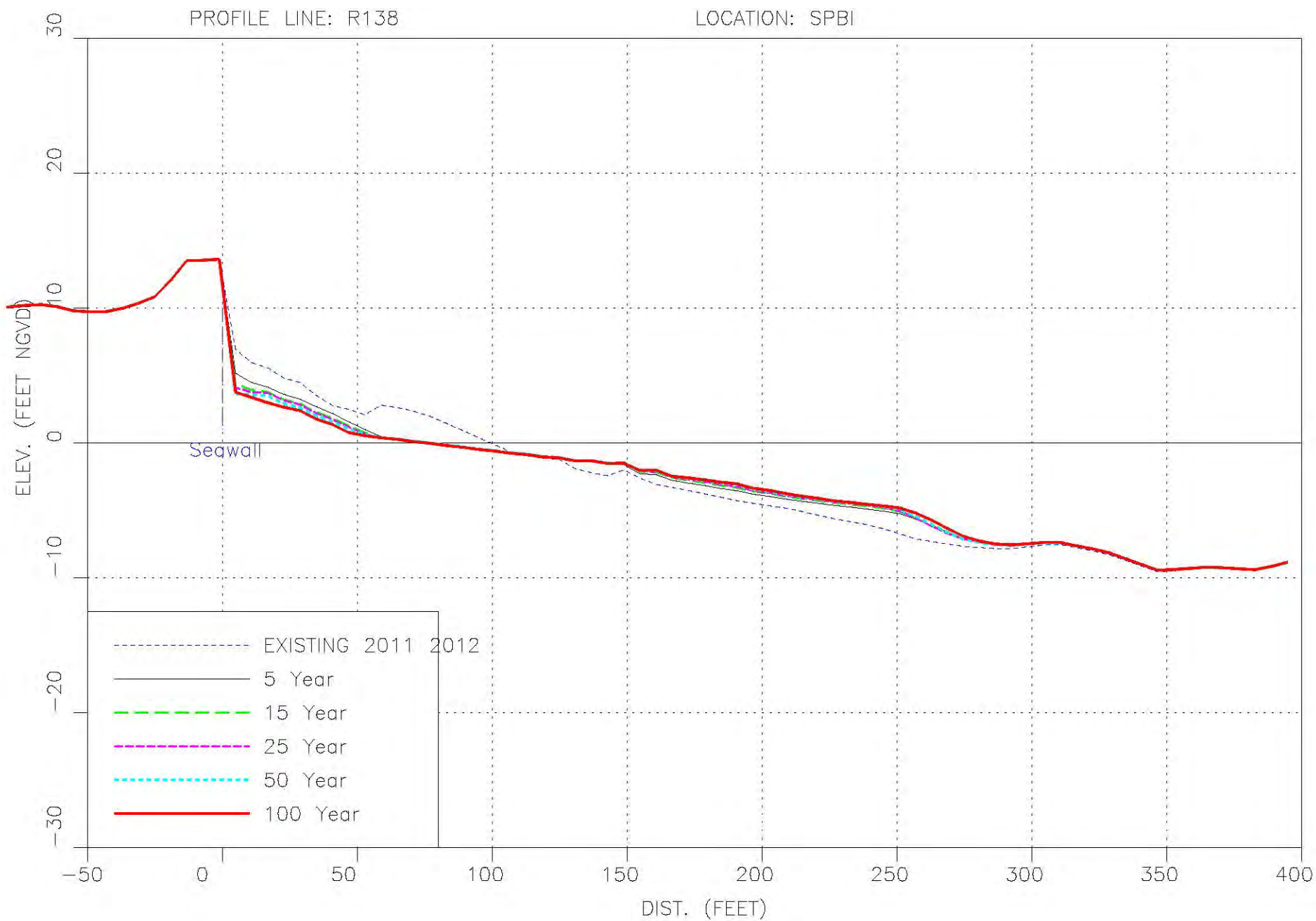






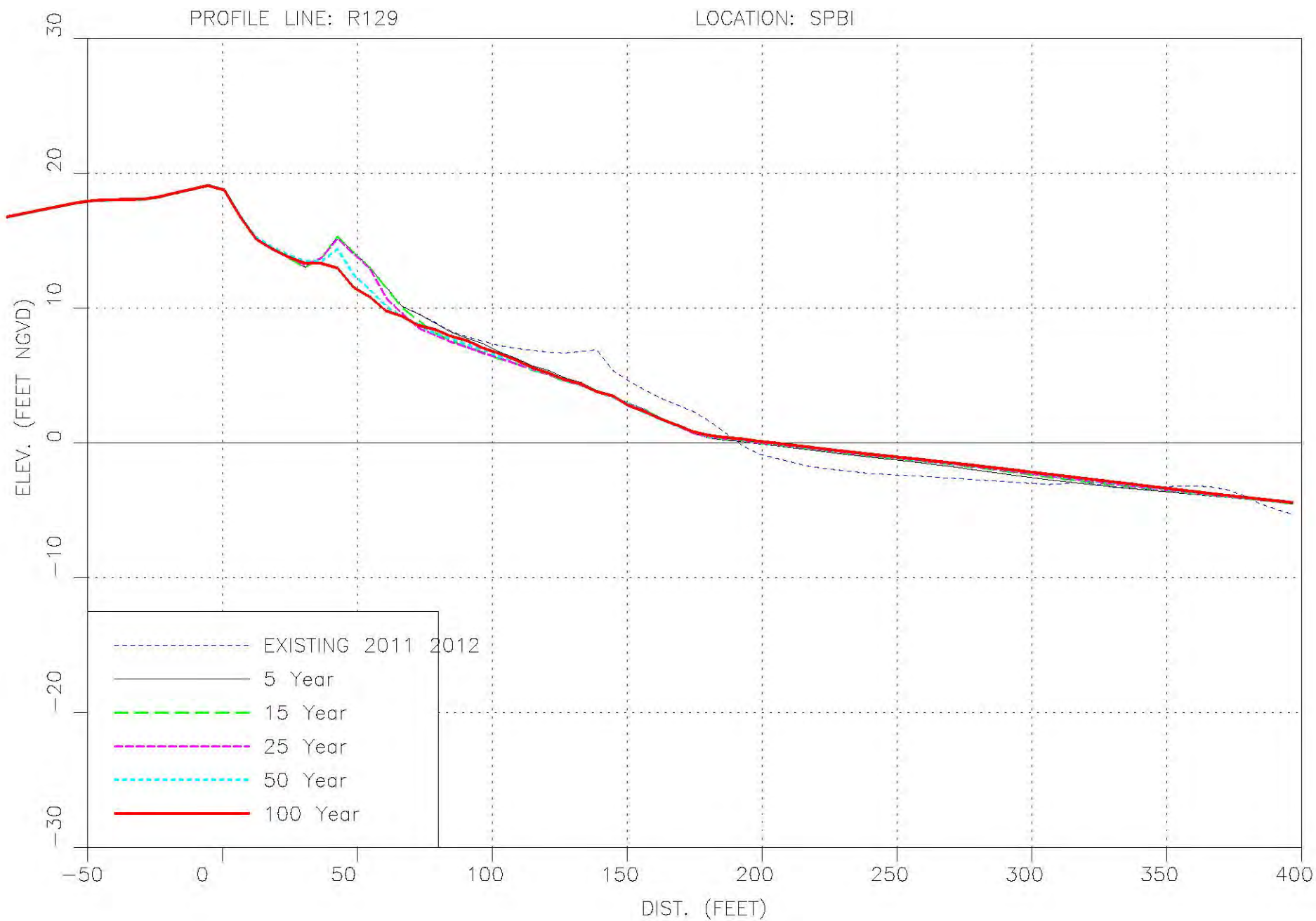


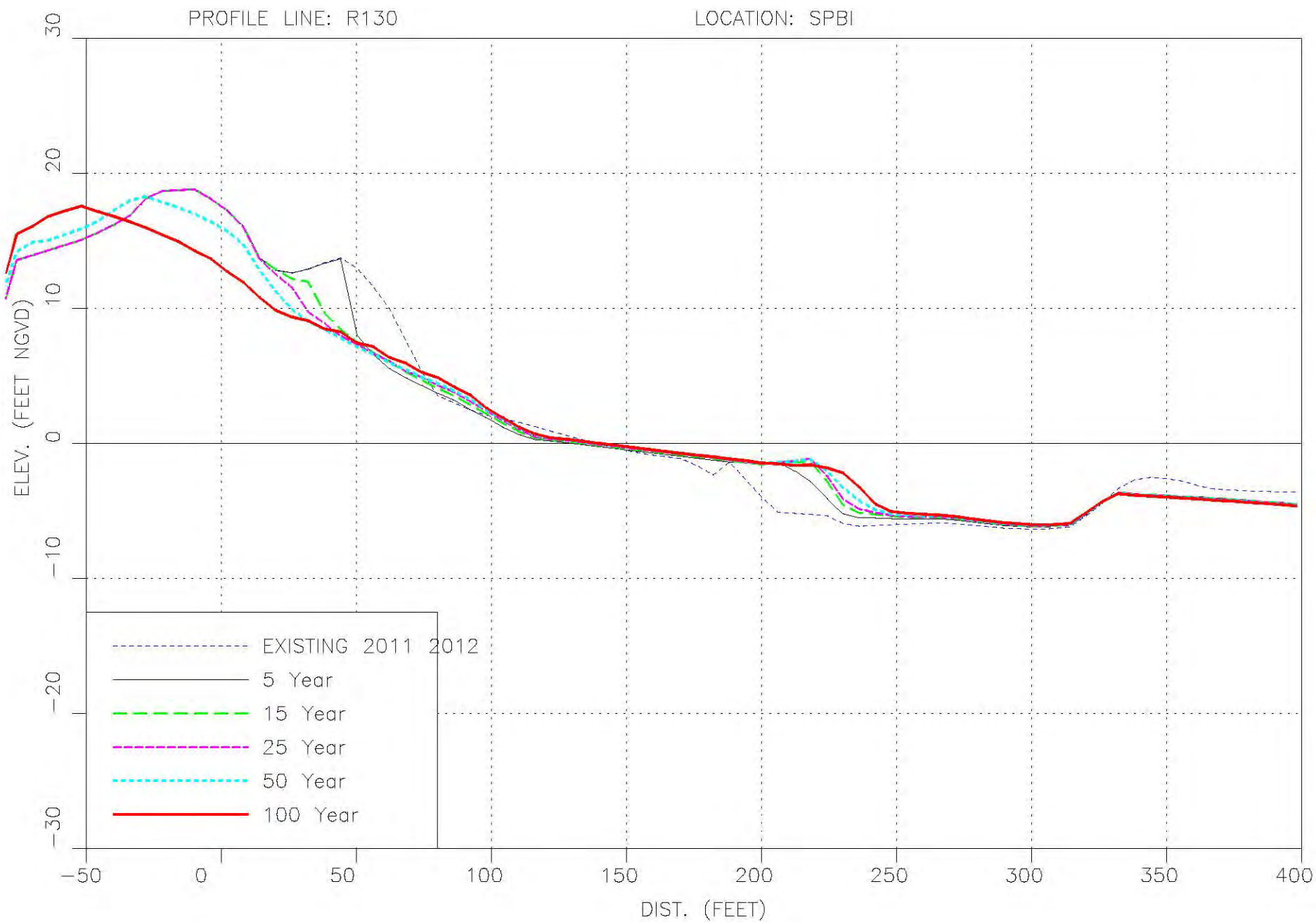


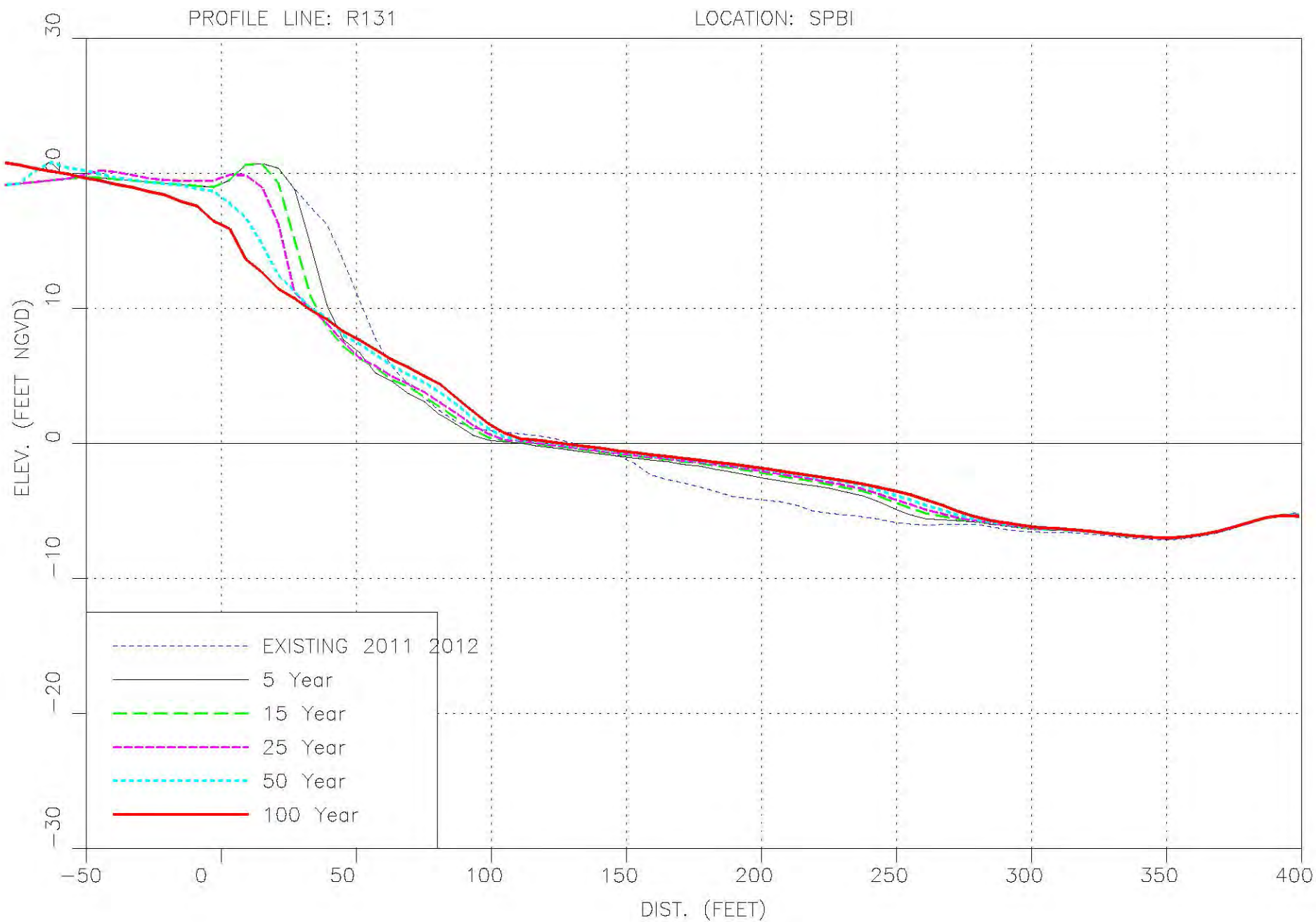


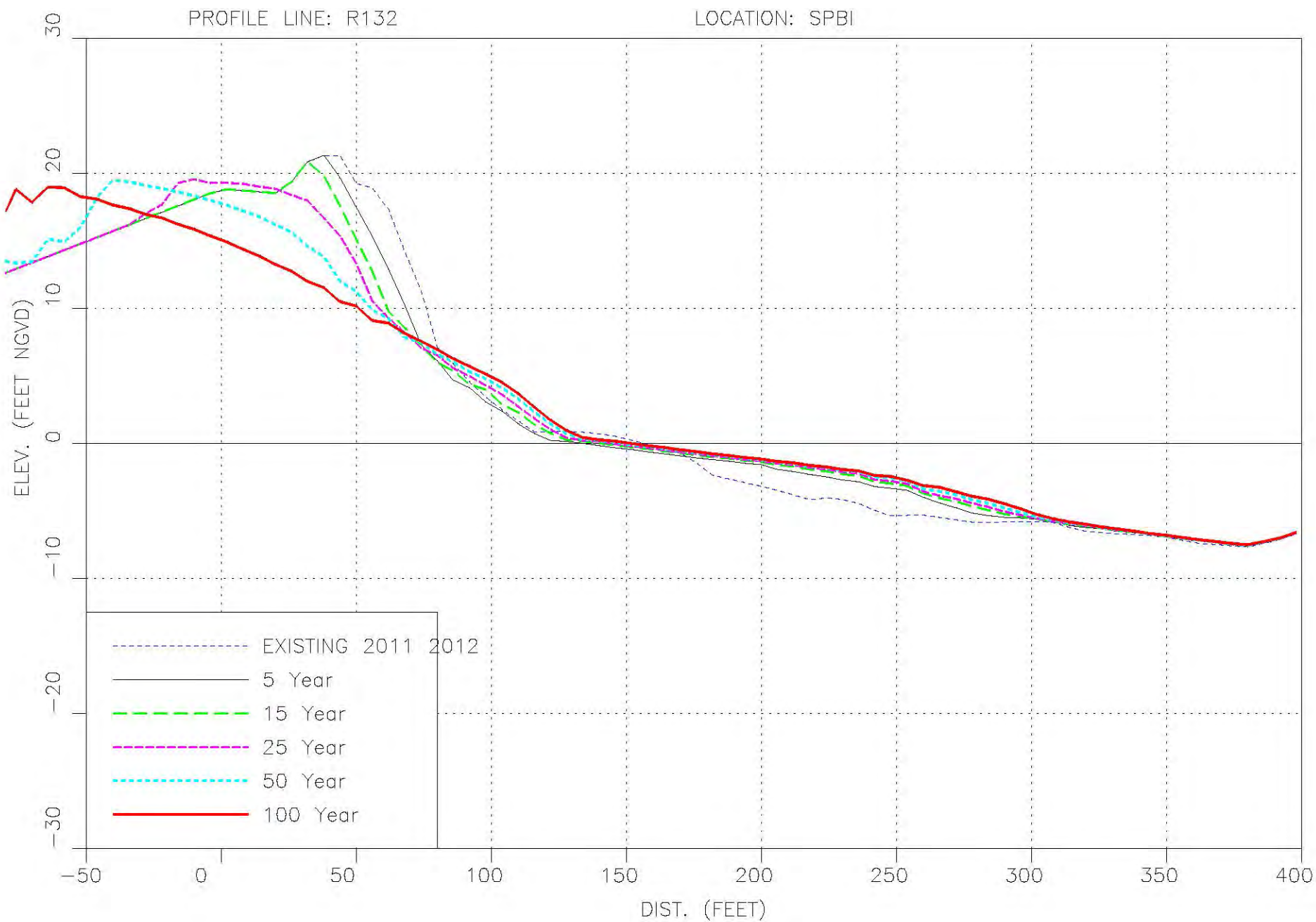
SUB-APPENDIX G-1
SBEACH ANALYSIS REPORT

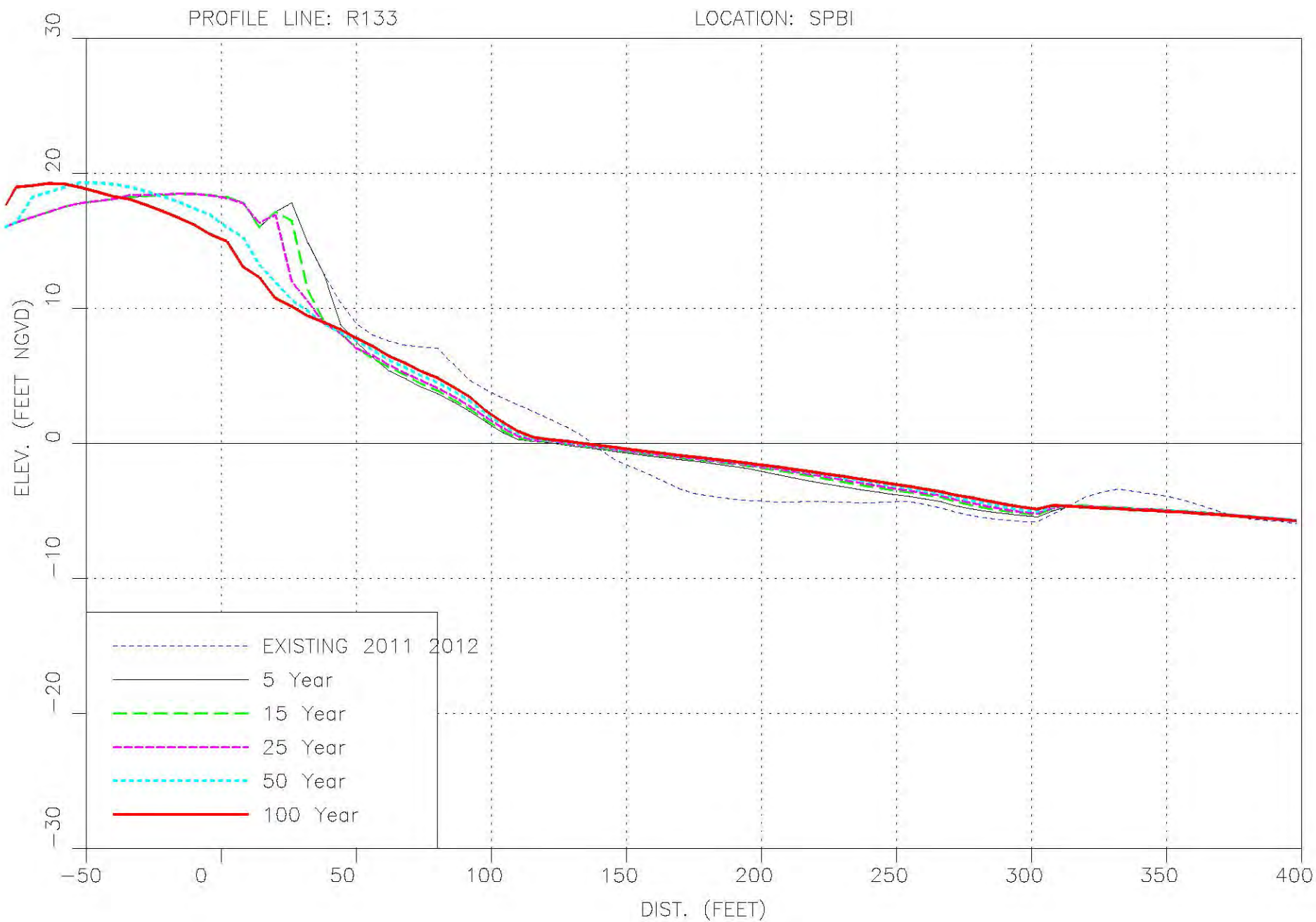
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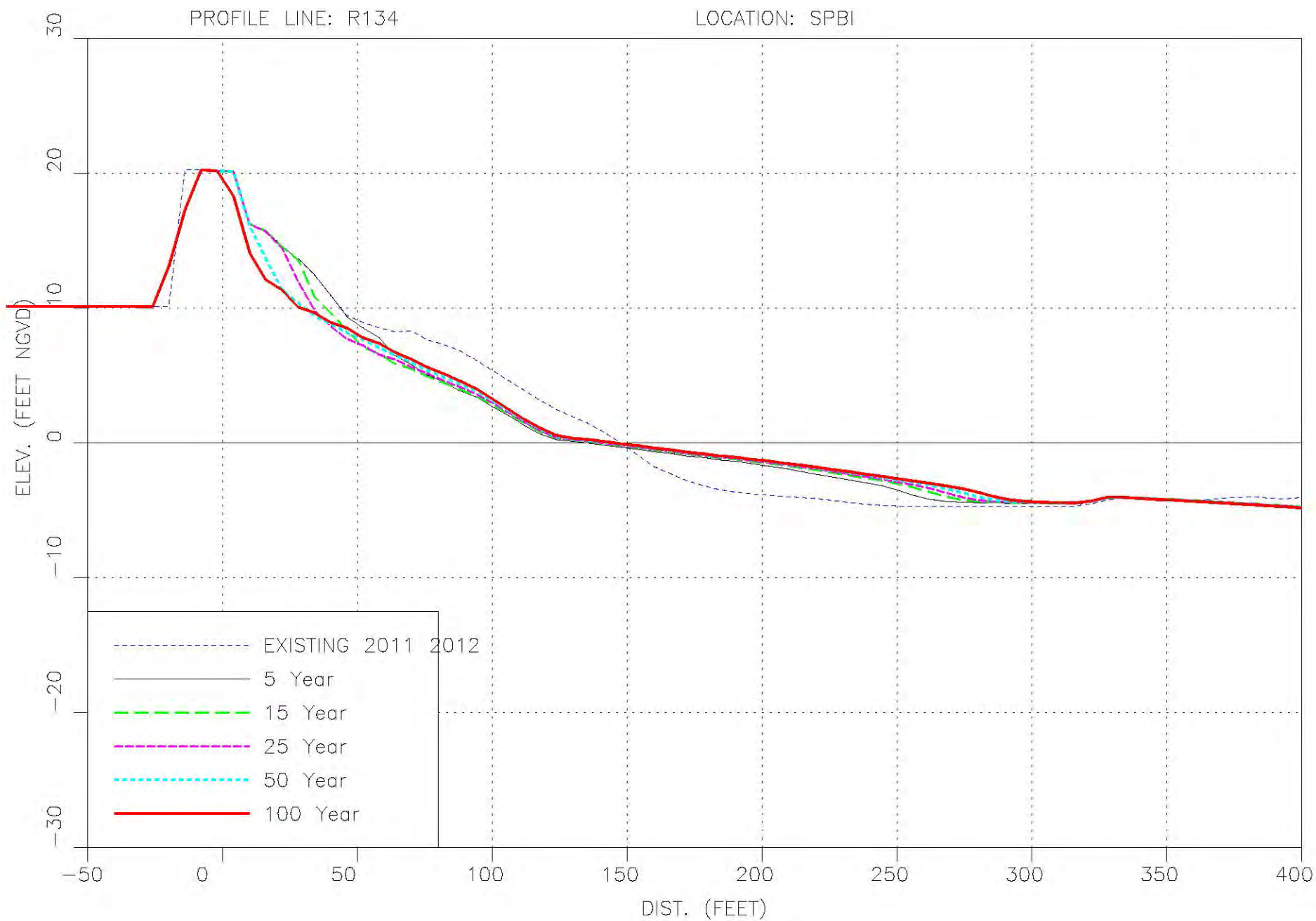


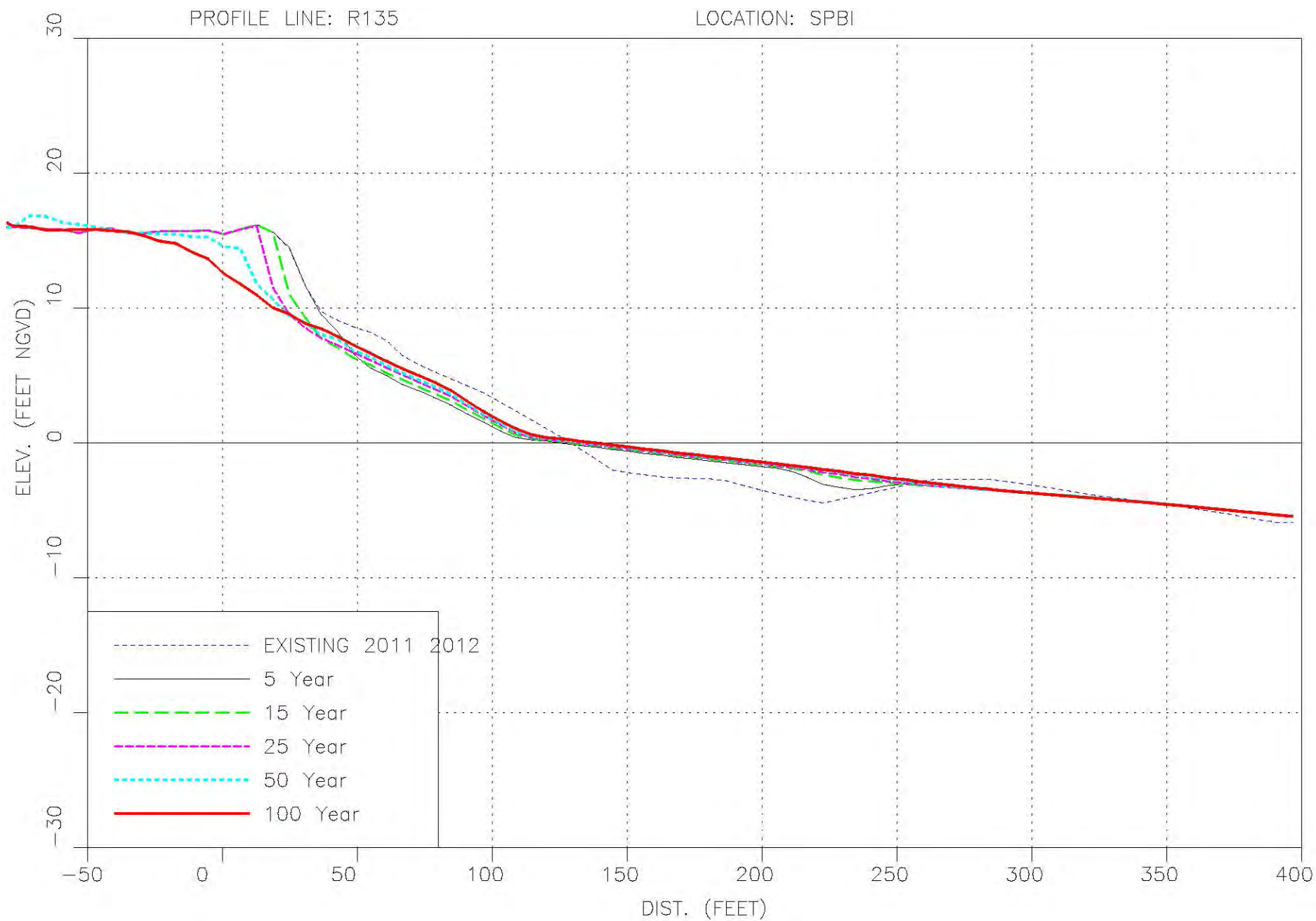


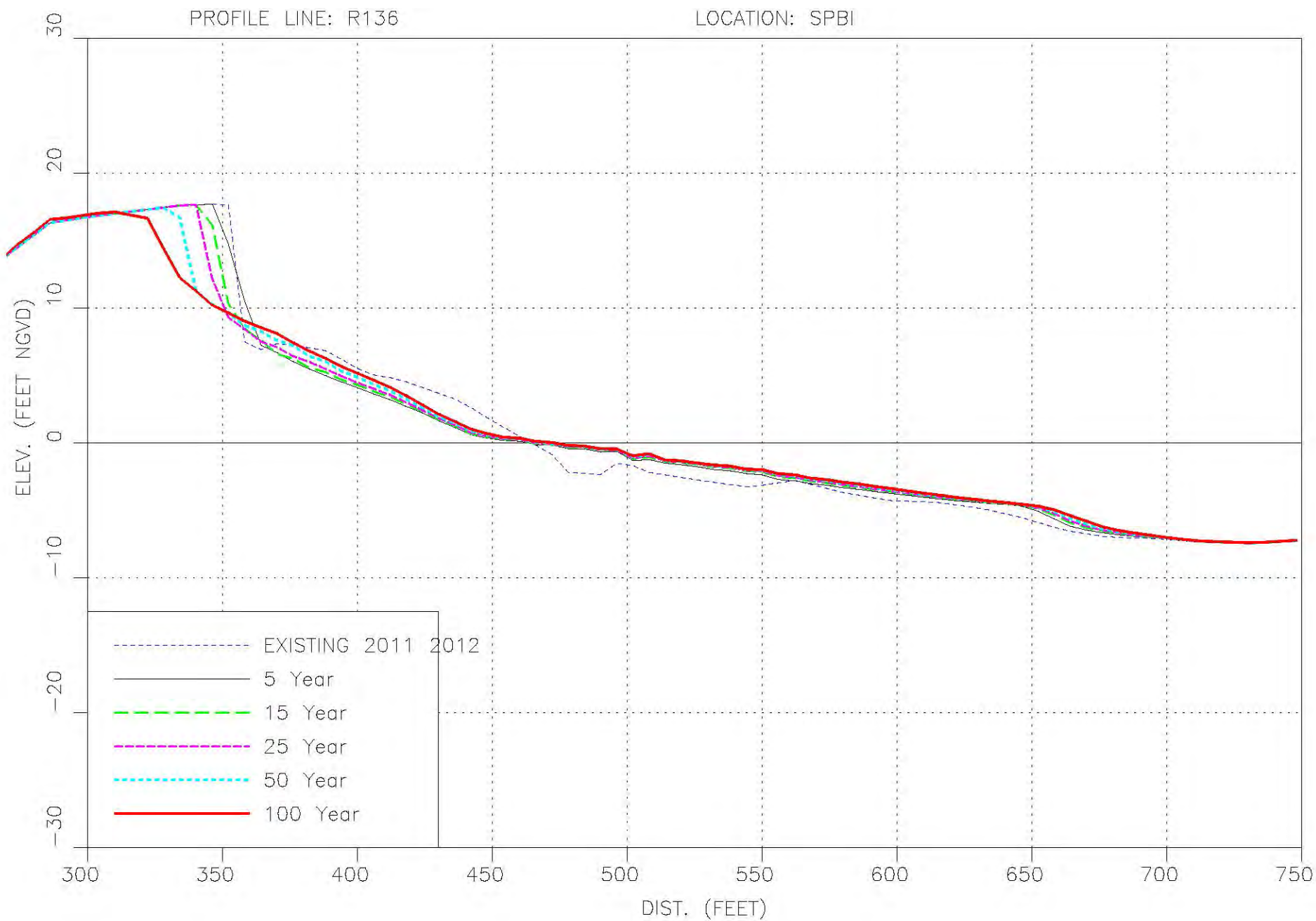


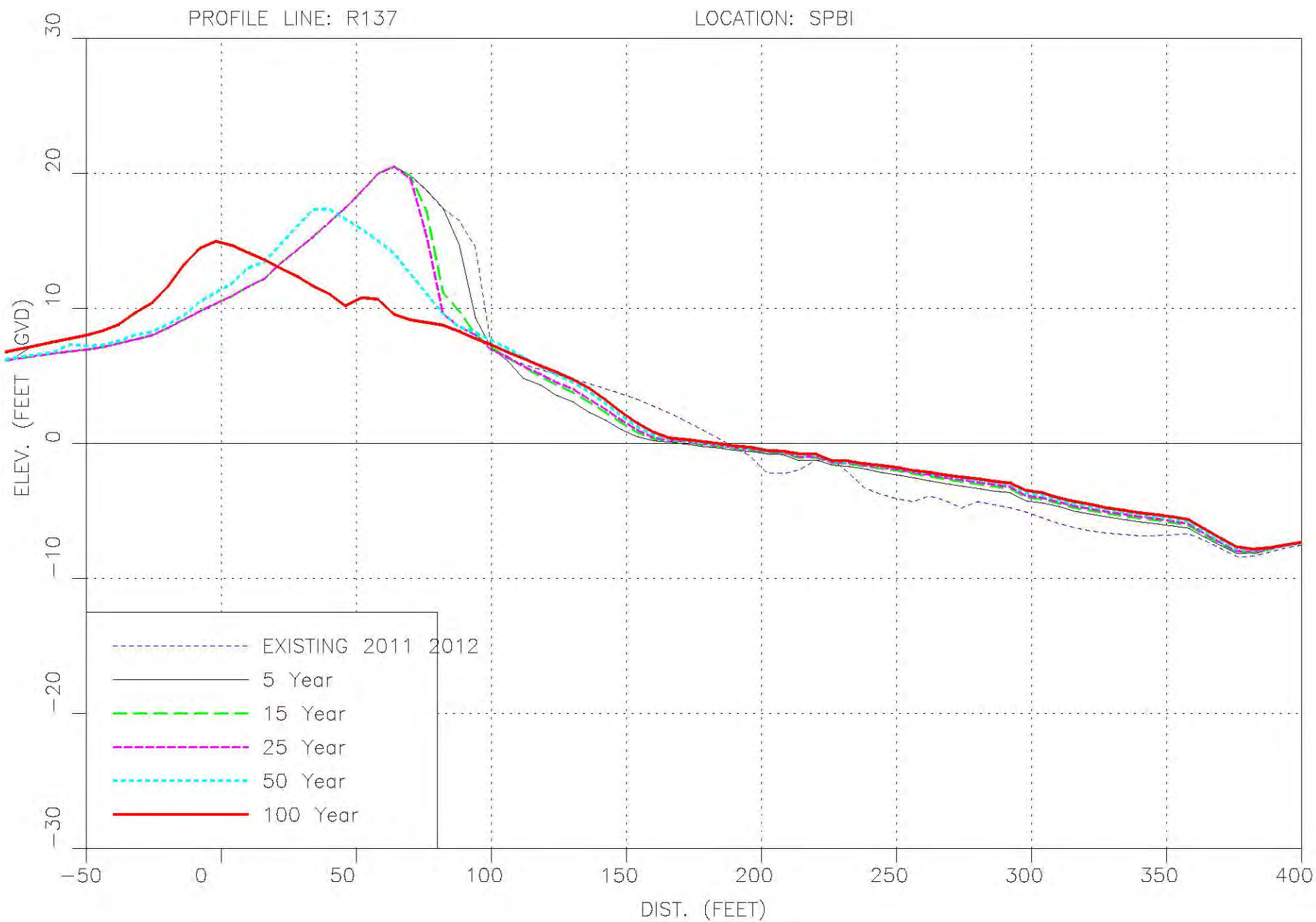


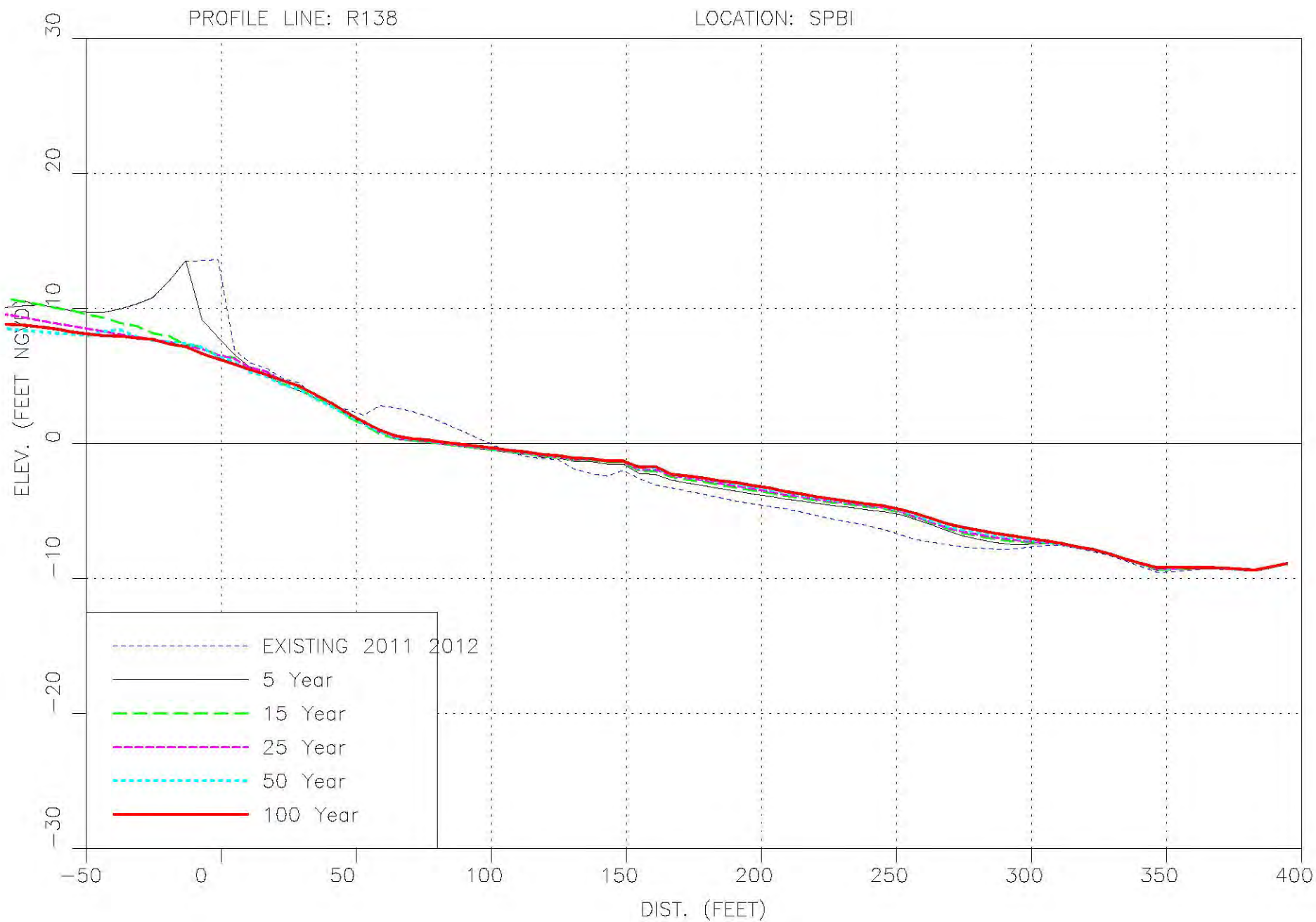






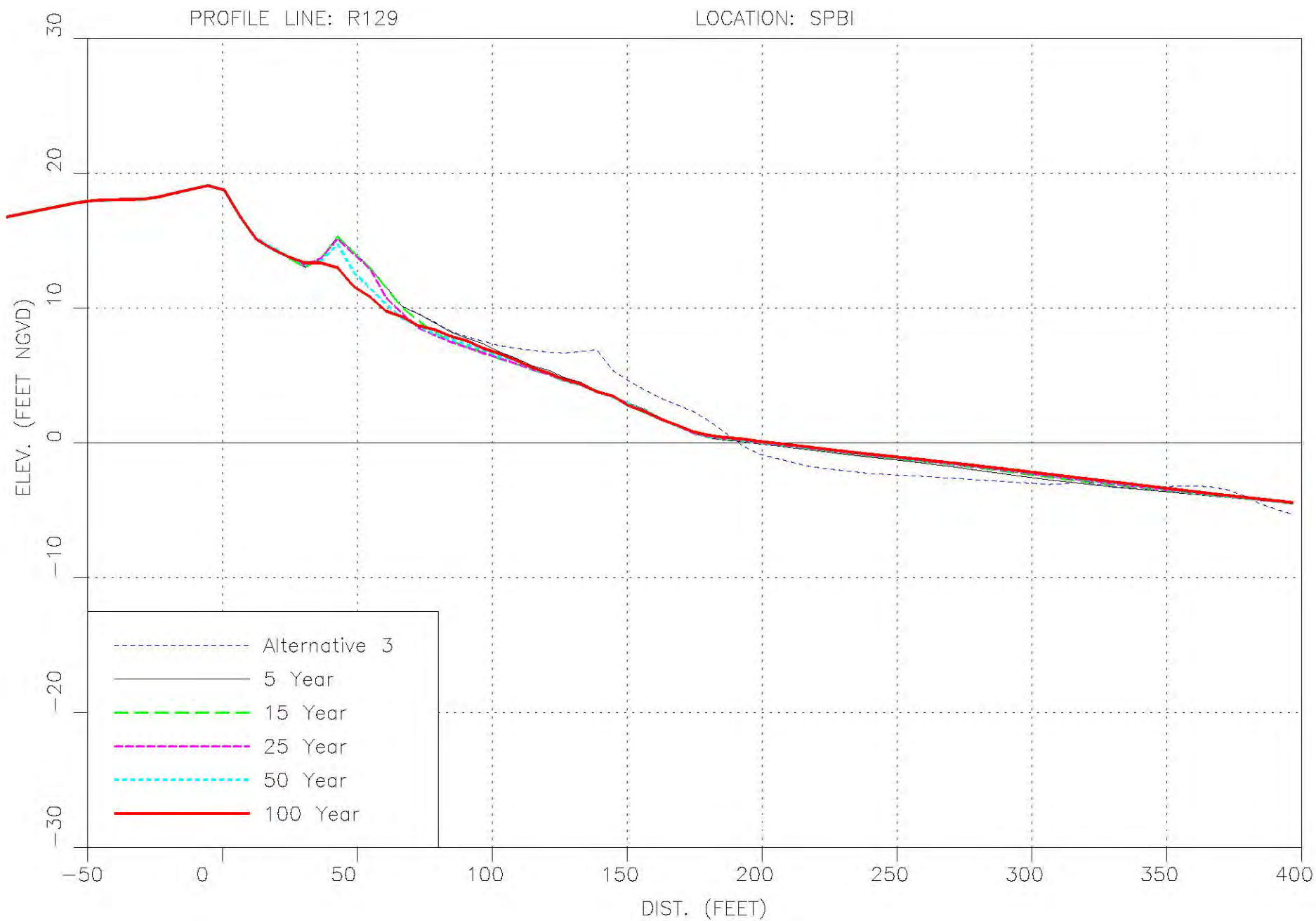


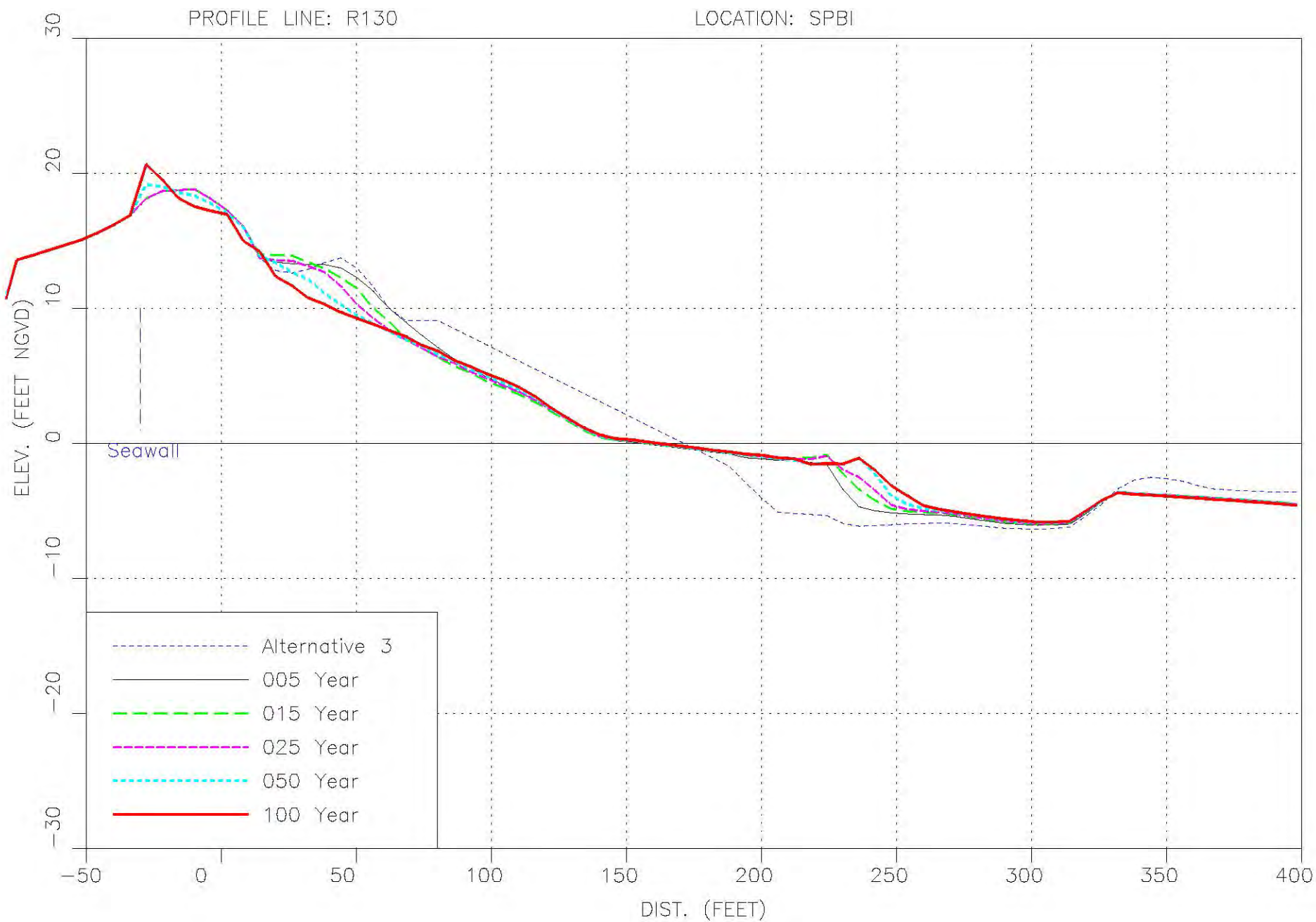


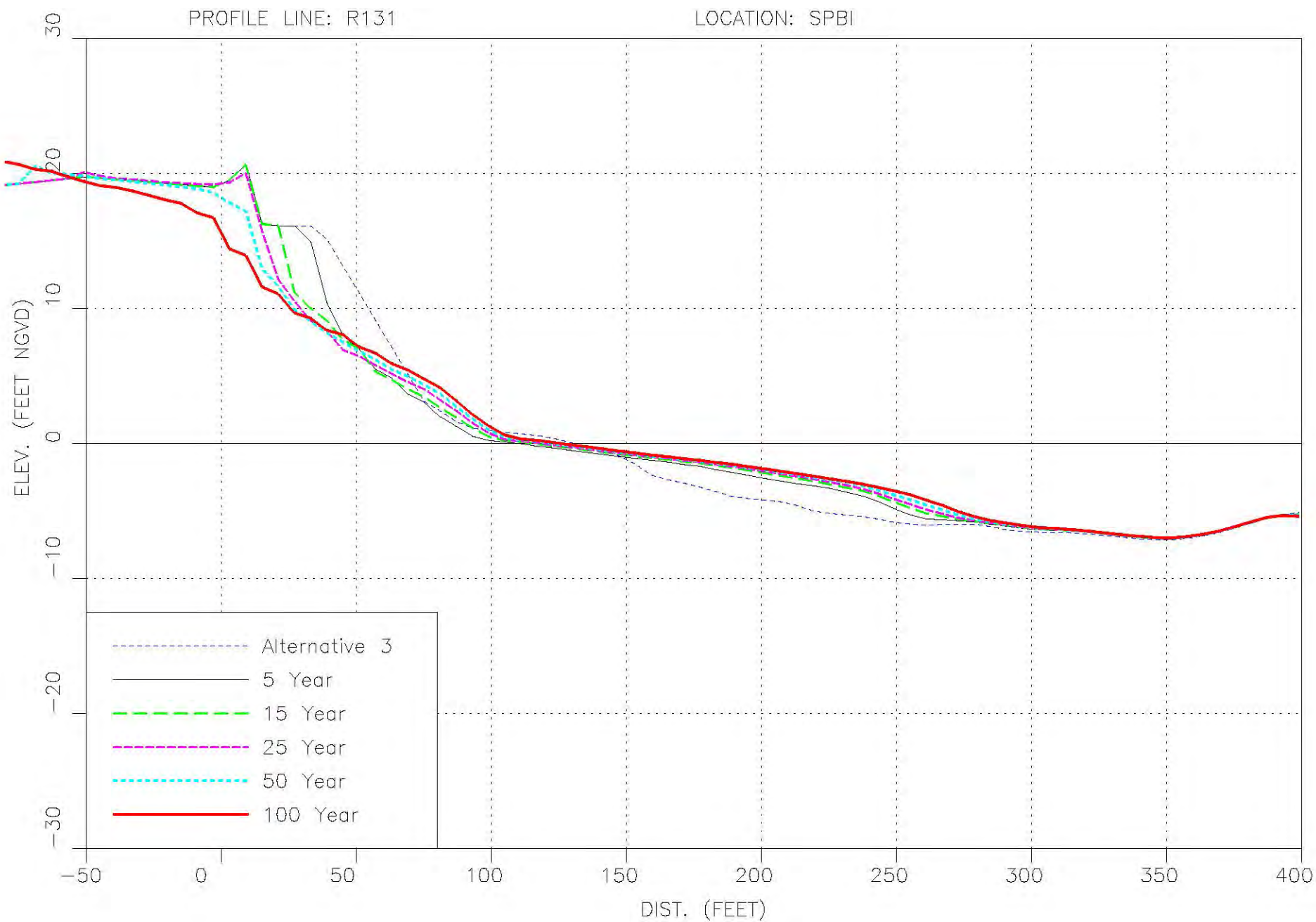


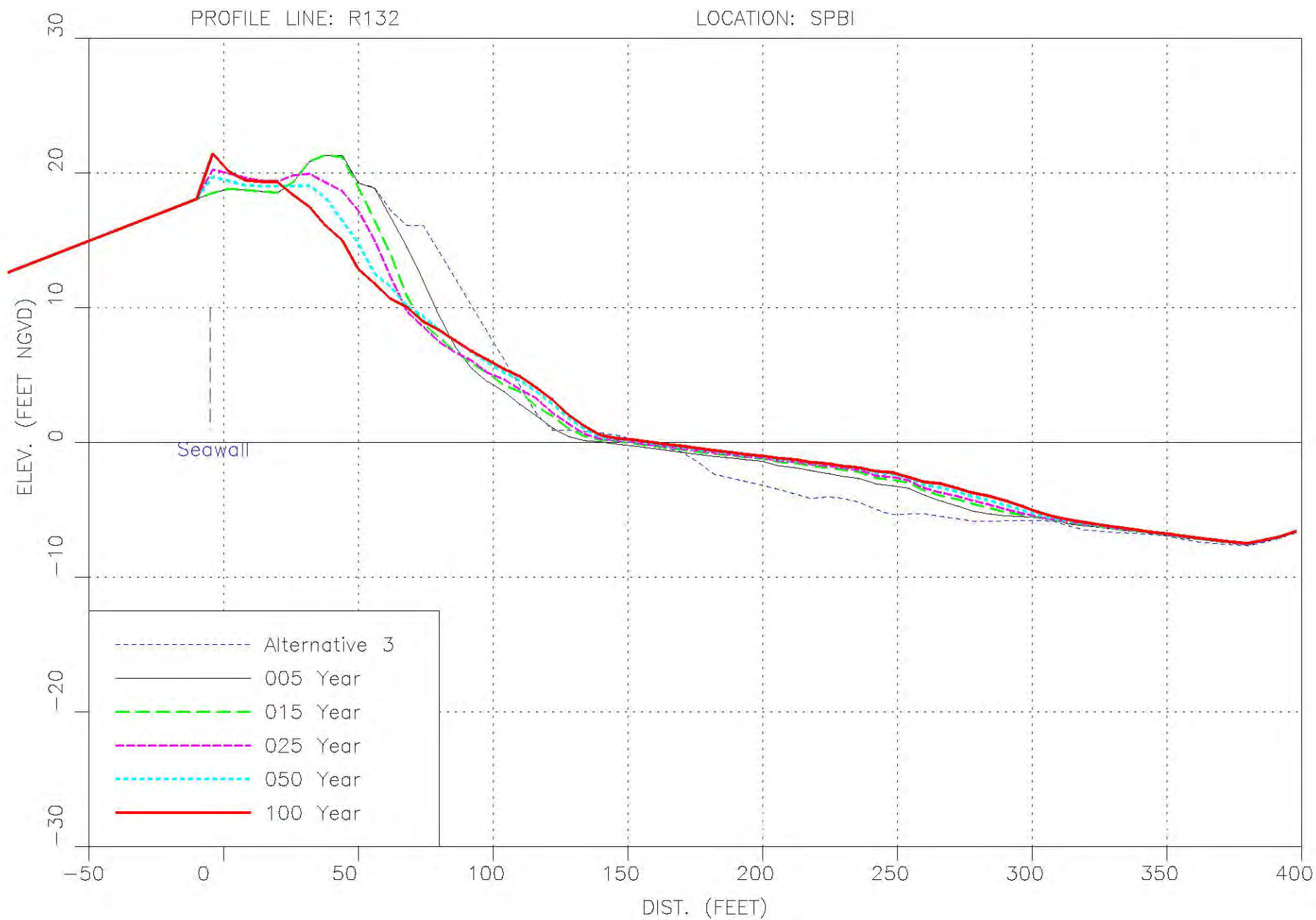
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SBEACH ANALYSIS REPORT

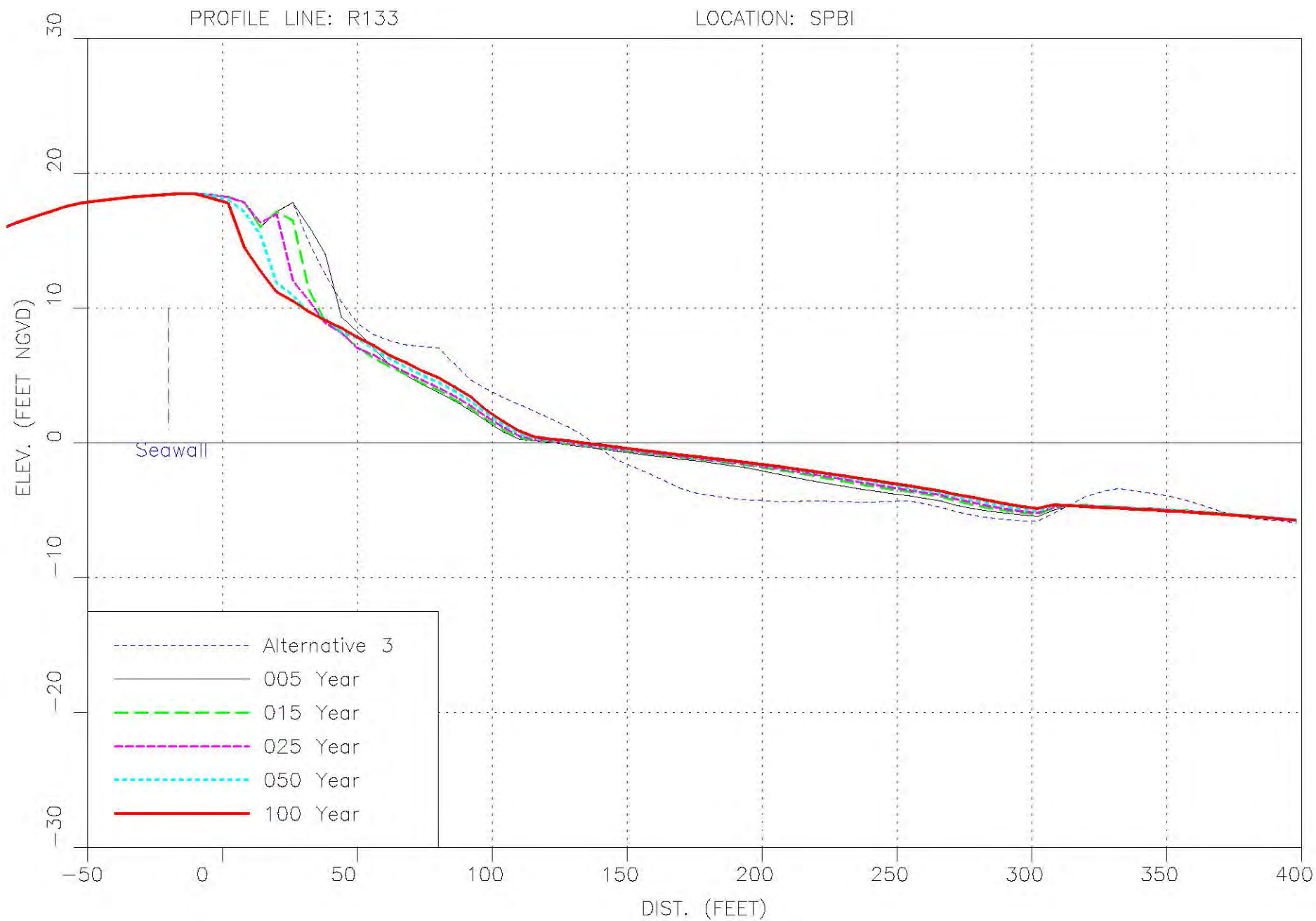
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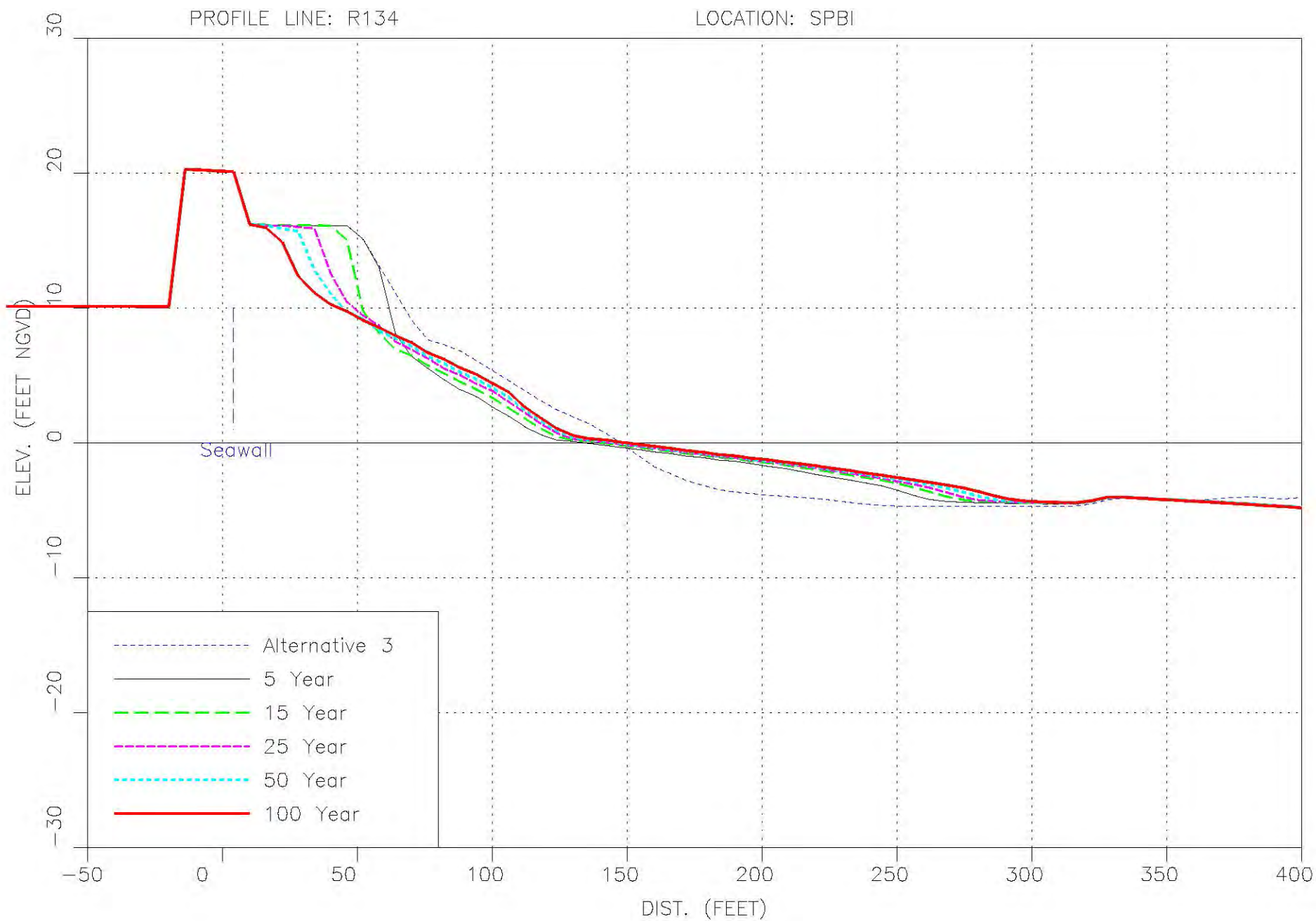


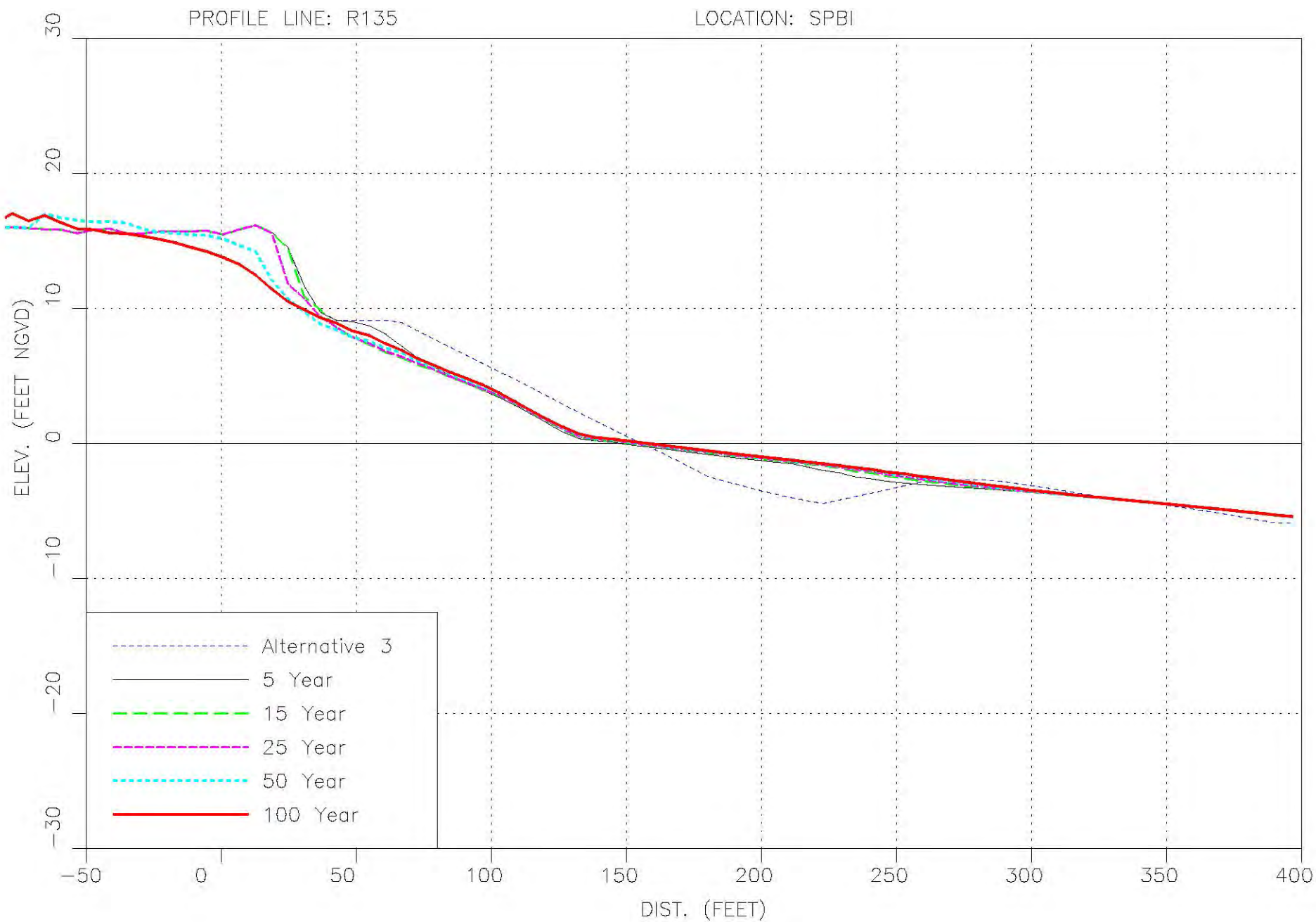


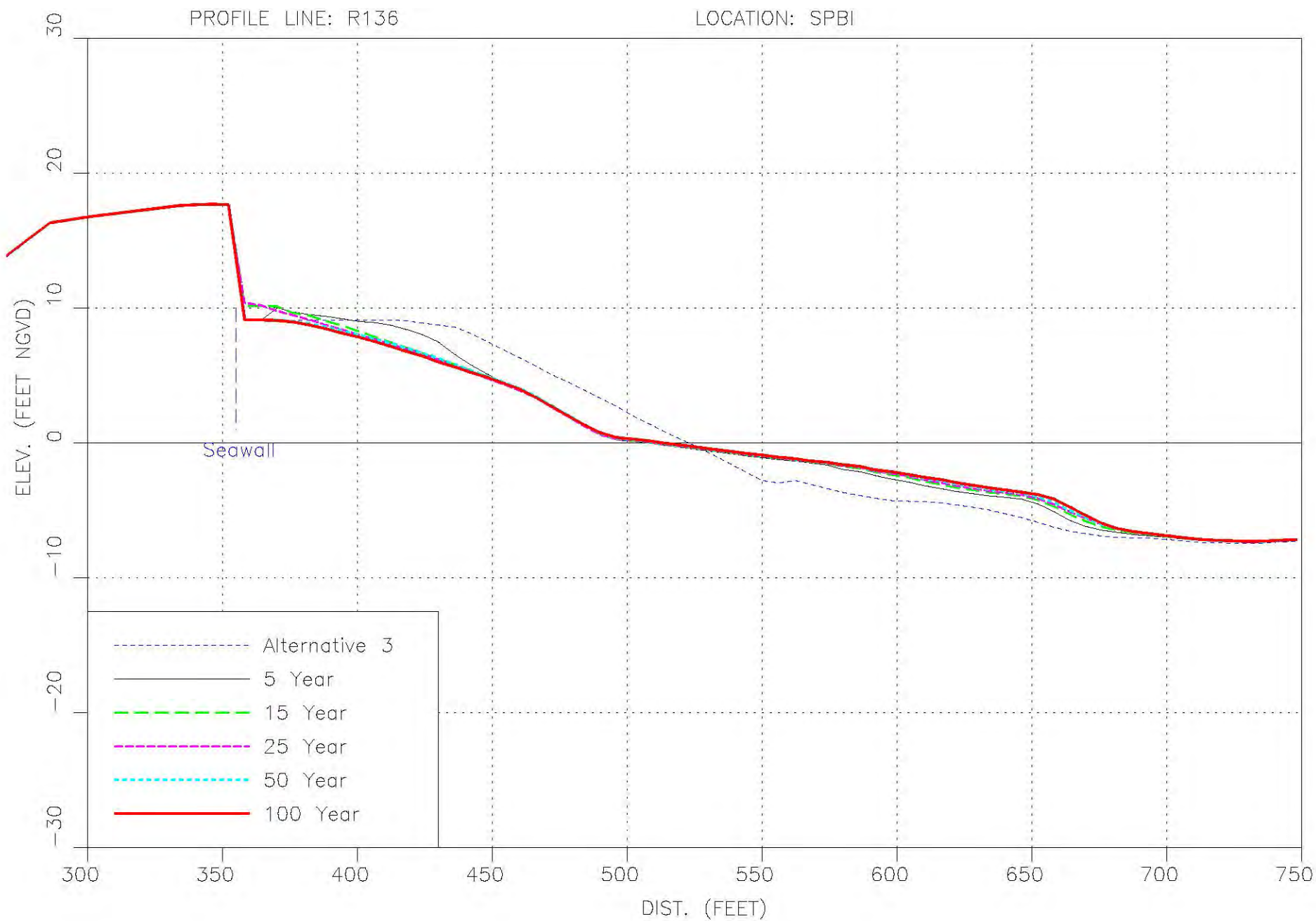


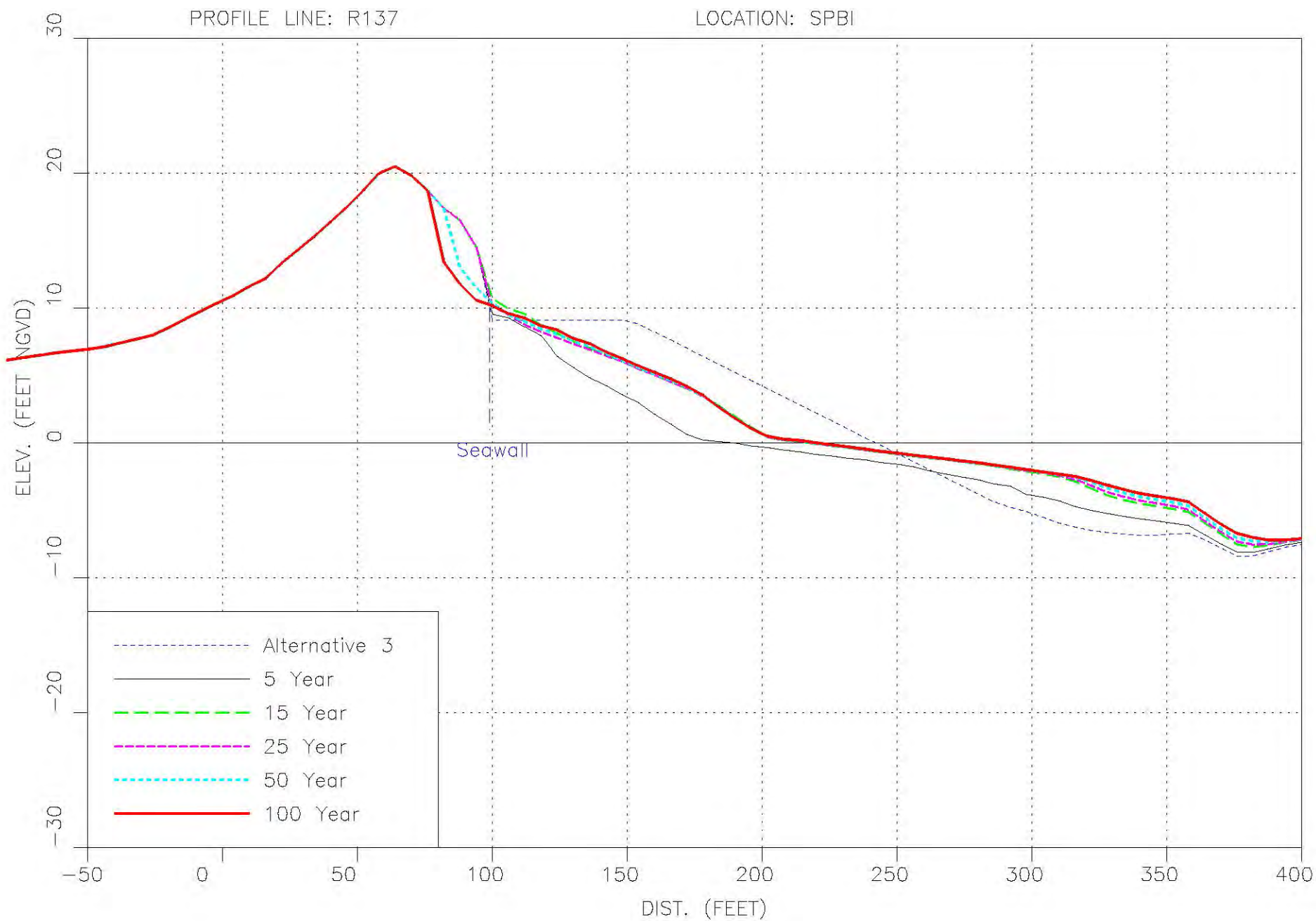


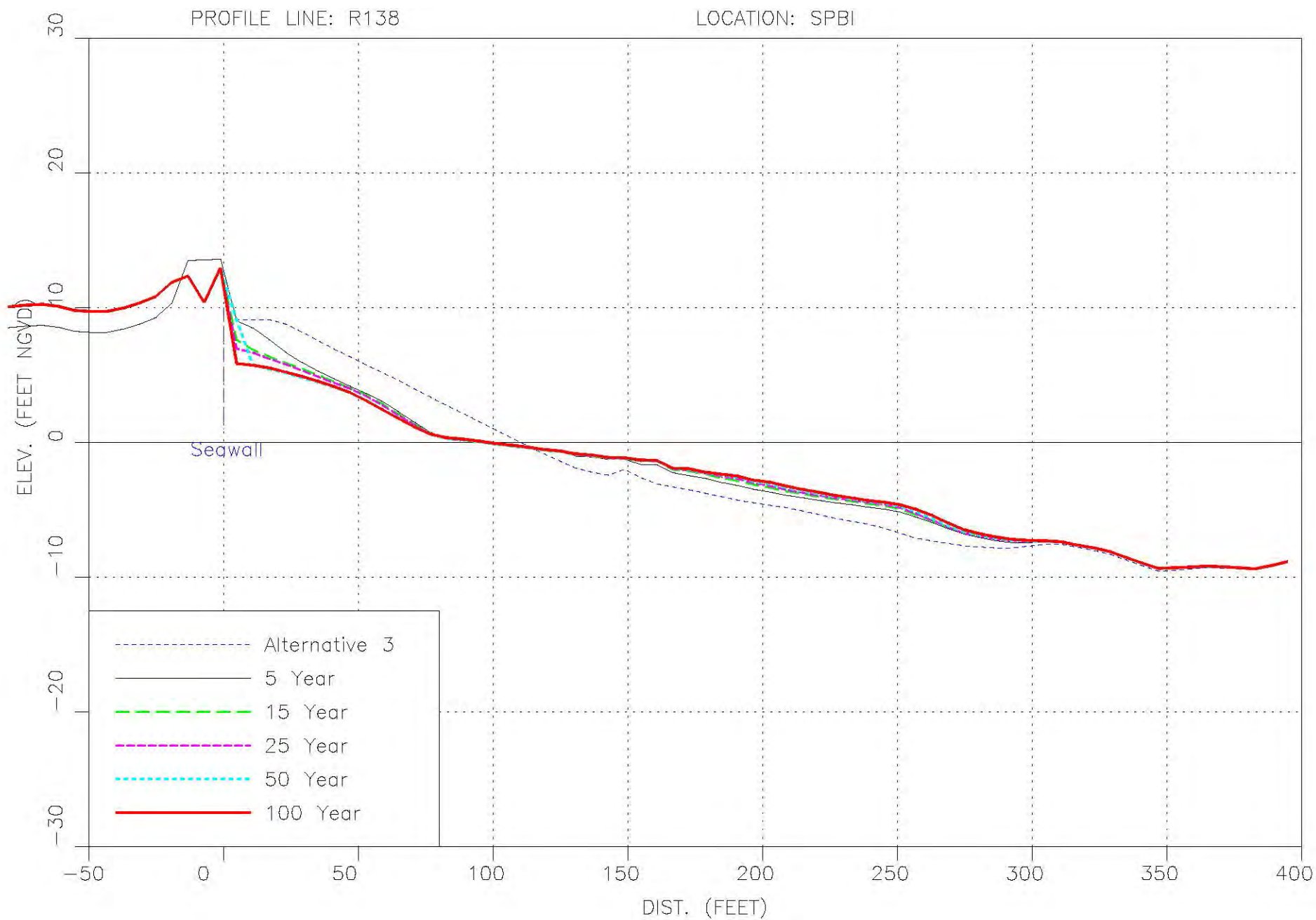






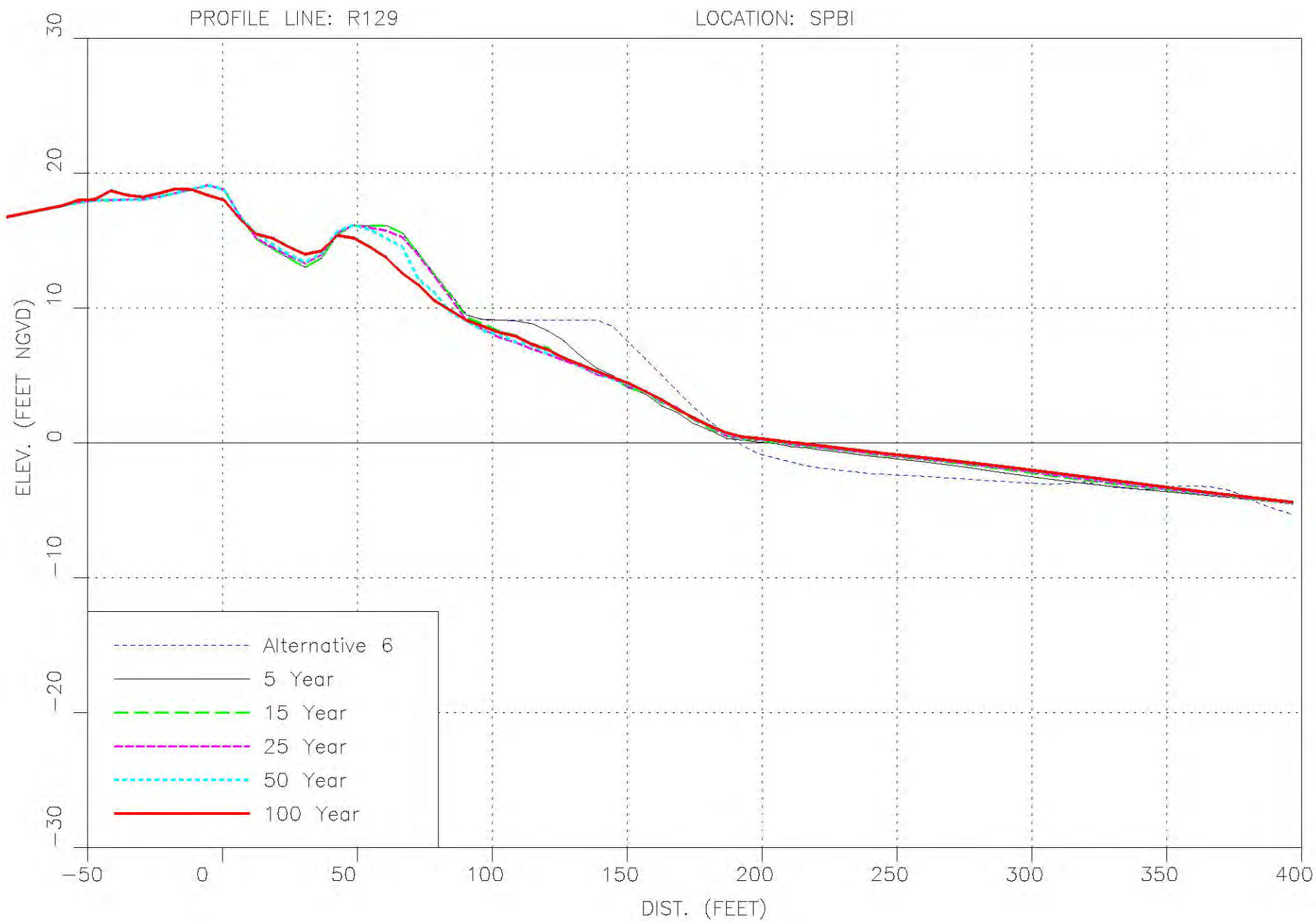


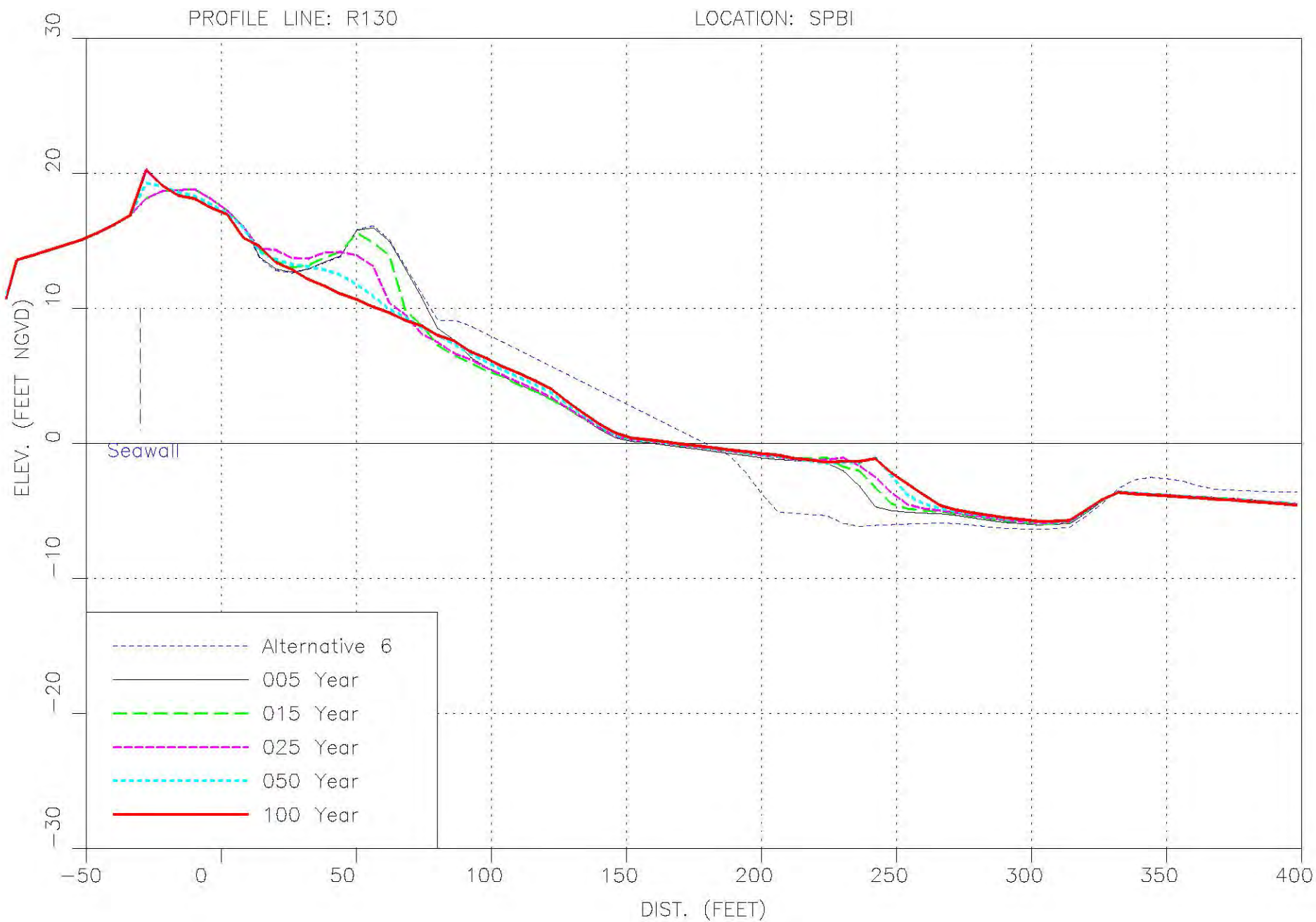


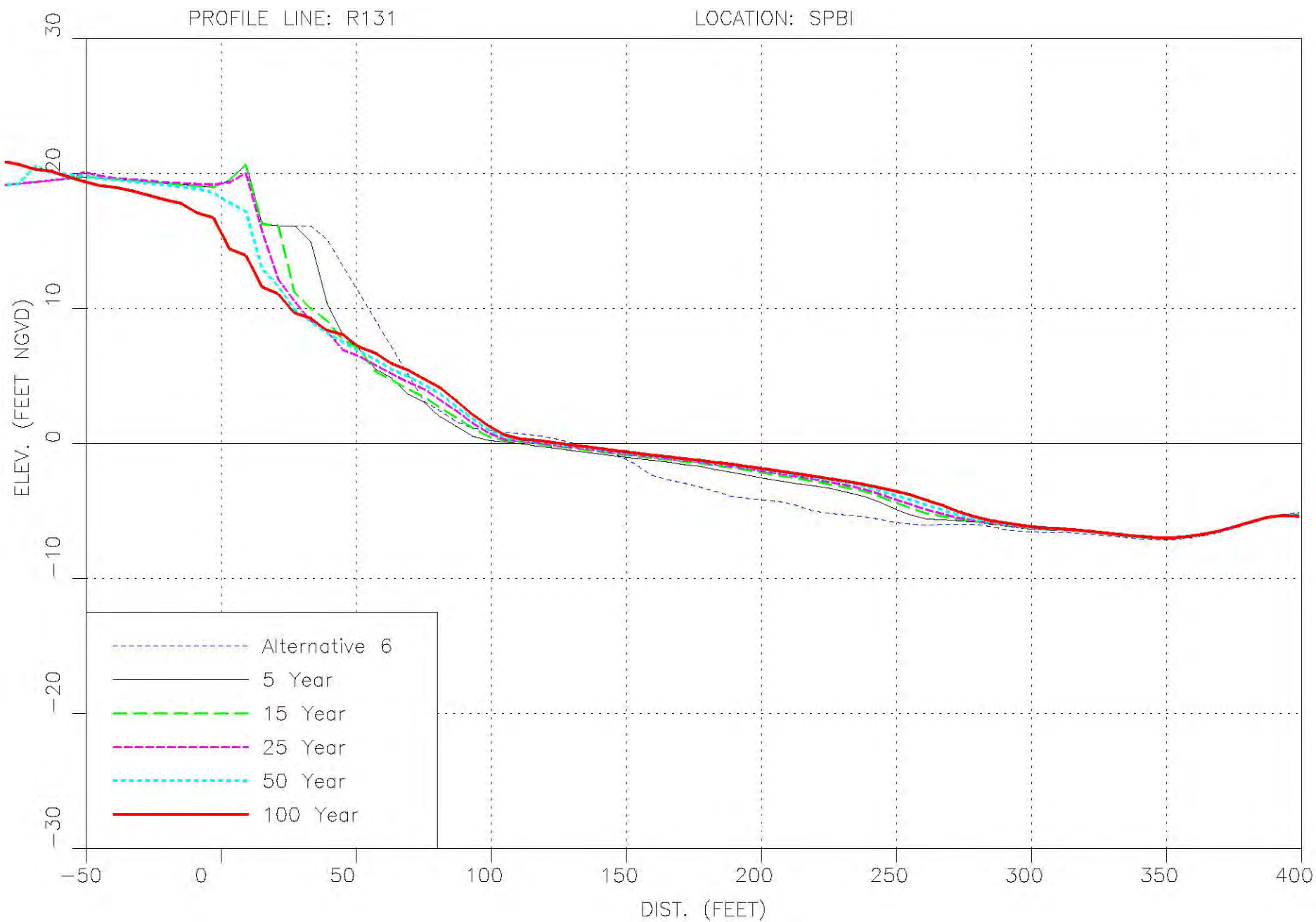


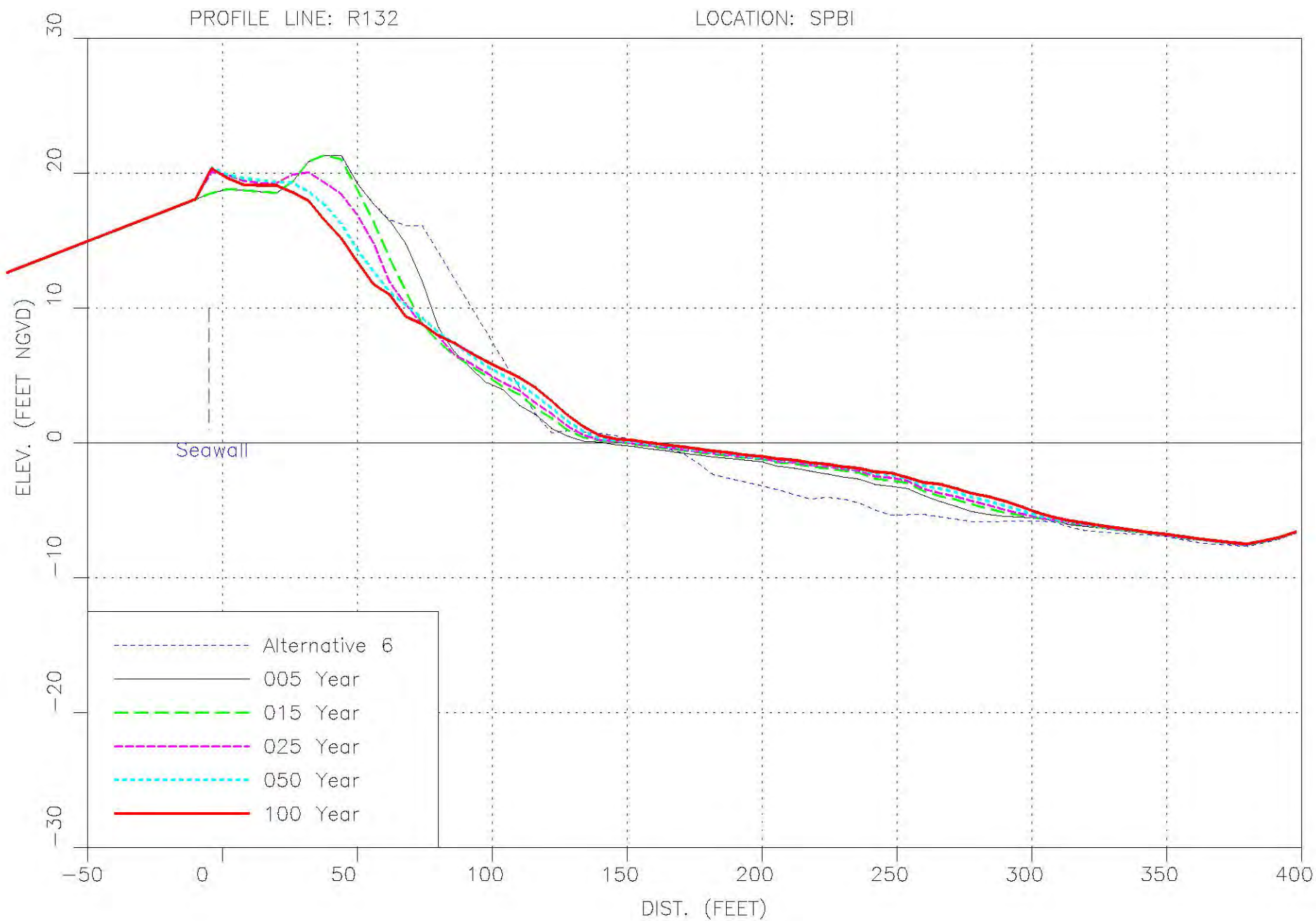
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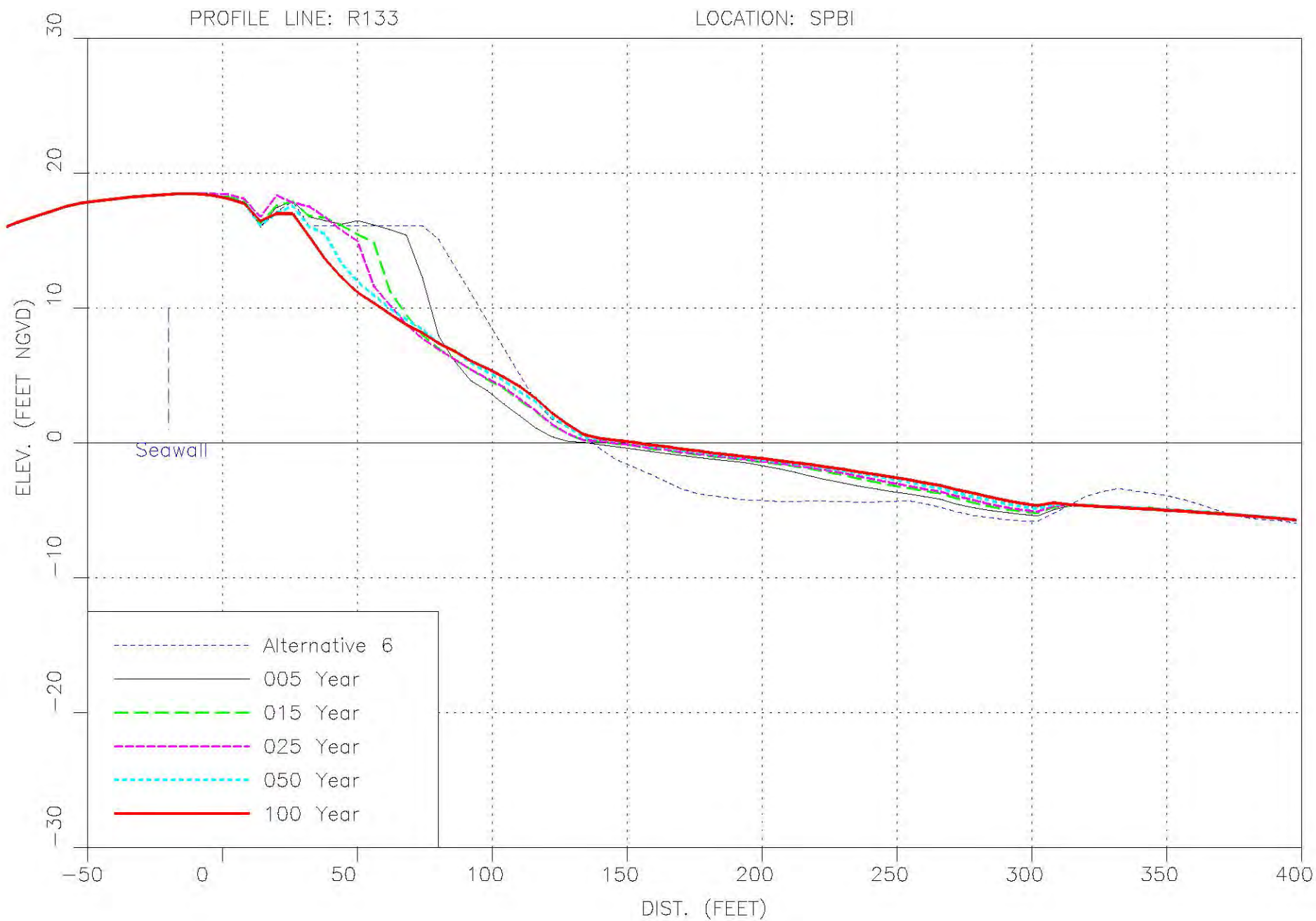
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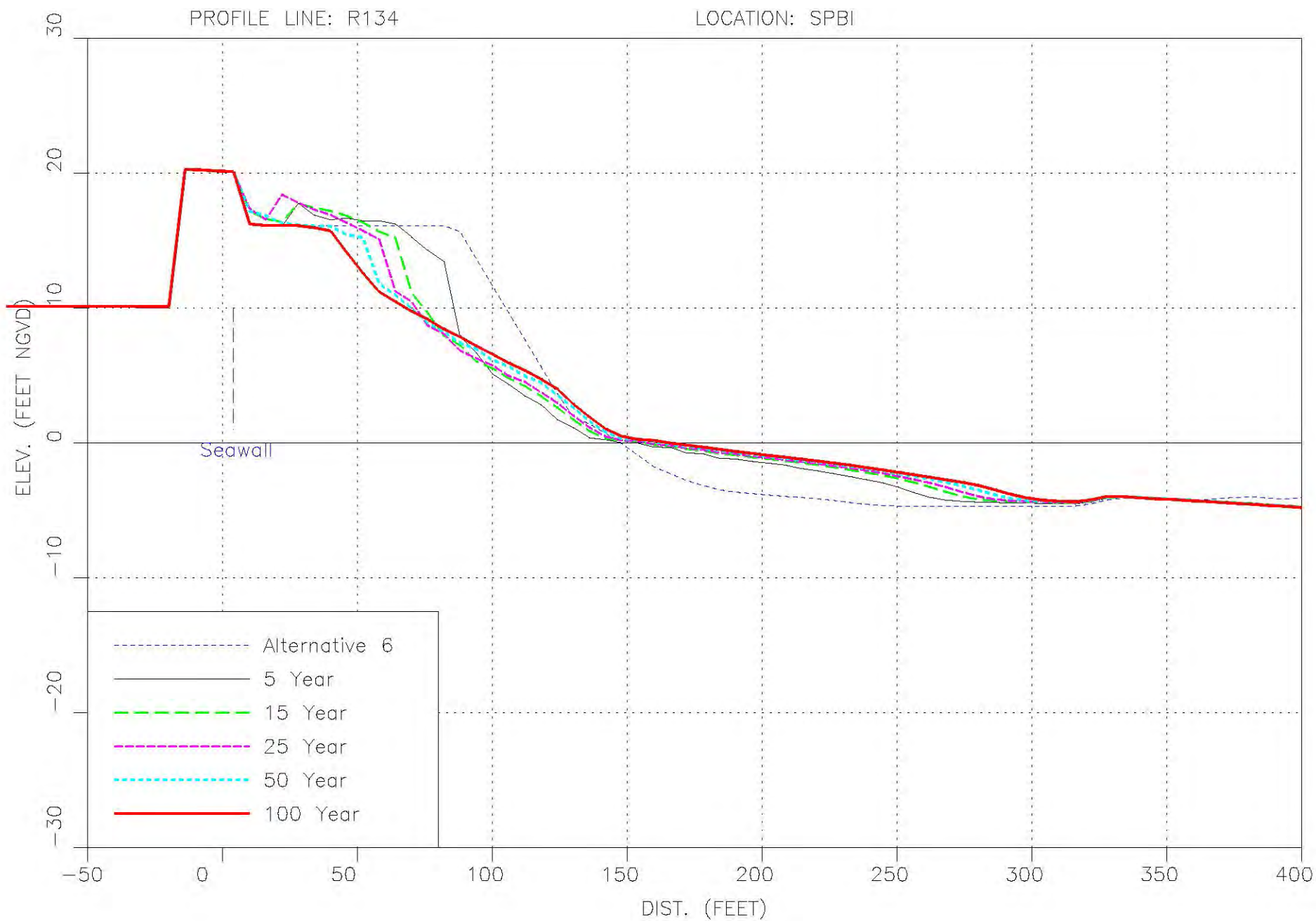


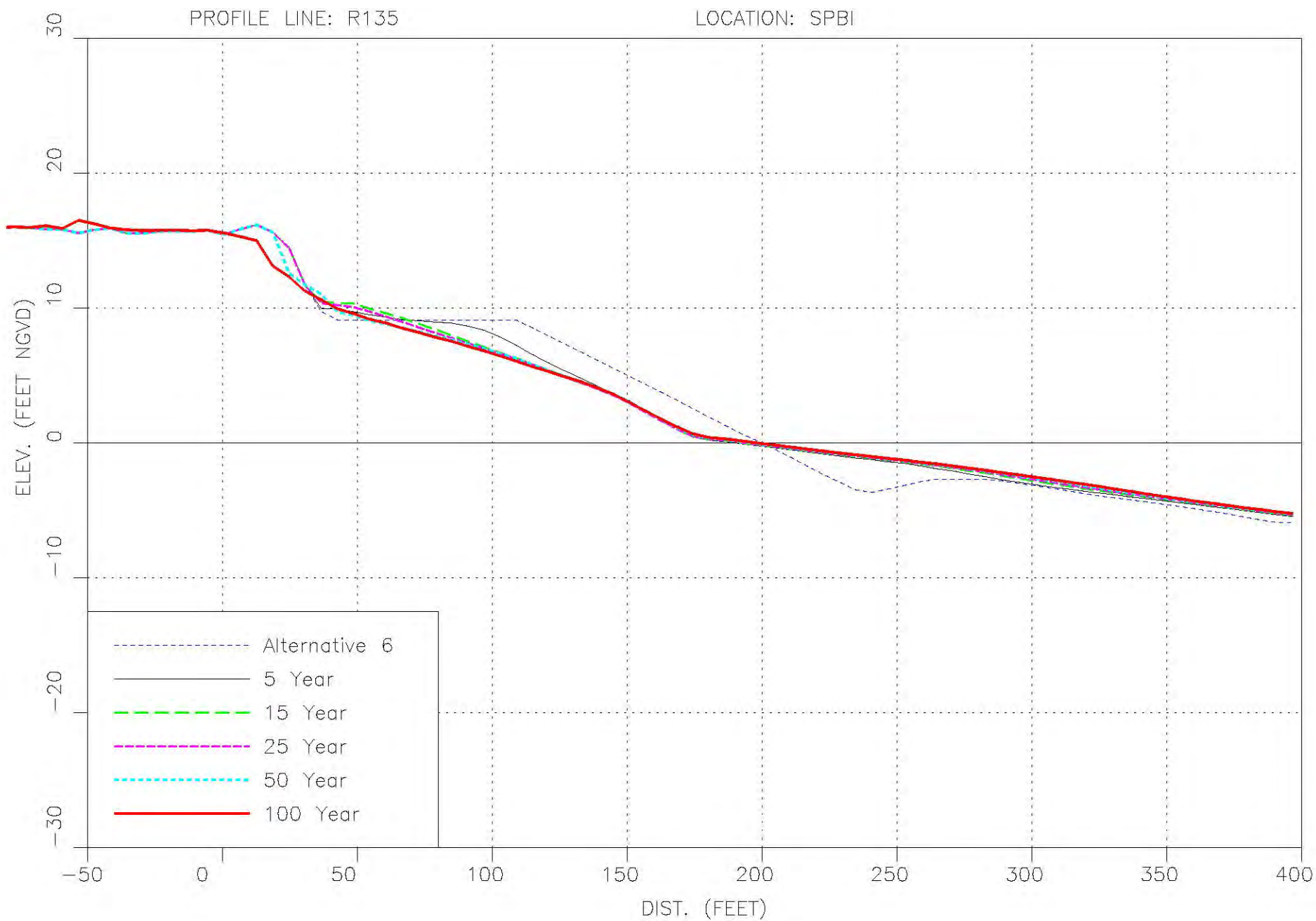


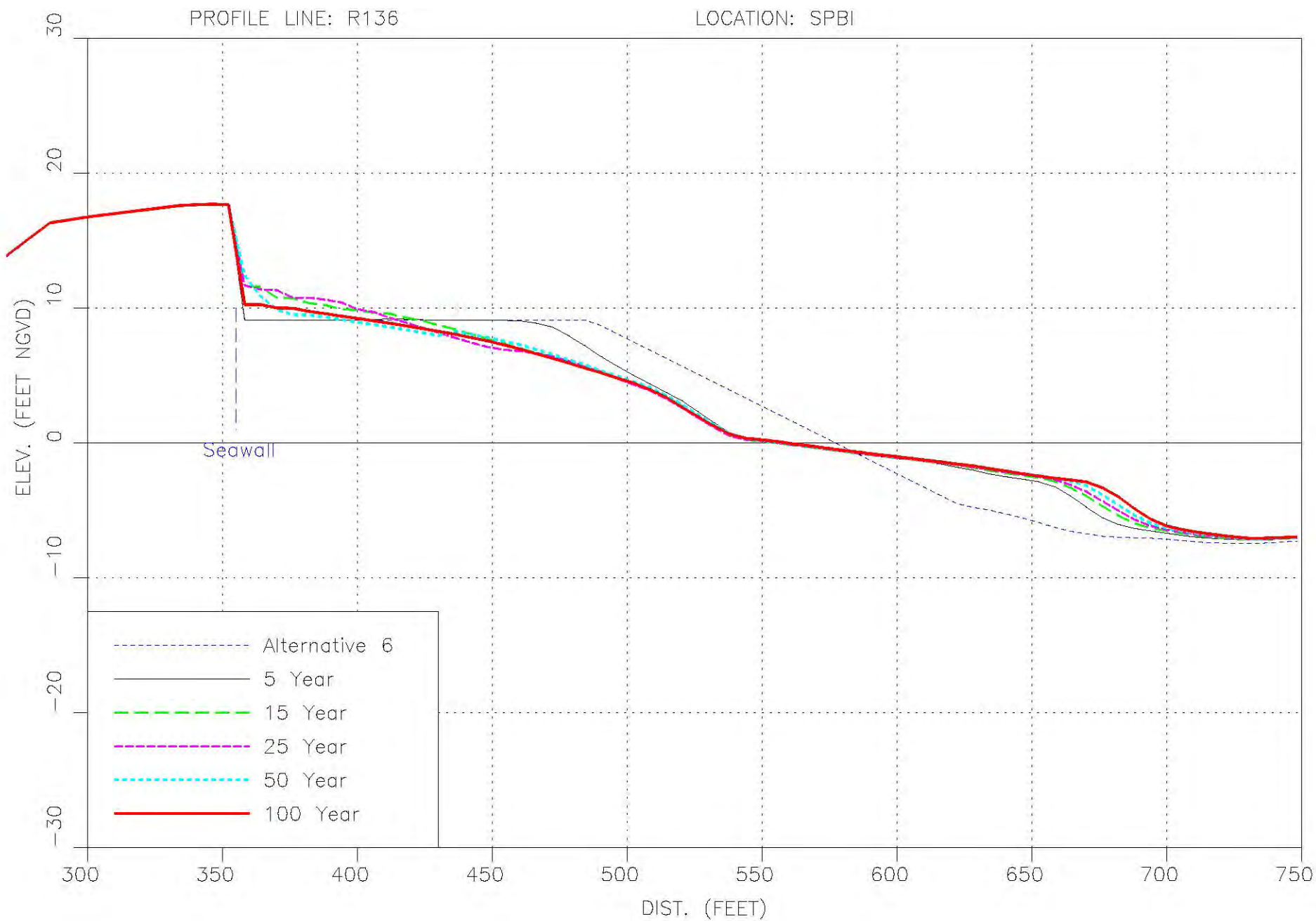


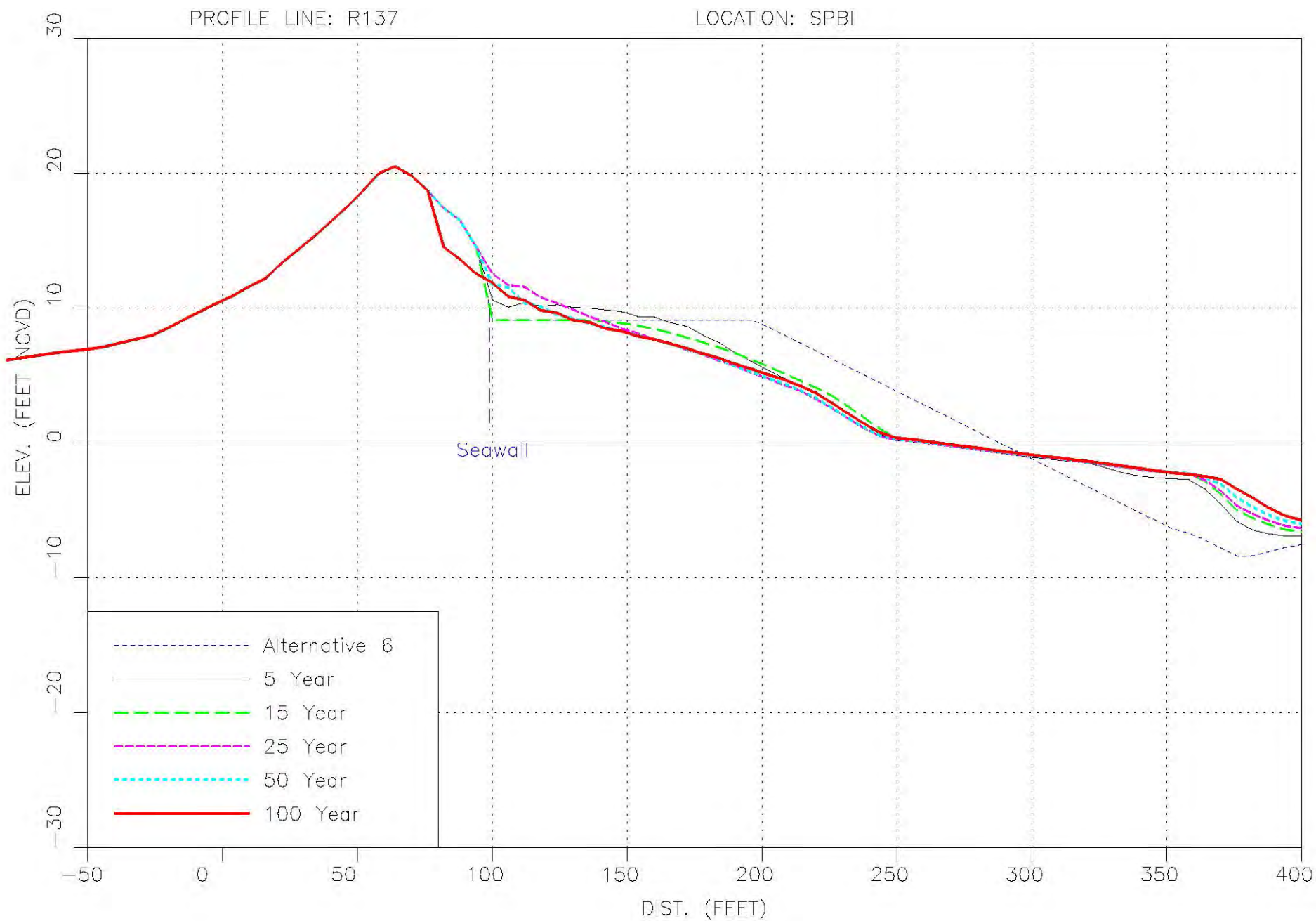


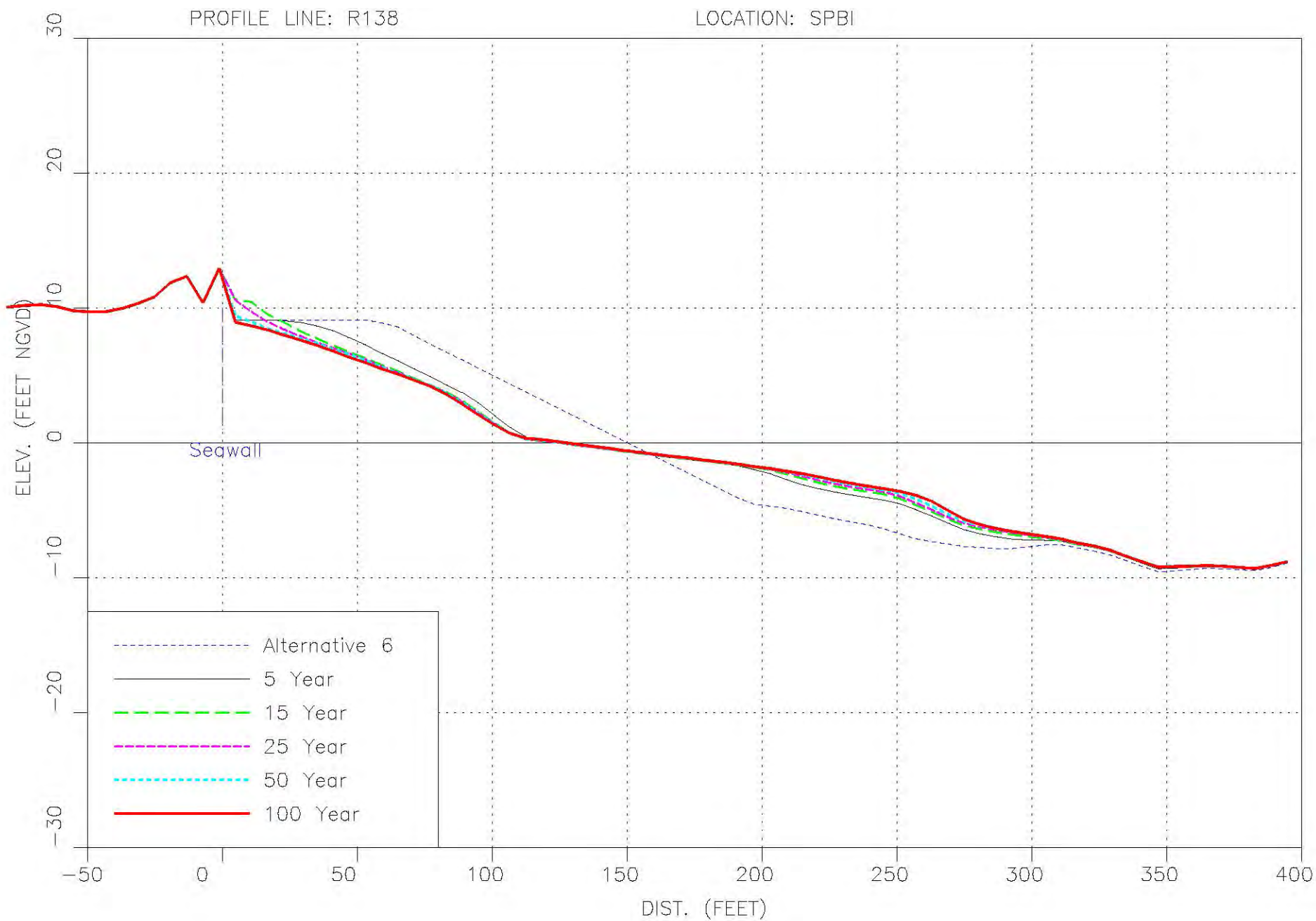






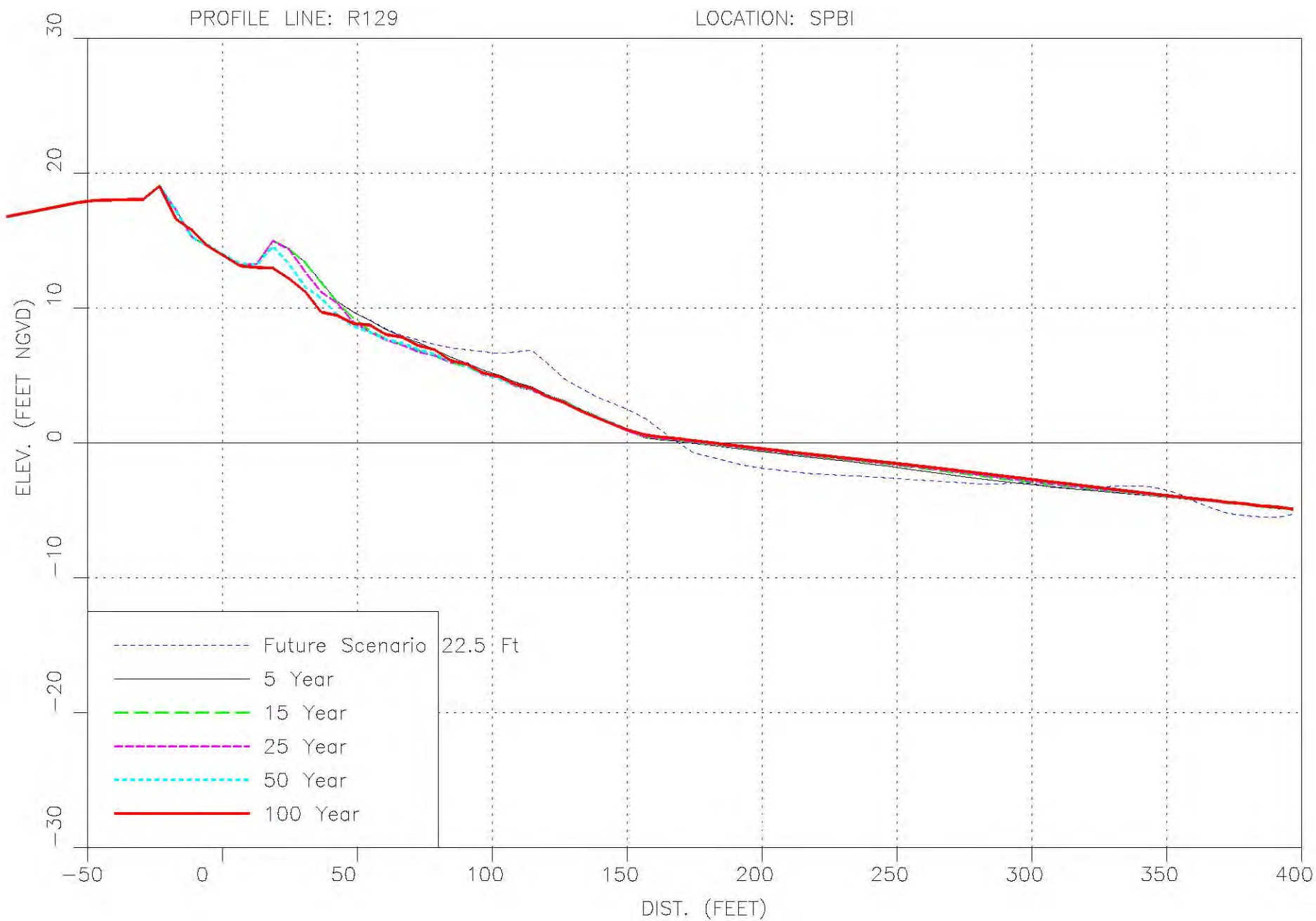


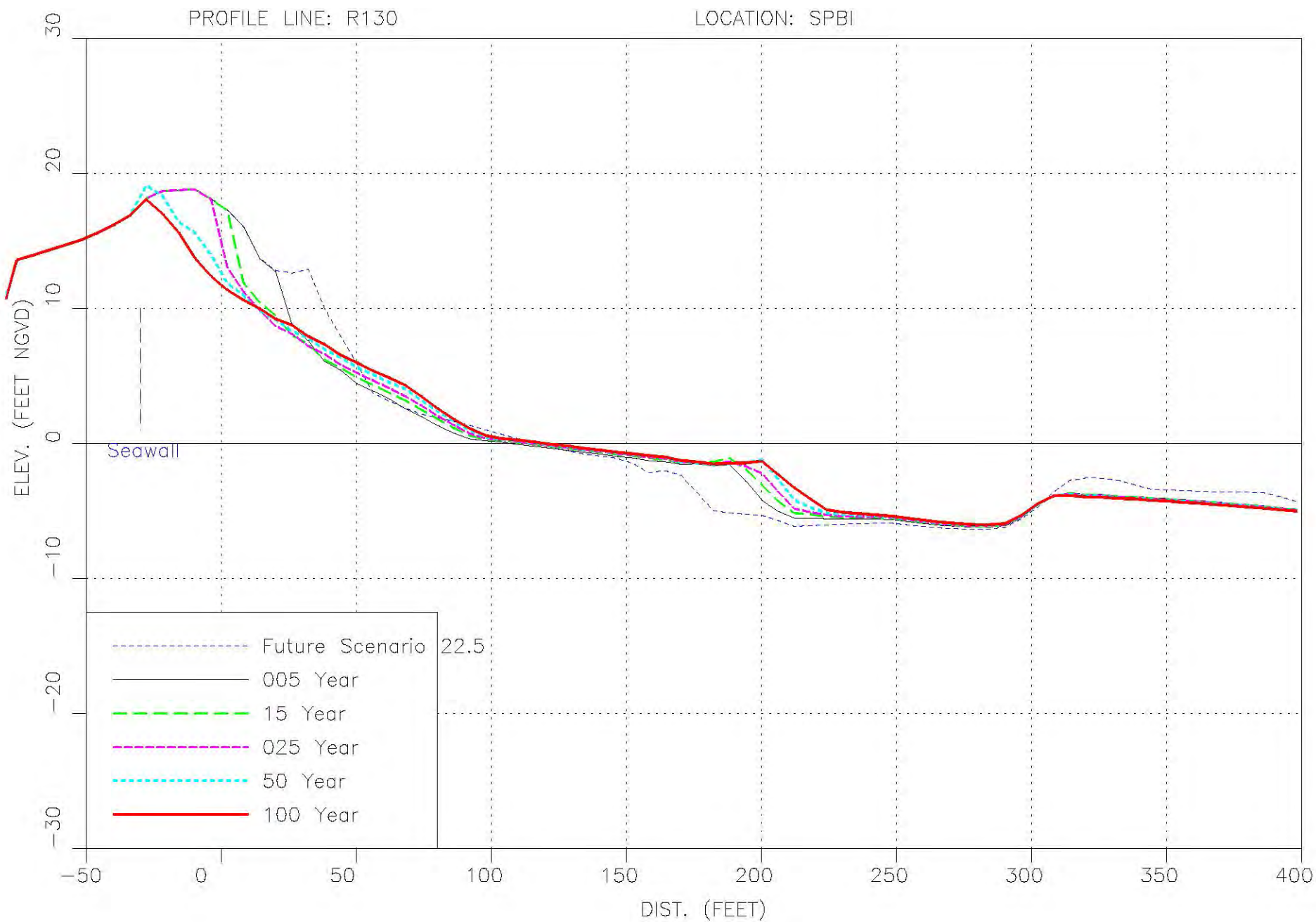


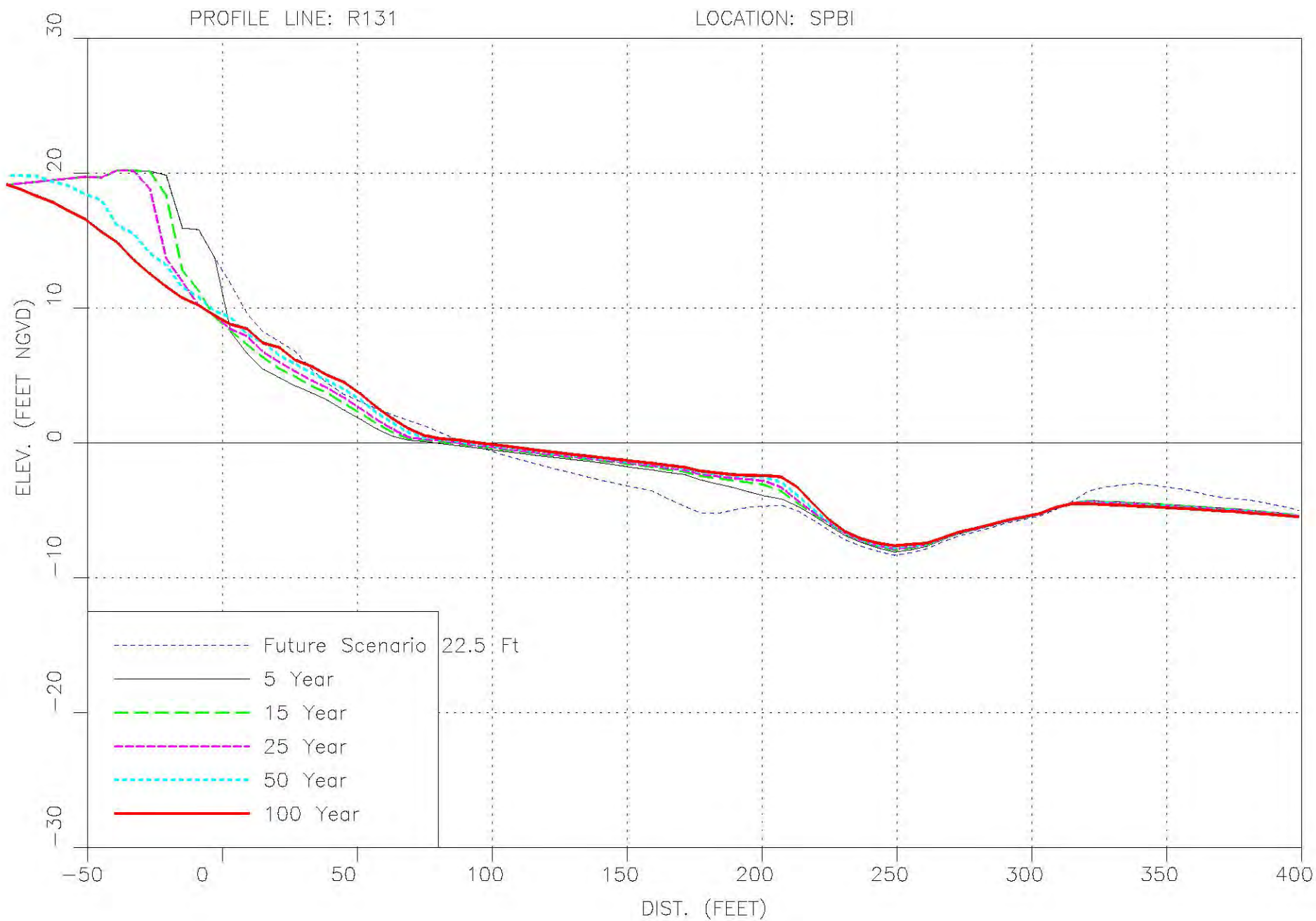


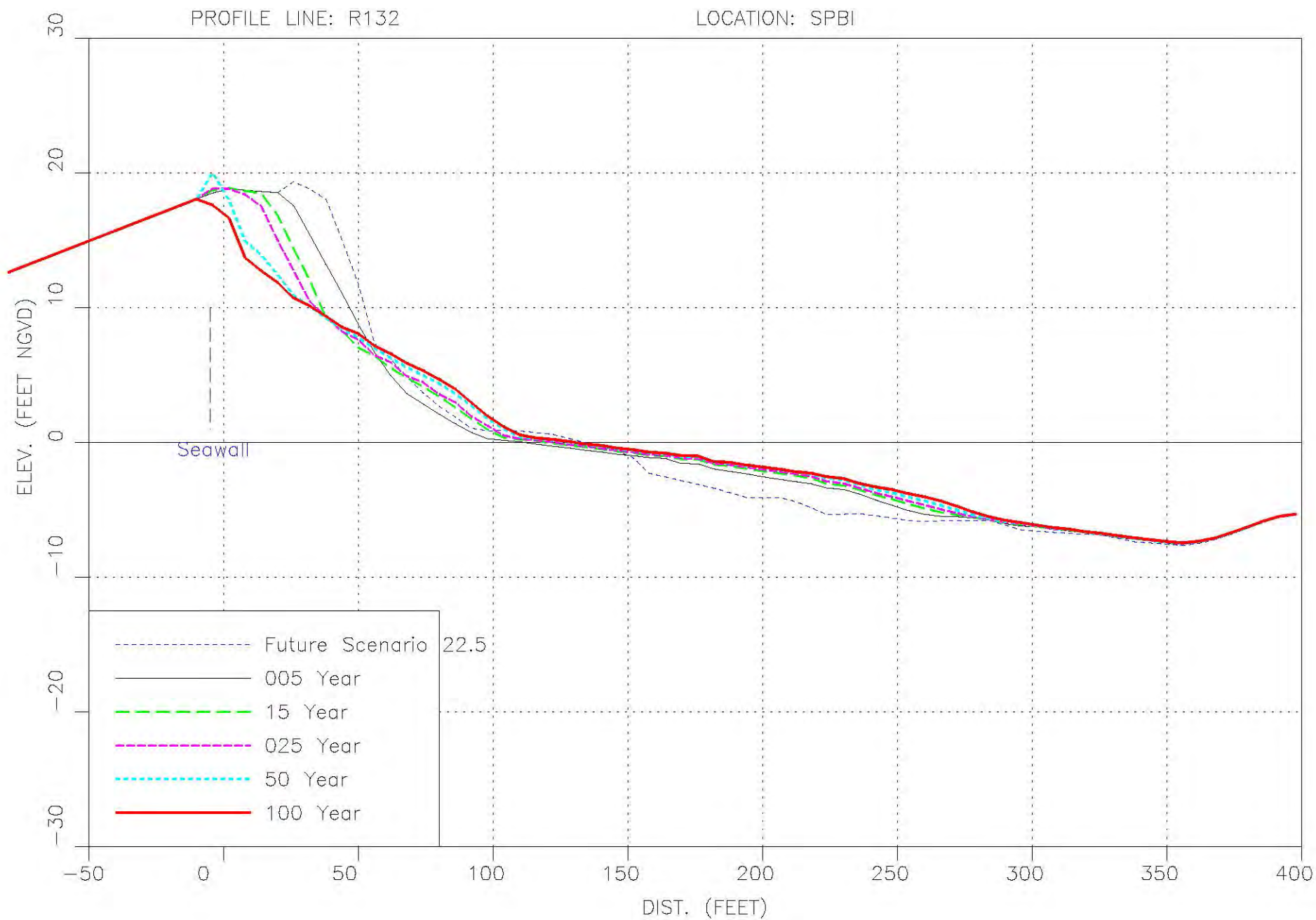
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SBEACH ANALYSIS REPORT

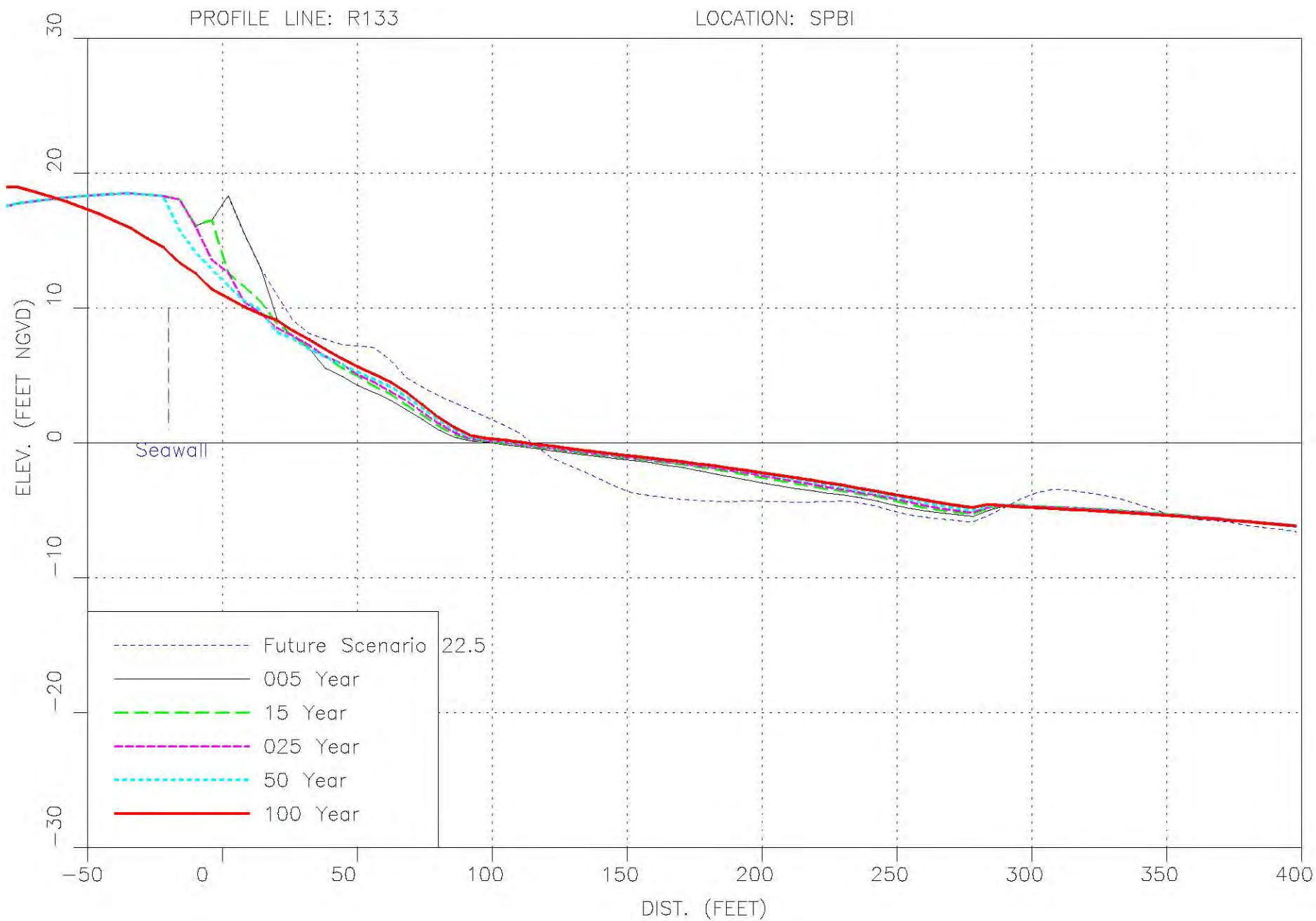
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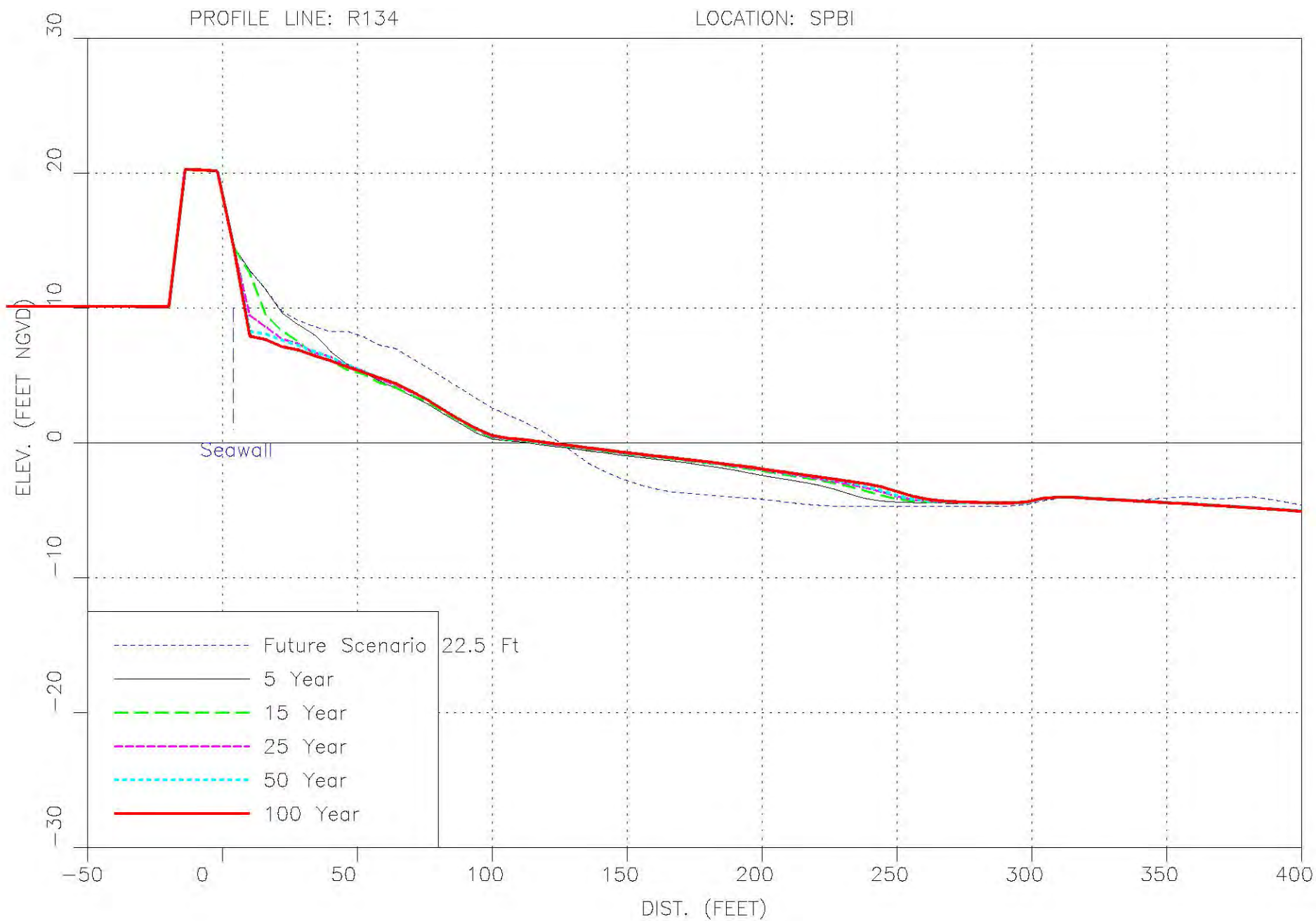


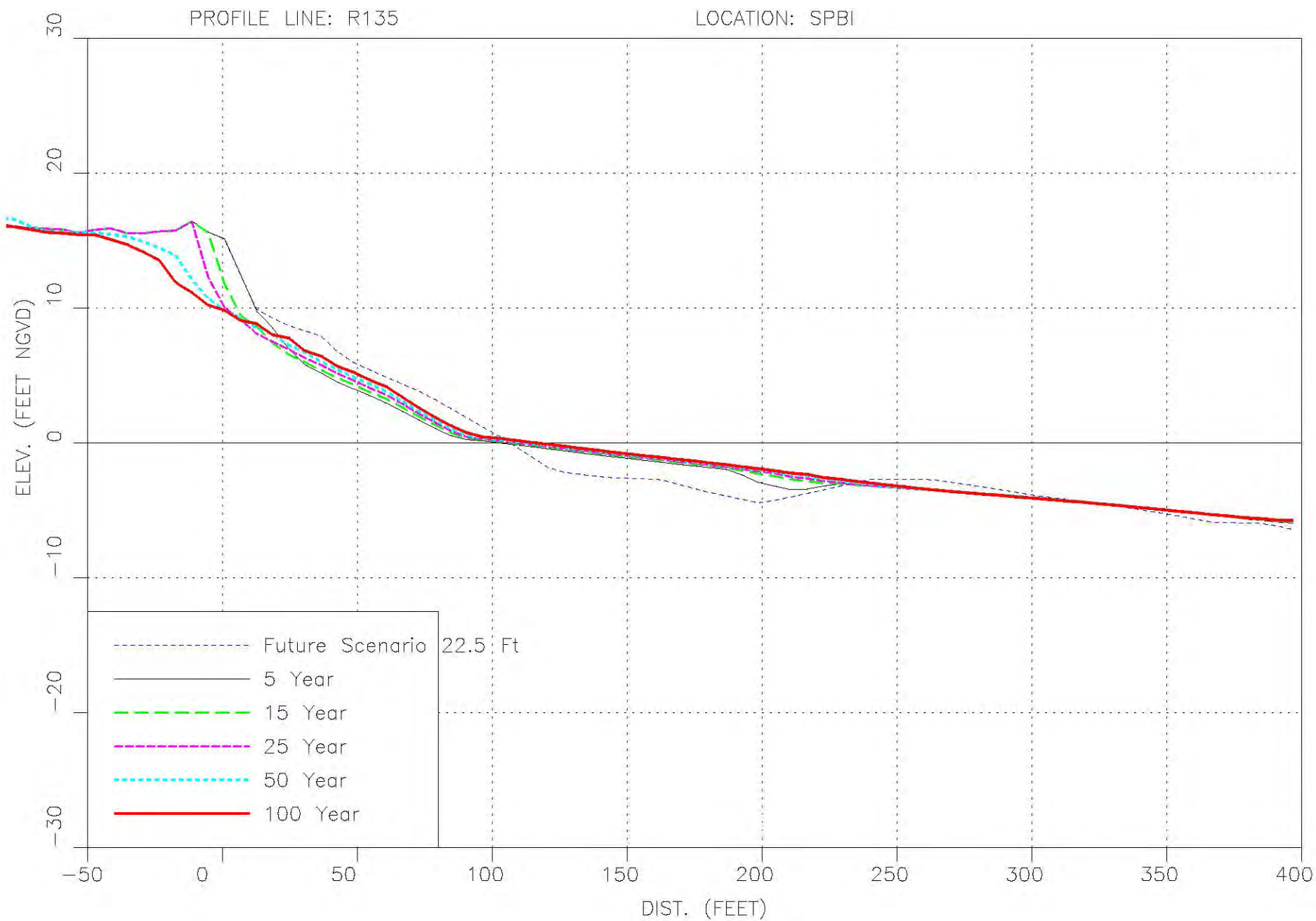


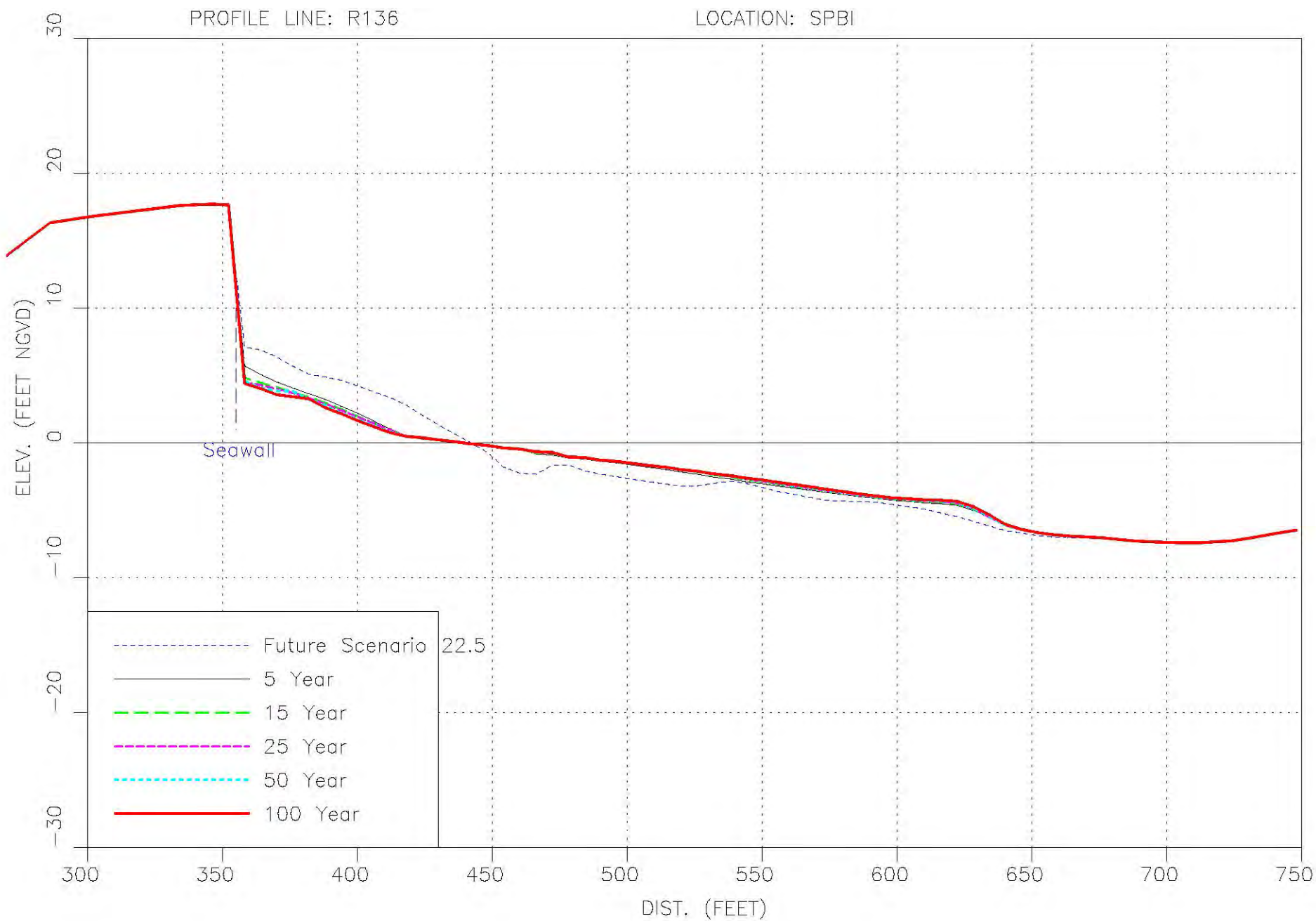


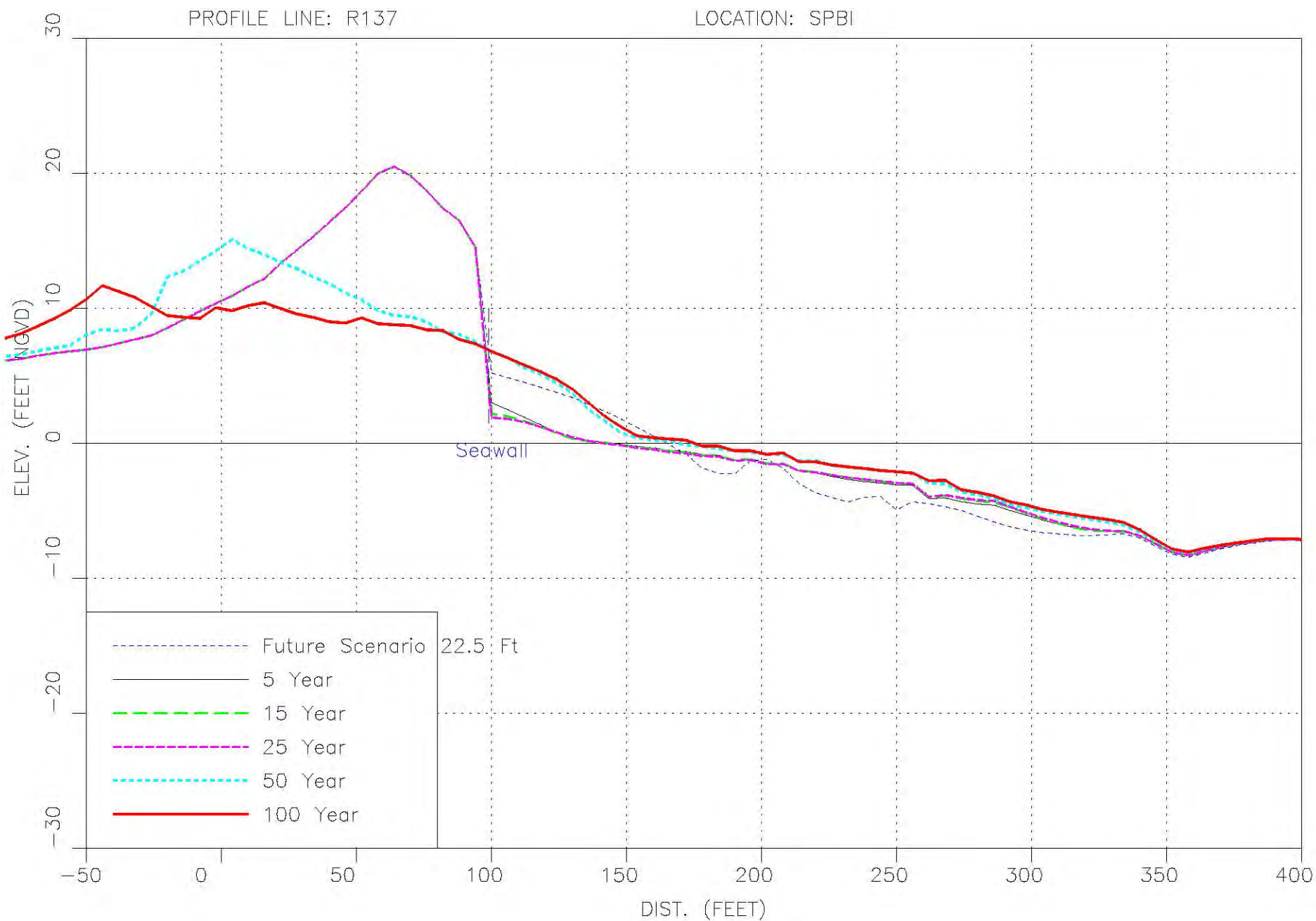


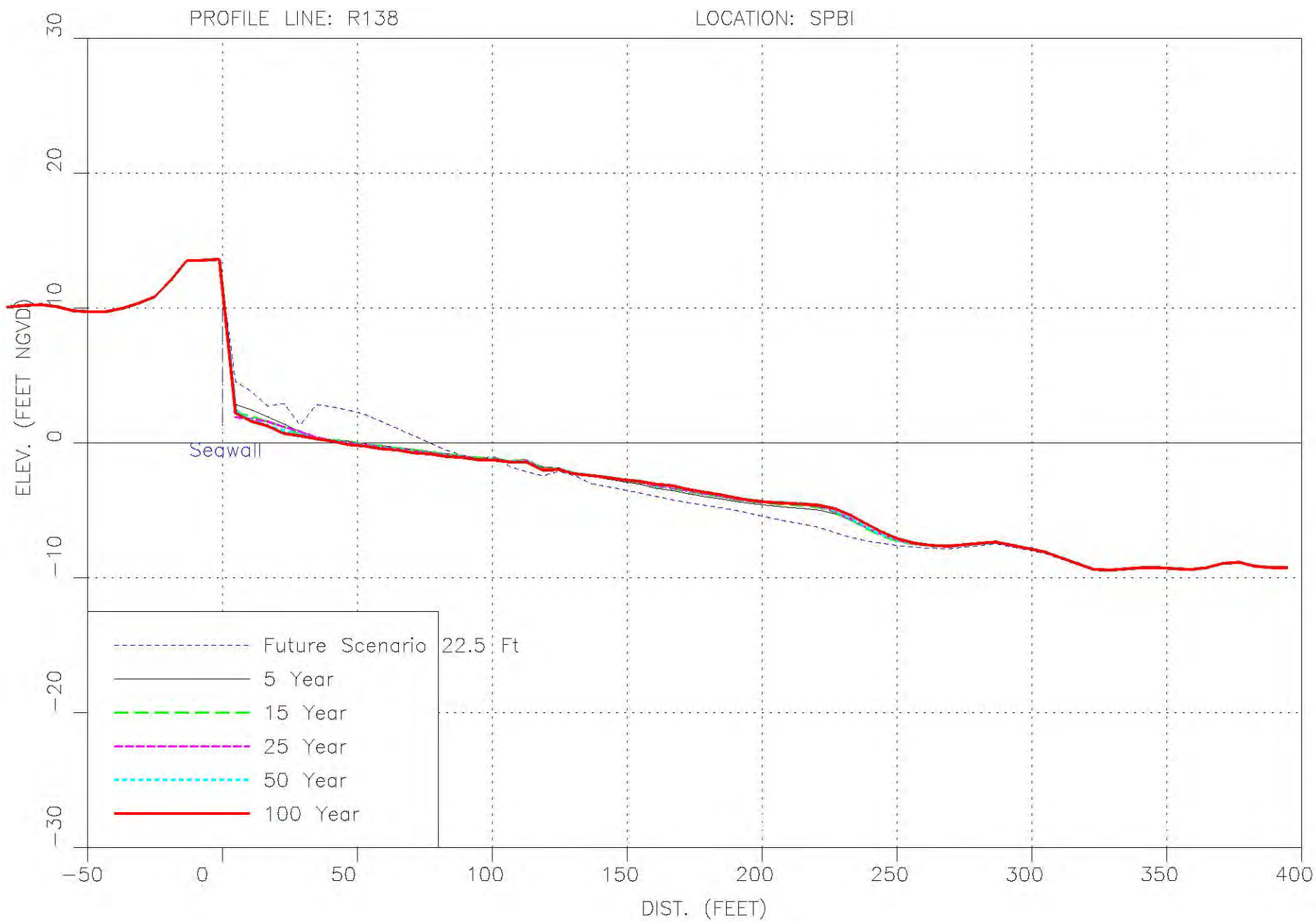


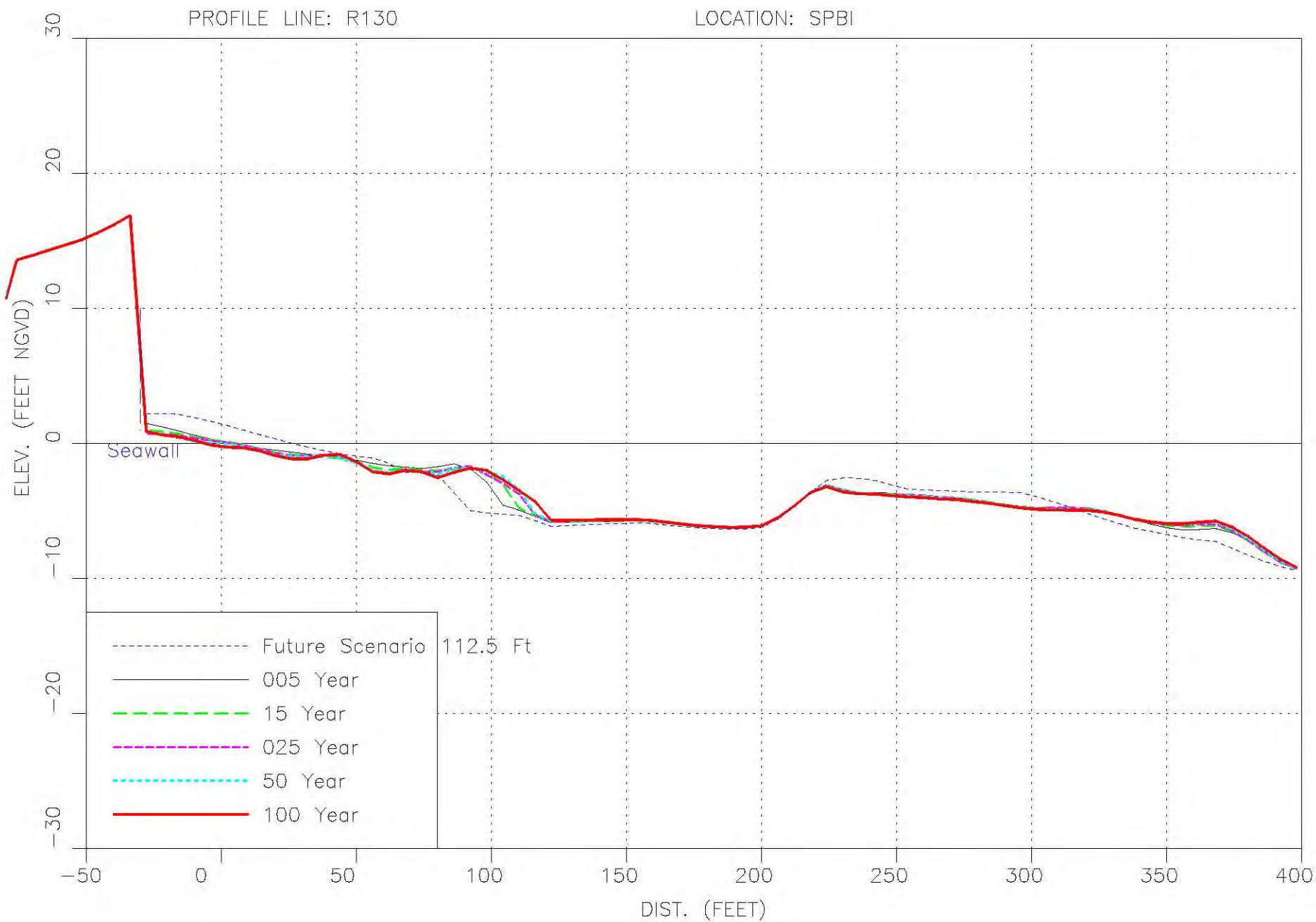


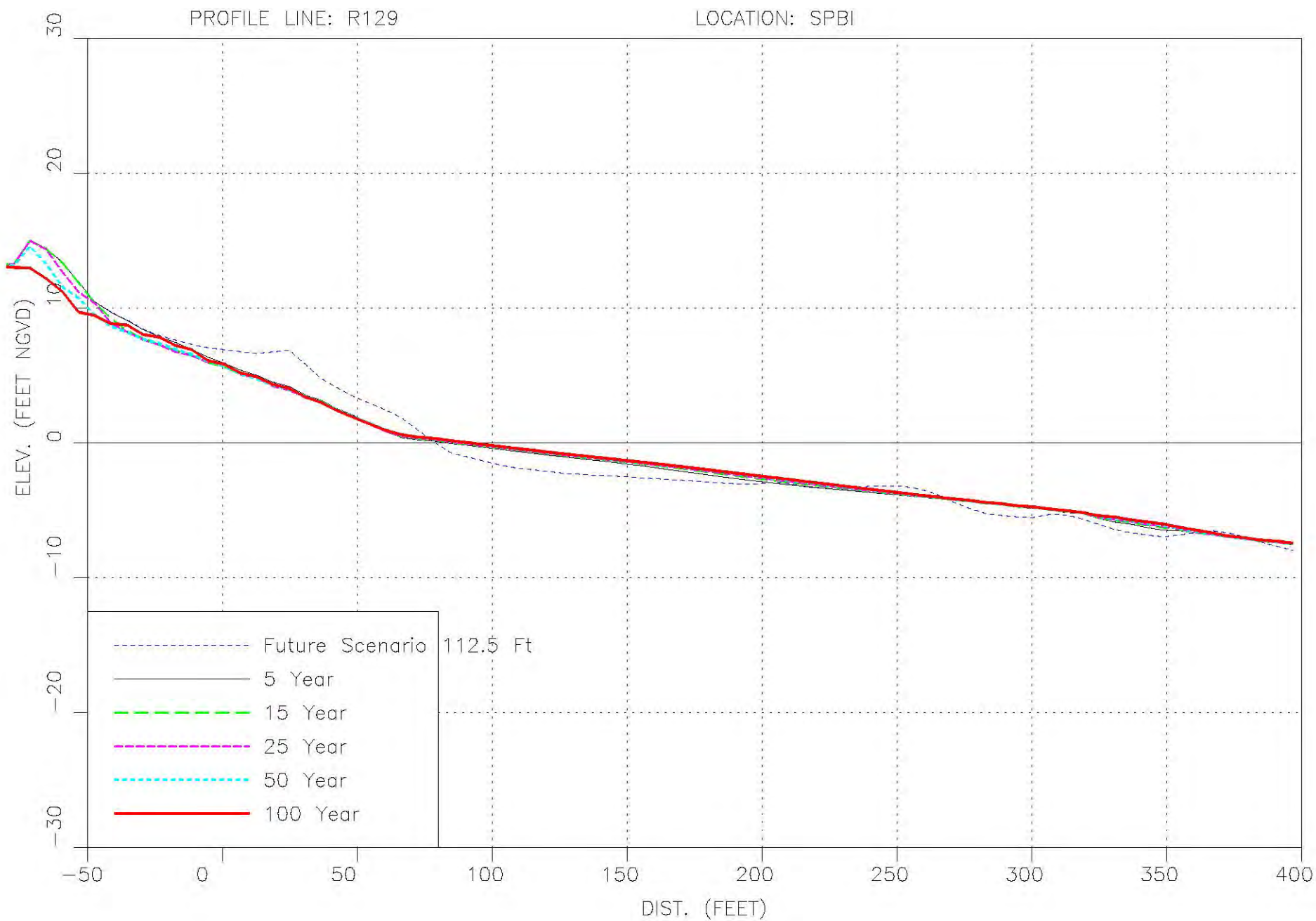


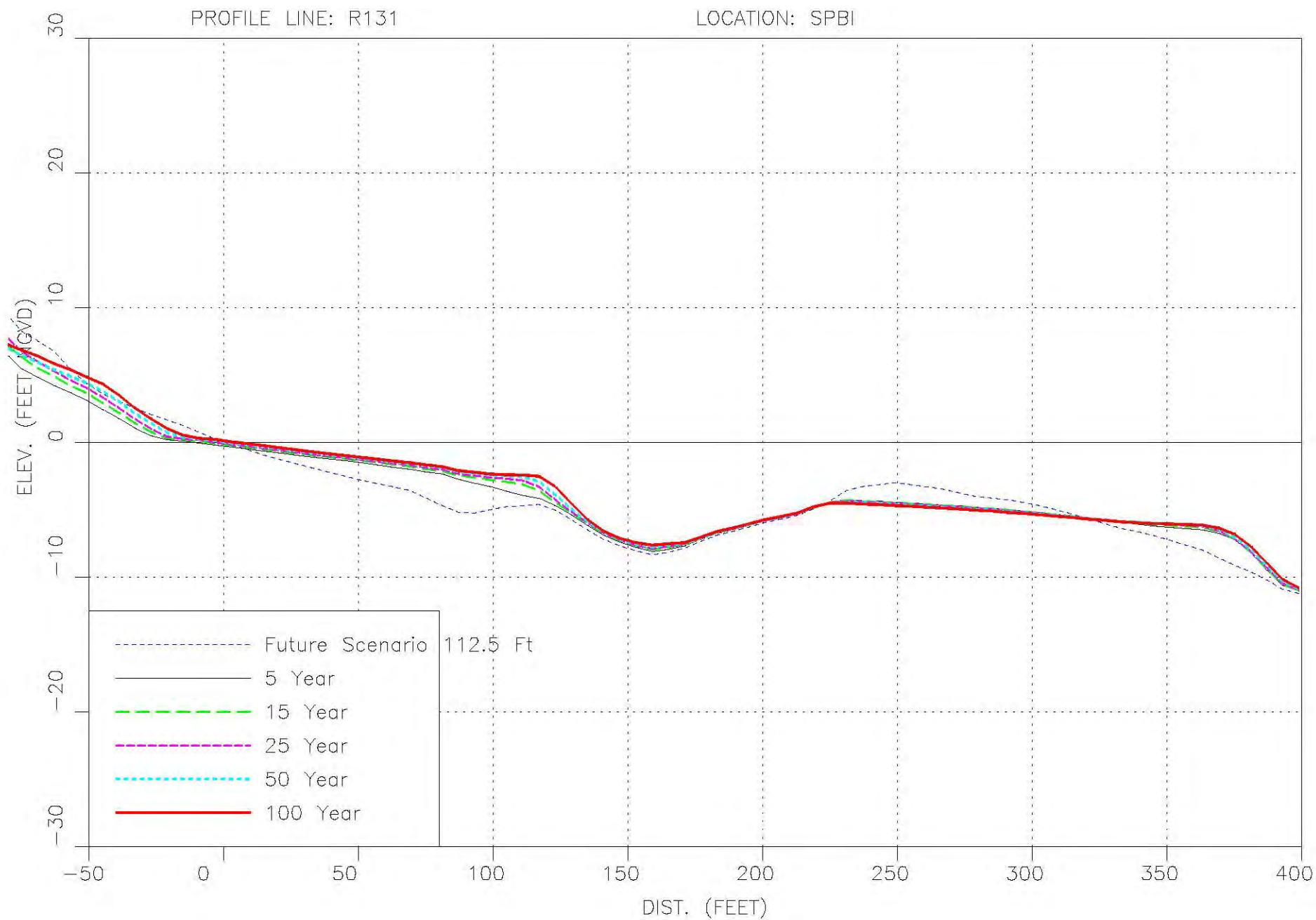


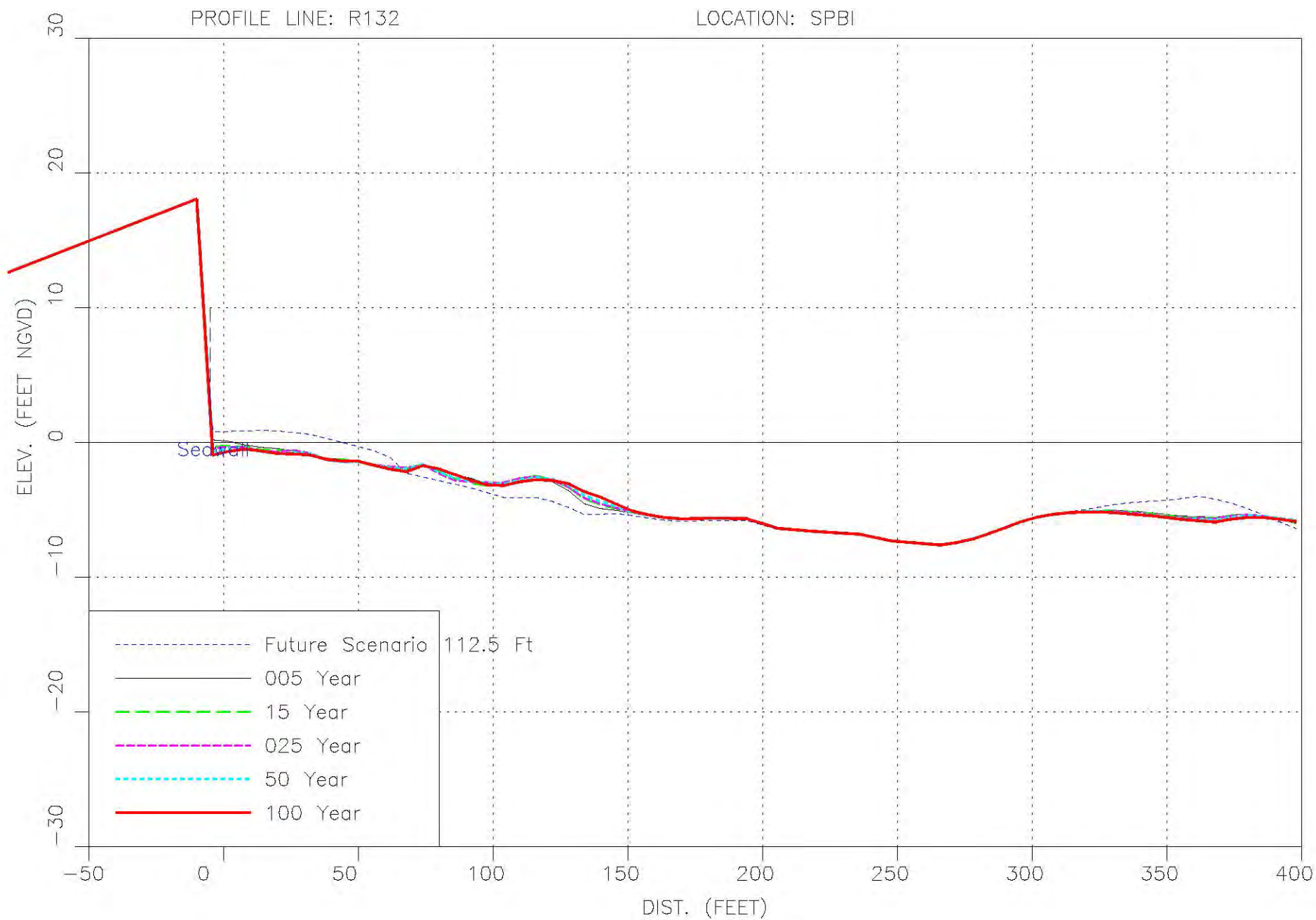


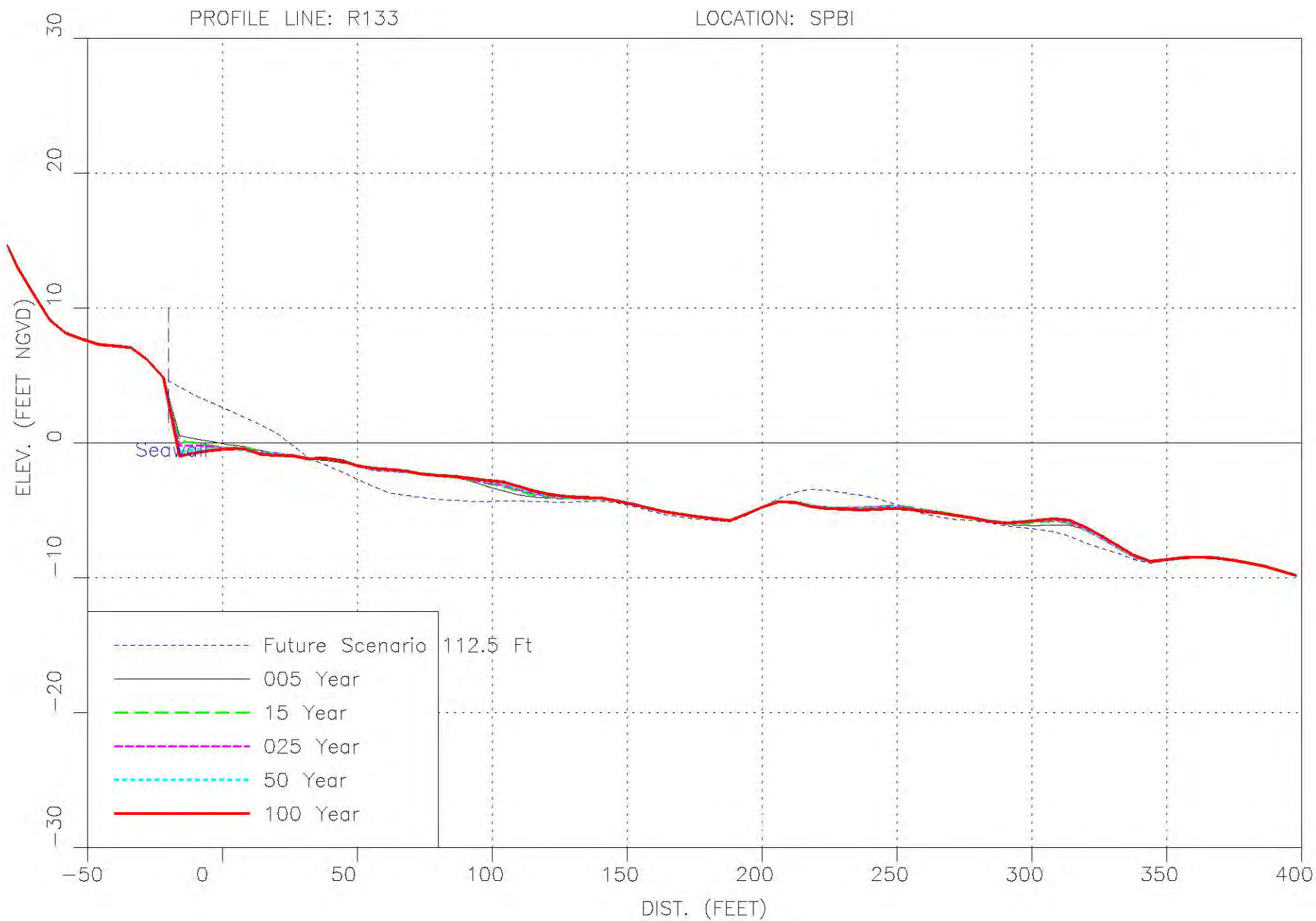


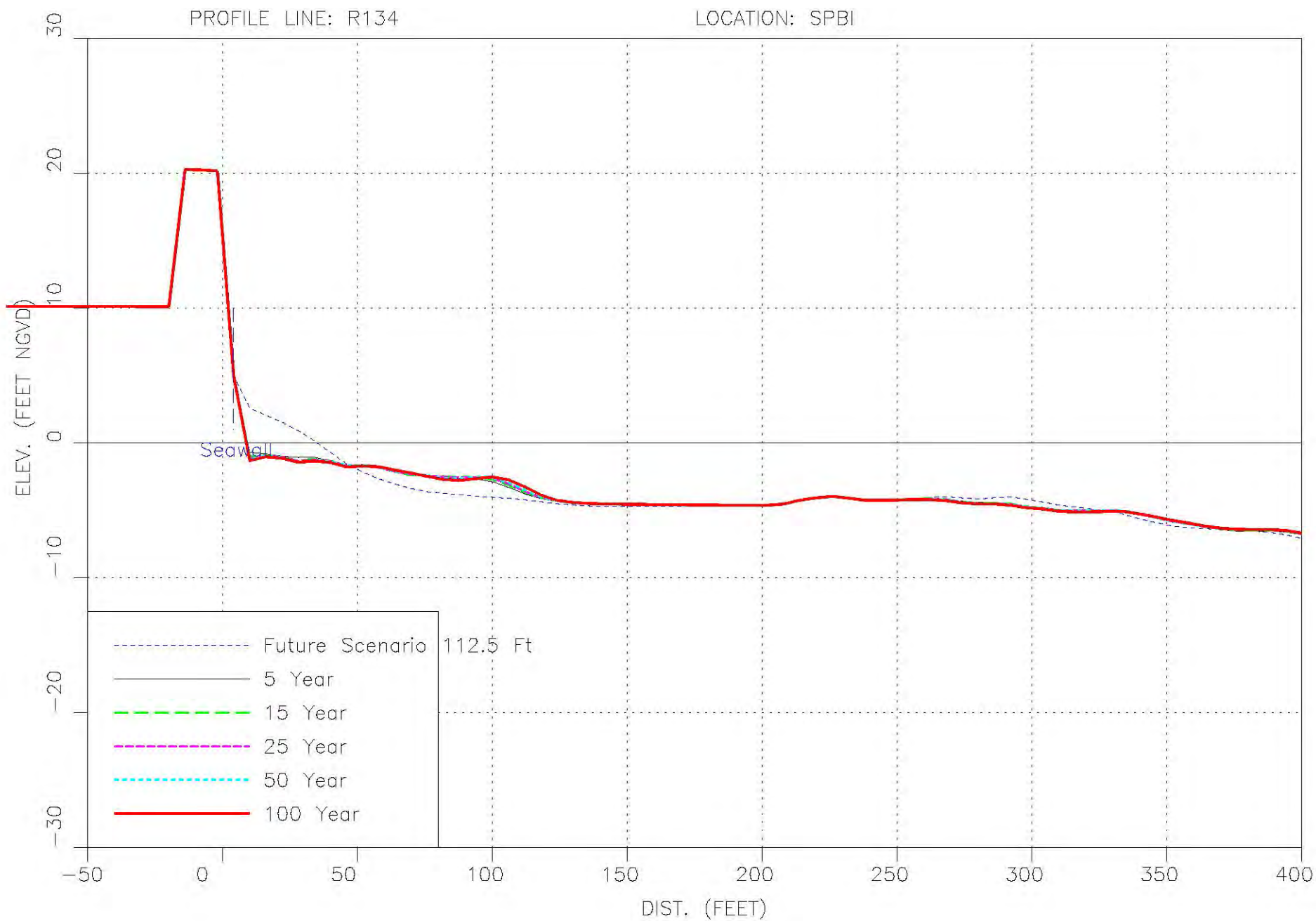


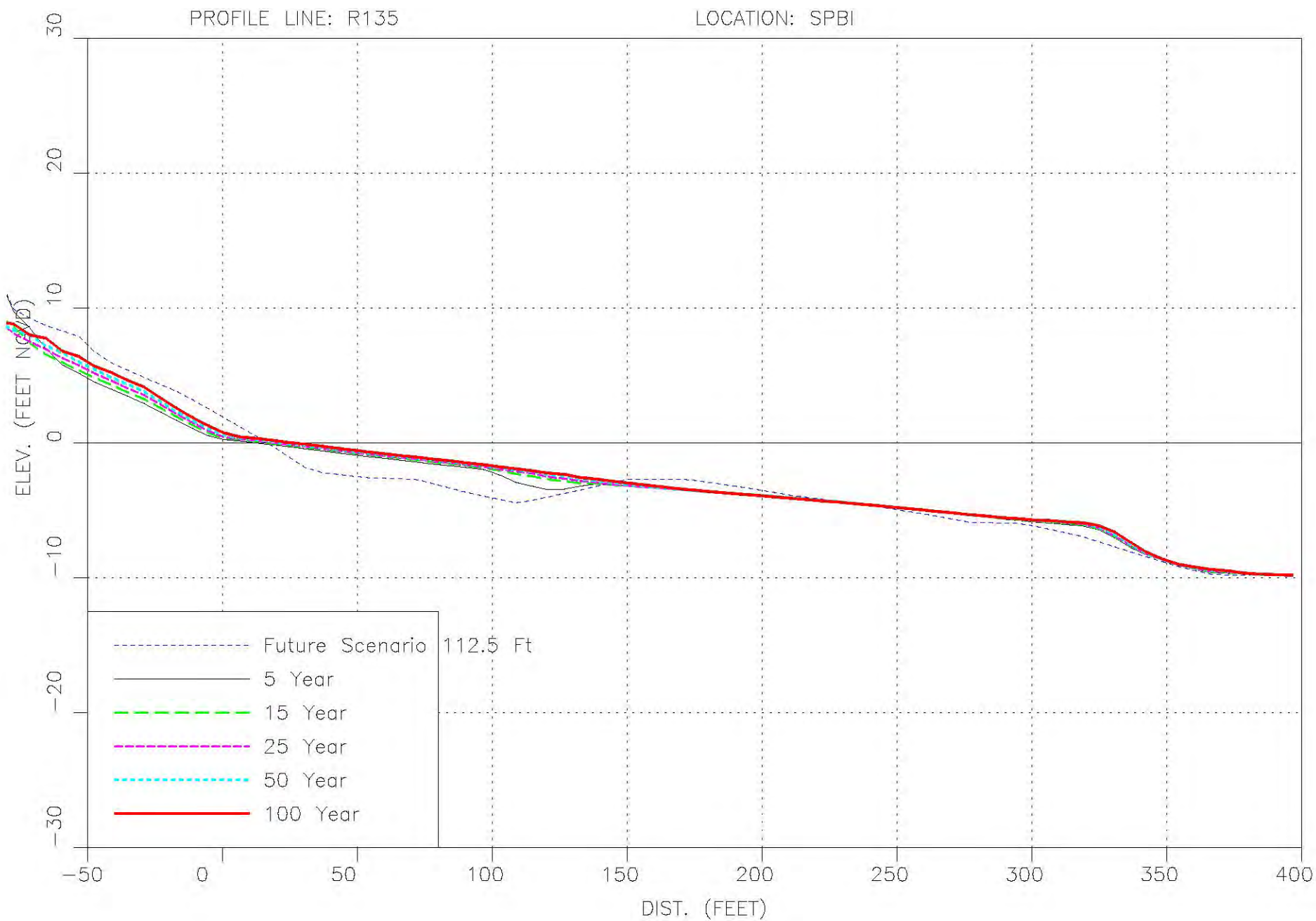


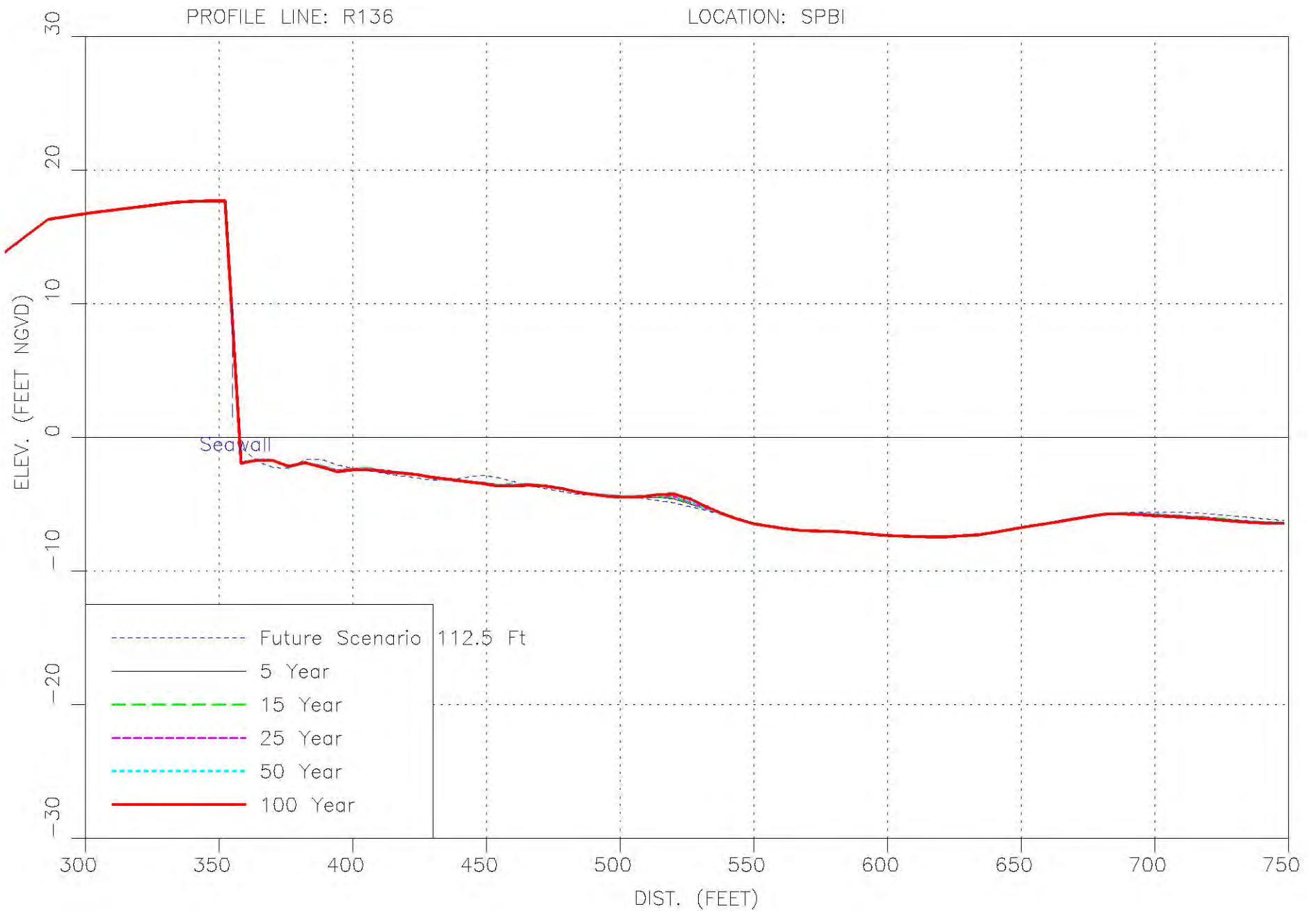


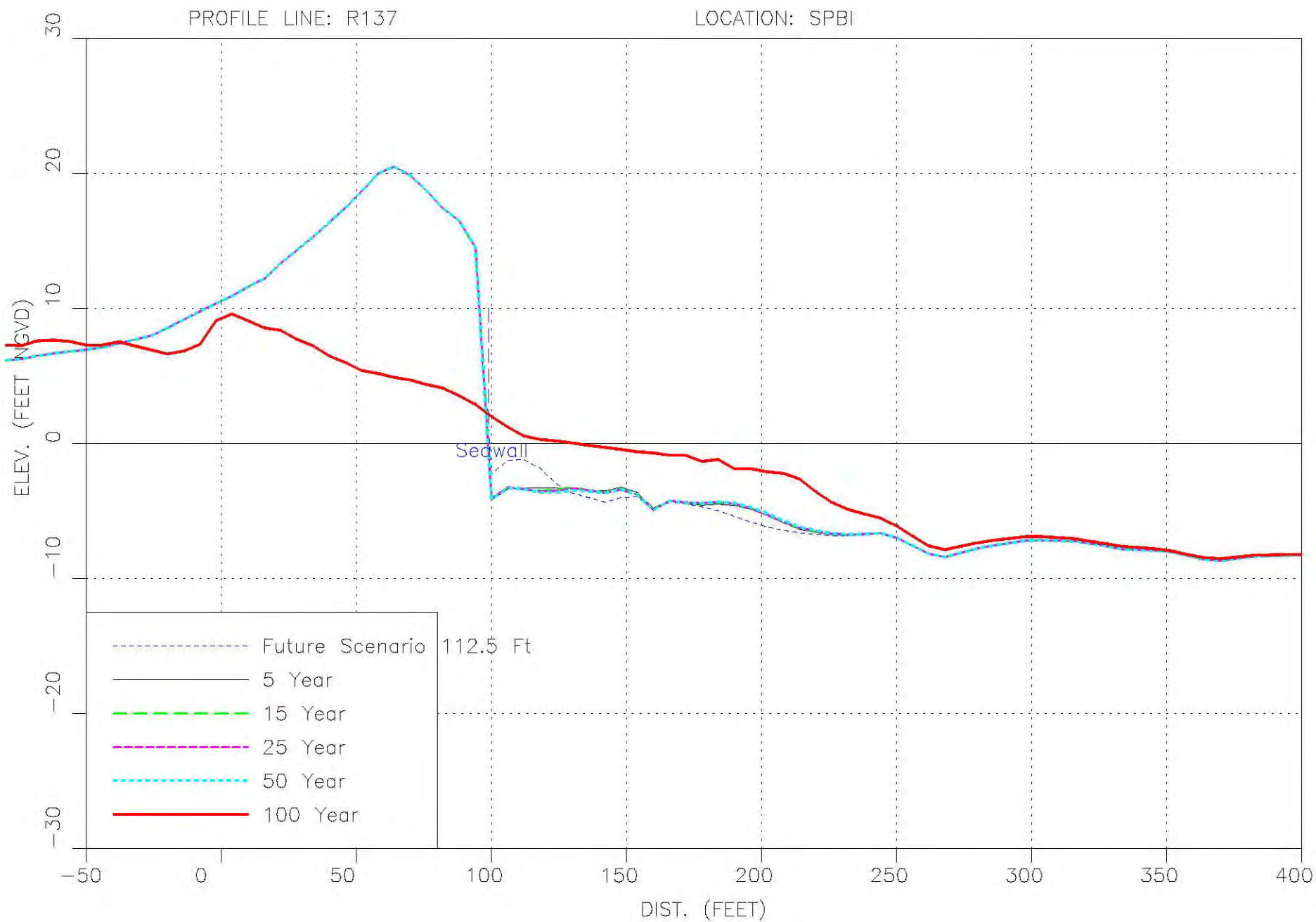


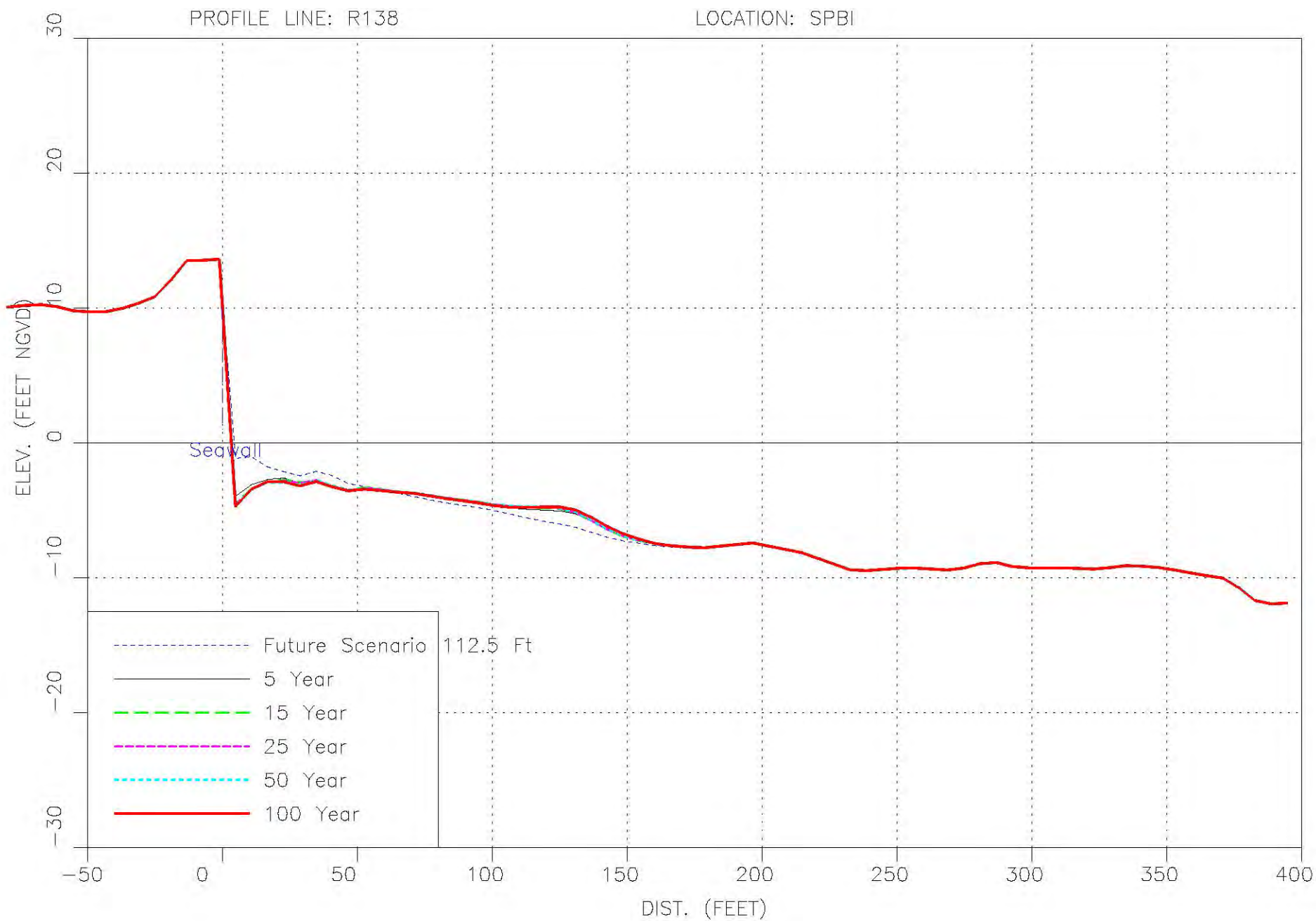








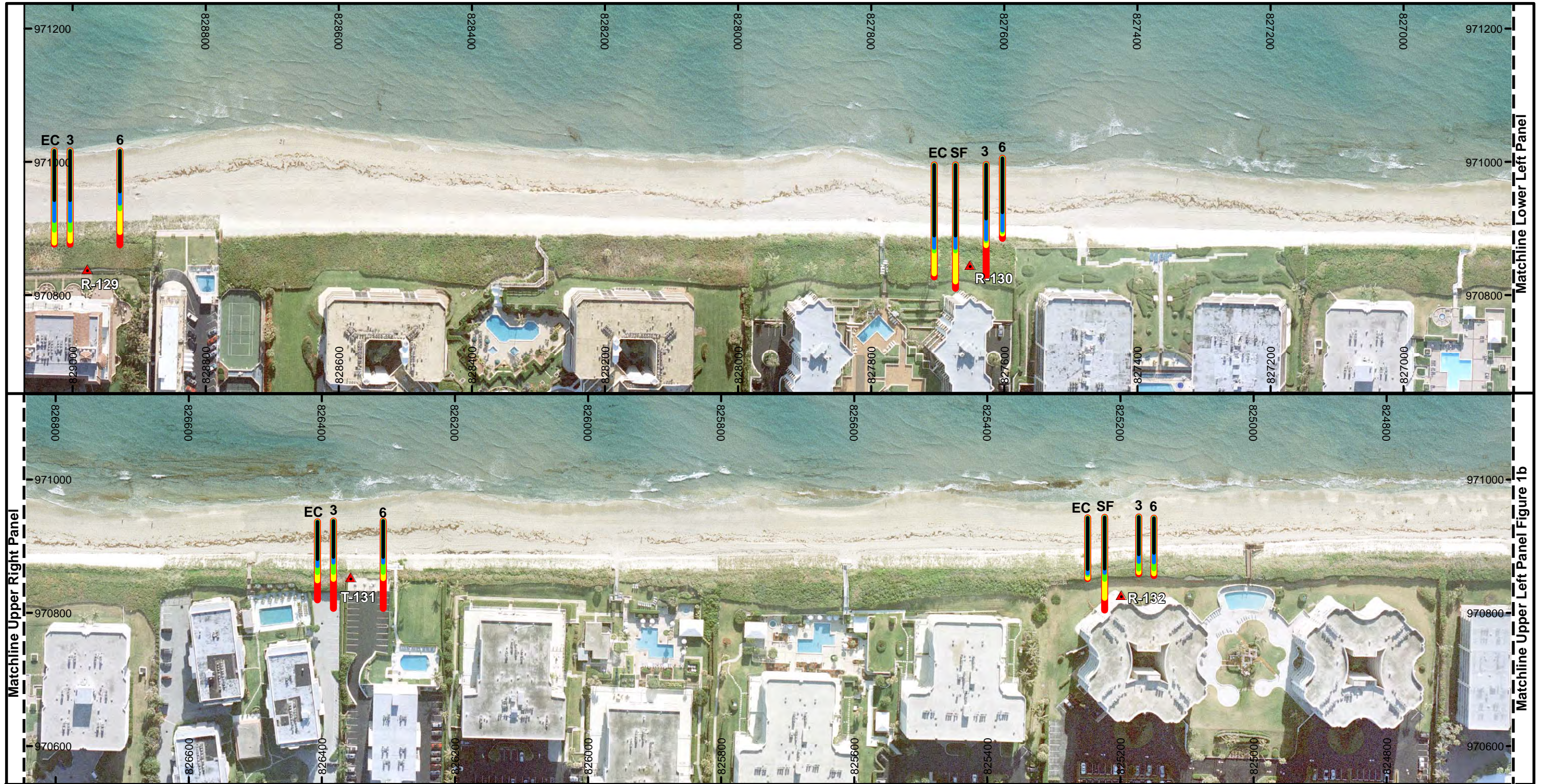




SUB-APPENDIX G-1
SBEACH ANALYSIS REPORT

ATTACHMENT D
LANDWARD LIMIT OF RECESSION BY RETURN PERIOD STORM BASED ON
SBEACH RESULTS

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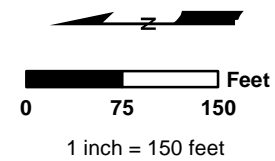
Notes:

1. Coordinates are in feet based on the Florida State Plane Coordinate System, East Zone, North American Datum of 1983 (NAD 83).
2. Aerial photography provided by the Town of Palm Beach, date flown March 30, 2012.

Legend:

- Year 5
- Year 15
- Year 25
- Year 50
- Year 100
- Monuments

EC - Existing Conditions (2011/2012 survey)
SF - Existing Conditions with Seawall Failure
3- Alternative 3
6 - Alternative 6

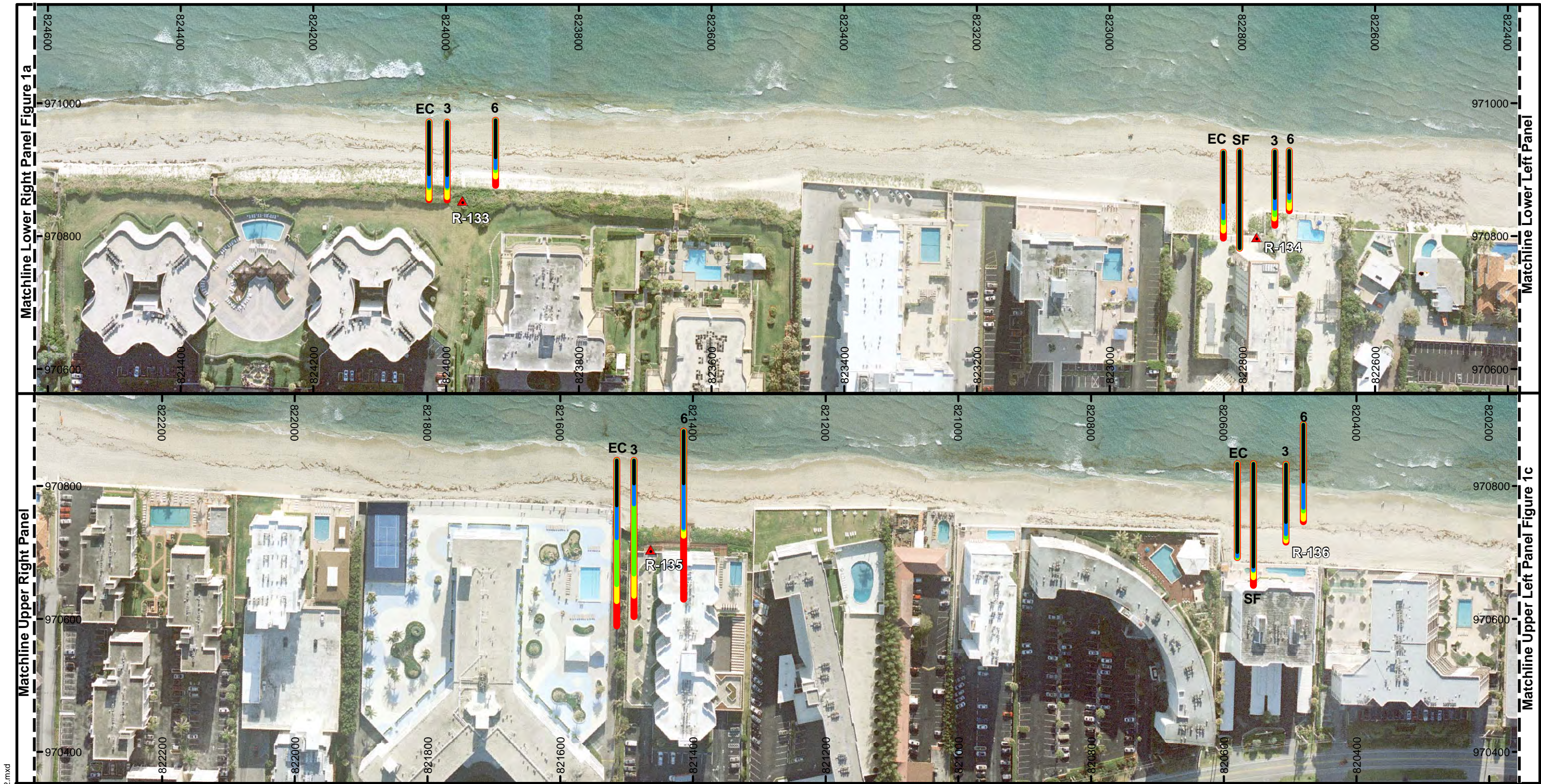


TITLE: **Southern Palm Beach Island
Shoreline Stabilization Project EIS
Landward Limit of Recession
by Return Period Storm
Based on SBEACH Model Results**

COASTAL PLANNING & ENGINEERING, INC.
A CB&I COMPANY
2481 N. W. BOCA RATON BOULEVARD
BOCA RATON, FL 33431
PH. (561) 391-8102 FAX (561) 391-9116

Date: 03/05/14 By: HMV Comm No. : 150814 **Figure No.: 1a**

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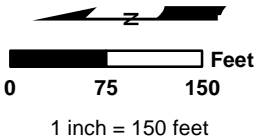
Notes:

- Coordinates are in feet based on the Florida State Plane Coordinate System, East Zone, North American Datum of 1983 (NAD 83).
- Aerial photography provided by the Town of Palm Beach, date flown March 30, 2012.

Legend:

- Year 5
- Year 15
- Year 25
- Year 50
- Year 100
- Monuments

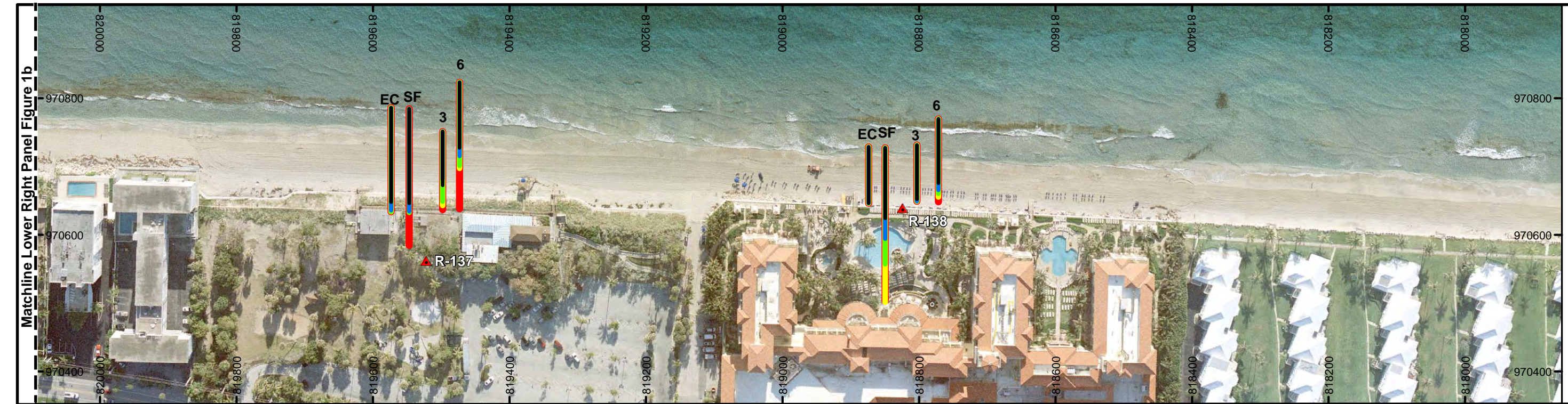
EC - Existing Conditions (2011/2012 survey)
SF - Existing Conditions with Seawall Failure
3- Alternative 3
6 - Alternative 6



TITLE: **Southern Palm Beach Island
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BOCA RATON, FL 33431
PH. (561) 391-8102 FAX (561) 391-9116

Date: 03/05/14 By: HMV Comm No. : 150814 Figure No.: 1b



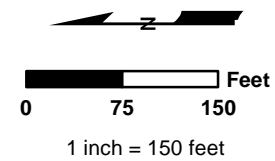
Notes:

1. Coordinates are in feet based on the Florida State Plane Coordinate System, East Zone, North American Datum of 1983 (NAD 83).
2. Aerial photography provided by the Town of Palm Beach, date flown March 30, 2012.

Legend:

- Year 5
- Year 15
- Year 25
- Year 50
- Year 100
- Monuments

EC - Existing Conditions (2011/2012 survey)
SF - Existing Conditions with Seawall Failure
3- Alternative 3
6 - Alternative 6



TITLE: **Southern Palm Beach Island
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Date: 03/05/14 By: HMV Comm No. : 150814 **Figure No.: 1c**

SUB-APPENDIX G-1
SBEACH ANALYSIS REPORT

ATTACHMENT E
SBEACH ADDITIONAL MODELING

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SUB-APPENDIX G-1 – ATTACHMENT E SBEACH ADDITIONAL SCENARIOS

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1.0 INTRODUCTION

Under direction of the U.S. Army Corps of Engineers (USACE), CB&I Coastal Planning & Engineering, Inc. (CB&I) assisted in the development of the Southern Palm Beach Island Comprehensive Shoreline Stabilization Project Environmental Impact Statement (EIS). The USACE determined that additional work was required to consider a range of sediment grain sizes in evaluating the Town of Palm Beach's (Town) request for maintaining flexibility with respect to potential sand sources for the Town's portion of the project area.

One factor contributing to the redistribution of sediments placed within the coastal system is grain size. An additional SBEACH modeling study was conducted to assess the level of storm protection afforded by the proposed alternatives considering a range of sediment grain sizes from potential sand sources identified by the Town. The additional modeling builds upon the previous study presented in Sub-Appendix G-1 that considered a single sediment grain size for the proposed alternatives.

The Florida Department of Environmental Protection (FDEP) Palm Beach Island Beach Management Agreement identifies compliance specifications for beach fill material that "take into account the variability of sediment on the native or existing beach" (FDEP, 2013). The agreement specifies that sediments with mean grain sizes ranging from 0.25 mm to 0.60 mm are acceptable beach fill material. The study presented herein evaluated the proposed alternatives to consider fill material with mean grain sizes of 0.25 mm and 0.60 mm placed within the Town's portion of the project. As such, the model setup and synthetic storm events from the previous modeling study were utilized in the additional modeling study to consider the two grain sizes.

Additional SBEACH modeling was performed for the Town's project area (R-129-210 to R-134+135) to evaluate the following.

- **Existing Conditions:** The existing conditions for the no action scenario (Alternative 1) was updated with more recent survey data (2014) obtained from the FDEP website. Storm vulnerability analysis was conducted for the no action scenario using a grain size of 0.36 mm for the five synthetic storms (return intervals of 5, 15, 25, 50, and 100 years) developed as part of the pervious modeling study. In addition a no seawall analysis was conducted for the existing conditions in the event the seawalls fail.
- **Future Scenario without Project:** Future scenarios without a project were developed by projecting beach conditions in 10 and 50 years and simulated with SBEACH. The existing condition profiles were translated landward based on the background erosion rate of 2.25 feet per year (CPE, 2013).
- **Project Alternatives:** The performance of the project alternatives was also evaluated using grain sizes of 0.25 mm, 0.36 mm, and 0.60 mm for the five synthetic storms events (return intervals of 5, 15, 25, 50, and 100 years) developed as part of the pervious modeling study. Alternatives 3, 6, 7a, and 7b were evaluated in this analysis. Alternatives 2, 4, 5, and 6 were not evaluated in this analysis because the fill template was identical to other alternatives modeled, had similar project template combinations, or due to the models restrictions described in more detail in Section 3.1.

2.0 SBEACH MODEL SETUP

Model setup and calibration described in Sub-Appendix G-1 was used to assess the performance of the additional modeling analysis. Six beach profile locations at FDEP R-monuments between R-129 and R-134 were modeled using SBEACH. To represent the most recent conditions, profile survey data collected in 2014 was used. The datum used during the surveys were the Florida State Plane Coordinate System, North American Datum of 1983. The surveys were converted to the National Geodetic Vertical Datum using Corpscon (ver. 6.x) for consistency of datums throughout the calibration and production runs. The beach profile cross sections were extended landward of the dune system for modeling purposes by appending survey data collected in 1990.

2.1. Grain Size Sensitivity Analysis (0.30 mm and 0.36 mm)

SBEACH modeling analysis was previously conducted using a median grain size of 0.30, which was based on the samples collected in 2006 to characterize the native beach (CPE, 2007). A mean grain size of 0.36mm was determined based on a composite for beach profiles locations between R-124 to R-139 (Palm Beach County, 1993), which was used in other numerical modeling analysis conducted for the Environmental Impact Statement (EIS). For consistency the grain size for the additional SBEACH modeling study was updated to 0.36 mm from 0.30 mm.

Sensitivity of the SBEACH model to the increase in grain size was evaluated with the 2014 beach profiles. During the 15 year storm, sand from the dune and beach (above 0 feet, NGVD) was eroded and deposited in the nearshore (between -5 feet and 0 feet, NGVD). The erosion and deposition trends were consistent for both grain sizes, but with subtle difference in the magnitude of the changes (Figure 2-1).

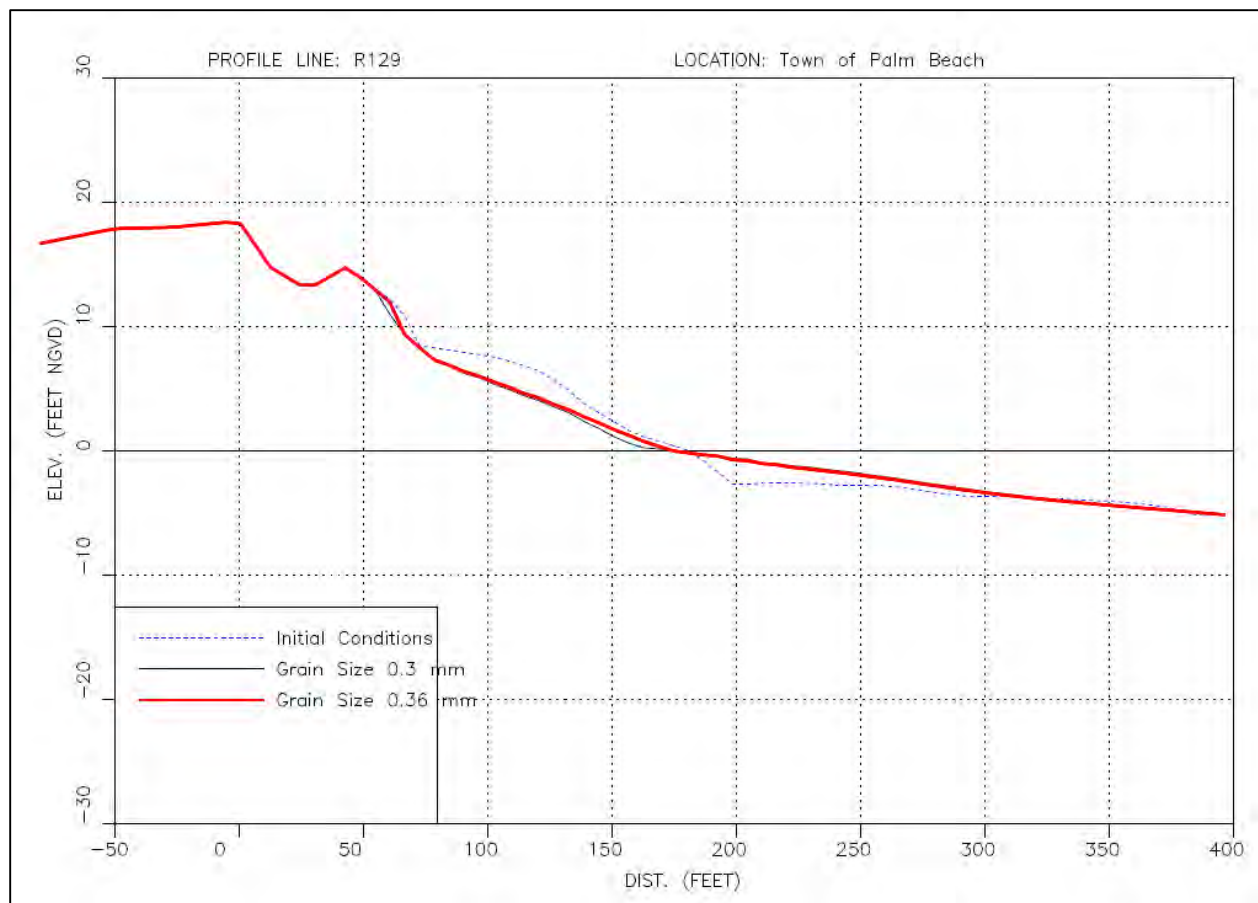


Figure 2-1. Sensitivity of grain size for R-129 profile, 15 year storm.

3.0 MODEL RESULTS

3.1. General

Eight alternatives were considered for the additional modeling study as outlined below.

- 1) No Action Alternative (Status Quo), which includes periodic dune nourishment of the dry beach.
- 2) The Applicants' Preferred Alternative (Proposed Action): Beach and Dune Fill with Shoreline Protection Structures
- 3) The Applicants' Preferred Project without Shoreline Protection Structures
- 4) The Town of Palm Beach Preferred Project and County Increased Sand Volume Project without Shoreline Protection Structures
- 5) The Town of Palm Beach Increased Sand Volume Project and County Preferred Project
- 6) The Town of Palm Beach Increased Sand Volume Project and County Increased Sand Volume without Shoreline Protection Structures Project
- 7) Options identified by the Coalition to Save Our Shoreline, Inc.'s (SOS) to stabilize the beach.
 - 7a) SOS option with increased sand volume utilizing offshore sand sources and Shoreline Protection Structures.
 - 7b) SOS preferred option utilizing upland sand sources and with Shoreline Protection Structures.

SBEACH modeling was conducted for Alternatives 1, 3, 6, 7a and 7b. Alternative 2 was not modeled since the fill design is the same as Alternative 3. SBEACH is a cross-shore transport model and does not consider the influence of groins as present in Alternative

2. Alternatives 4 and 5 were not modeled as they were various combinations of the Town and County projects represented by Alternatives 2 and 6. SBEACH is a cross-shore transport model and does not consider the influence of adjacent fill distributions as presented in Alternatives 4 and 5.

SBEACH results for the additional modeling study appear in Attachment F and include the post-storm profiles for all design storms.

3.2. Existing Conditions (2014 Beach Profiles)

The existing beach conditions within the Project Area consist of eroded dunes, exposed seawalls and steep gradient beach berms. At the time of the 2011/2012 survey, along the Town of Palm Beach there were continuous dune features and vegetation separating the beach from the residential infrastructure. At the time of the 2014 survey, portions of the dune had been eroded exposing seawalls that had not been identified in the previous modeling study. In particular, the seawall at profile T-131 was exposed that had been previously buried during the 2011/2012 survey. A seawall was included in the analysis at profile T-131 for this additional modeling study, whereas the previous modeling study did not include a seawall at this location.

The degree of erosion during a storm varies spatially due to presence of seawalls and the condition of the beach (i.e. dune elevation, beach width, and offshore bars). Seawalls were most exposed at profiles T-131 and R-134 and the profiles experience the most erosion with respect to both volume and MHW changes during the 15 year storm (Table 3-1). In general, greater erosion was experienced south from profile T-131 as compared to the north (Table 3-2; Attachment F-1). The average volume changes above mean low water throughout the Project Area during the 5, 15, 25, 50 and 100-year return interval storms were -4 cy/ft, -6 cy/ft, -6 cy/ft, -7 cy/ft and -8 cy/ft, respectively. Erosion at each profile increased as the magnitude of storms increased.

The landward limit of erosion was quantified to determine the potential impacts to infrastructure and property landward of the Project Area (Table 3-3). According to the

Coast of Florida Erosion and Storms Effect Study Region III (USACE, 1995), the extent of beach erosion is determined to be the region where the post-storm elevation is reduced by 0.5 ft or more. Therefore, the landward limit of erosion was defined as the landward position where at least 0.5 feet of elevation was lost as a result of the storm. The values in Table 3-3 are referenced to the seaward limit of the maintained property, which refers to landscaped areas or paved/ gravel areas. As the profiles eroded landward towards the limit of the maintained property, the values in the table decrease until they retreat landward into the maintained property at which point the values are negative. The table values in red signify that recession landward of the improved or maintained property occurred.

Under existing conditions, damage to property landward of the Project Area was simulated at profiles T-131, R-133, and R-134 for storms with return intervals of 15 years and greater. The landward limit of erosion at profiles T-131 and R-134 became constant with increasing return period storms except in the presence of exposed seawalls that prevented erosion further landward.

To consider damages in the event that seawall failure occurred, simulations performed without including seawalls in the SBEACH model. Damage to property landward of the Project Area was simulated at profiles T-131 and R-134 for storms with return intervals of 5 years and greater.

Table 3-1. SBEACH shoreline and volumetric changes for existing conditions (2014) and a 15 year storm.

| Profile | MHW Change (ft) | Volume Change above MLW (cy/ft) |
|---------|-----------------|---------------------------------|
| R-129 | -6 | -5 |
| R-130 | -6 | -4 |
| T-131 | -18 | -8 |
| R-132 | -8 | -6 |
| R-133 | -11 | -5 |
| R-134 | -18 | -7 |

NOTE: Mean High Water (MHW) = 2' NGVD
Mean Low Water (MLW) = -0.73' NGVD.

Table 3-2. SBEACH volume change existing conditions (2014).

| Profile | 5 Year Storm Volume Change above MLW (cy/ft) | 15 Year Storm Volume Change above MLW (cy/ft) | 25 Year Storm Volume Change above MLW (cy/ft) | 50 Year Storm Volume Change above MLW (cy/ft) | 100 Year Storm Volume Change above MLW (cy/ft) |
|---------|--|---|---|---|--|
| R-129 | -4 | -5 | -6 | -6 | -7 |
| R-130 | -3 | -4 | -4 | -5 | -6 |
| T-131 | -6 | -8 | -8 | -9 | -9 |
| R-132 | -4 | -6 | -6 | -7 | -8 |
| R-133 | -4 | -5 | -5 | -6 | -7 |
| R-134 | -6 | -7 | -8 | -8 | -9 |
| Average | -5 | -6 | -6 | -7 | -8 |

NOTE: Mean Low Water (MLW) = -0.73' NGVD.

Table 3-3. SBEACH landward limit of storm erosion, existing conditions (2014).

| FDEP R-Monument ¹ | Simulation ID | Landward Limit of Storm Erosion ² (feet from seaward edge of maintained property) Given Return Period in Years: | | | | |
|------------------------------|-------------------------------------|---|-----|-----|-----|-----|
| | | 5 | 15 | 25 | 50 | 100 |
| R-129 | Existing Conditions 2014 (0.36) | 79 | 56 | 50 | 30 | -17 |
| R-130 | Existing Conditions 2014 (0.36) | 76 | 54 | 37 | 31 | 4 |
| | Seawall Failure ³ (0.36) | 76 | 54 | 37 | 31 | -8 |
| T-131 | Existing Conditions 2014 (0.36) | 18 | -8 | -10 | -11 | -11 |
| | Seawall Failure ³ (0.36) | -17 | -17 | -23 | -43 | -58 |
| R-132 | Existing Conditions 2014 (0.36) | 17 | 11 | 11 | 2 | -9 |
| | Seawall Failure ³ (0.36) | 17 | 11 | 11 | -4 | -35 |
| R-133 | Existing Conditions 2014 (0.36) | 24 | -5 | -13 | -20 | -31 |
| | Seawall Failure ³ (0.36) | 24 | -5 | -13 | -34 | -56 |
| R-134 | Existing Conditions 2014 (0.36) | -1 | -1 | -2 | -2 | -2 |
| | Seawall Failure ³ (0.36) | -17 | -17 | -17 | -17 | -75 |

¹Profile R-129 does not have a seawall.²Values bolded in red represent erosion landward of the edge of maintained or improved property or infrastructure. Cells shaded yellow represent exposed seawalls.³Simulations run assuming seawall had failed.

3.3. Future Scenario without Project Conditions

Evaluating the existing conditions alone does not provide a complete perspective of the beach response to storms without a project. Based on the erosional trend along the Project Area, the beach profile is likely to continue receding and lowering in elevation. To represent future scenarios without a project, 10-year and 50-year projections of

beach profiles were developed and simulated with SBEACH. The existing condition profiles were translated landward based on a background erosion rate of 2.25 feet per year (CPE, 2013). Seawalls were included in the future scenarios as they were in the existing conditions simulations.

The landward limits of erosion for the future scenarios are presented in Table 3-4. Based on the future scenario simulations, all storm protection provided by the dune throughout the Town is lost. Seawalls that were buried within the dune have become exposed by year 50 and are subject to wave action.

Table 3-4 SBEACH landward limit of storm erosion future scenario.

| FDEP R-Monument ¹ | Simulation ID (years in the future) | Landward Limit of Storm Erosion ² (feet from seaward edge of maintained property) Given Return Period in Years: | | | | |
|------------------------------|--|---|-----|-----|-----|------|
| | | 5 | 15 | 25 | 50 | 100 |
| R-129 | 10 | 56 | 32 | 26 | 7 | -41 |
| | 50 | -34 | -58 | -64 | -83 | -132 |
| R-130 | 10 | 53 | 32 | 13 | 8 | -5 |
| | 50 | -23 | -24 | -24 | -24 | -24 |
| T-131 | 10 | -10 | -11 | -11 | -11 | -11 |
| | 50 | -11 | -11 | -11 | -11 | -11 |
| R-132 | 10 | -7 | -12 | -13 | -15 | -20 |
| | 50 | -26 | -26 | -26 | -26 | -26 |
| R-133 | 10 | 1 | -28 | -31 | -31 | -31 |
| | 50 | -31 | -31 | -31 | -32 | -32 |
| R-134 | 10 | -1 | -2 | -2 | -2 | -2 |
| | 50 | -2 | -2 | -2 | -2 | -2 |

¹Profile R-129 does not have a seawall.

²Values bolded in red represent erosion landward of the edge of maintained or improved property or infrastructure.

3.4. Project Alternatives

3.4.1. Alternative 3: Applicants' Preferred Project without Shoreline Protection Structures

The Applicants' Preferred Alternative fill design consists of dune only and dune and berm fill from R-129-210 to R-134+135. No fill was simulated at R-129 since the existing conditions met the design criteria for the seaward dune extent.

In general, for a grain size of 0.25 mm, 0.36 mm, and 0.6 mm the project provides storm protection against a 15-year storm with little to no impact to the pre-construction profile (Table 3-5 to Table 3-7). Under the occurrence of a 5, 15 and 25-year storm, the frontal dunes retained their shape but lost volume. Recession into the pre-construction profile increases with increasing magnitude of return interval storm.

The landward limit of erosion was quantified for each grain size and for each alternative to determine the potential impacts to infrastructure and property landward of the Project Area. Based on the landward limit of erosion calculation, for alternative 3 with a grain size of 0.25 mm, 0.36 mm, and 0.6 mm damage to property is possible adjacent to profile R-133 as a result of a 50-year return interval or stronger storm (Table 3-5 to Table 3-7). Likewise, property along profiles R-129 are at risk of damage during the occurrence of a 100-year return interval storm or stronger storm for a grain size of 0.25 mm, 0.36 mm, and 0.6 mm. Property along profile T-131 is at risk of damage during the occurrence of a 50-year return interval storm or stronger storm only for a grain size of 0.25 and 0.36 but not for the coarser grain size of 0.6 mm (Table 3-5 and Table 3-6).

As shown in Figure 3-1, beach profiles simulated with a finer fill sediment grain size of 0.25 mm experienced higher erosion above MLW (-0.73 feet, NGVD) than the beach profiles simulate with a coarser grain size of 0.36 and 0.6 mm. Profiles with coarser grain size experienced less cross-shore redistribution of the sand during model simulations as compared to the profiles with finer grain sizes.

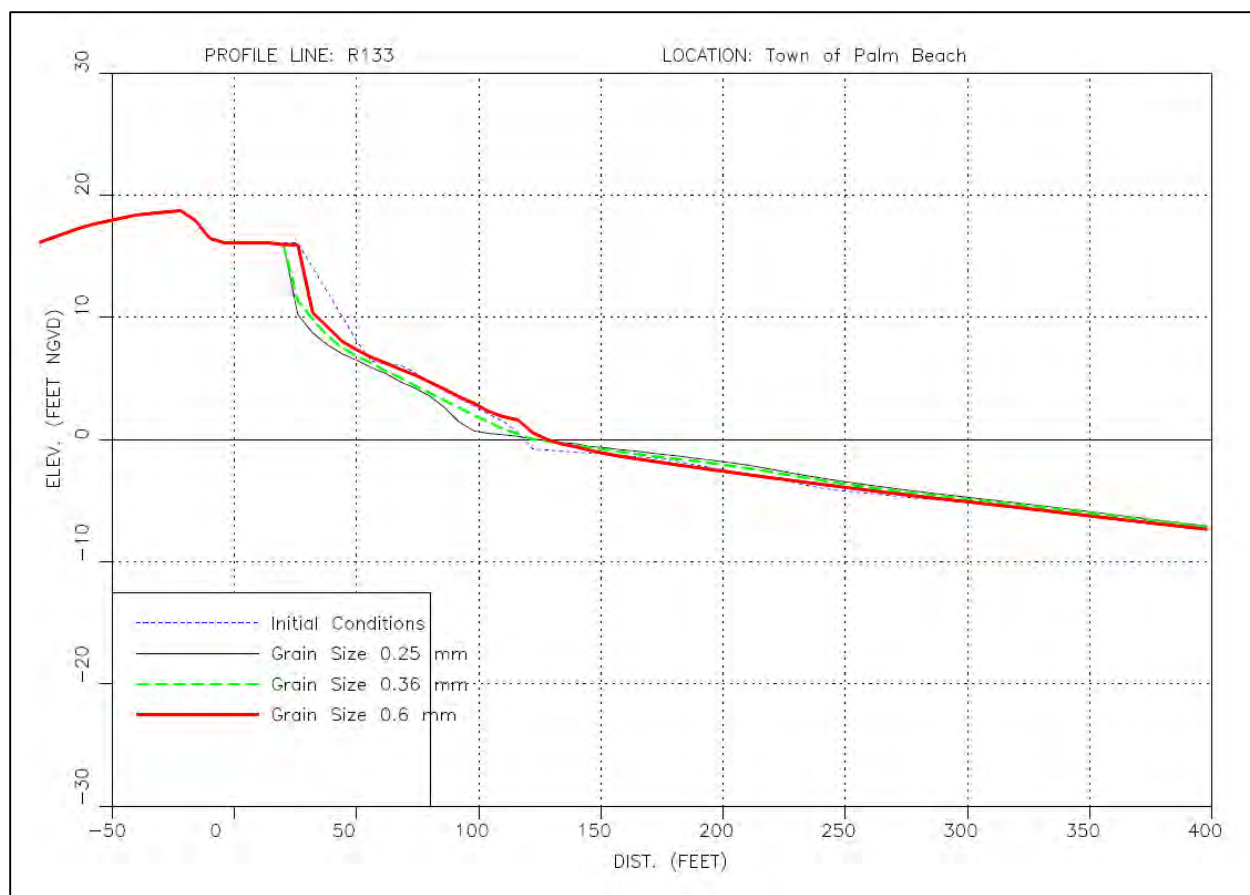


Figure 3-1. Sensitivity of grain size for R-133 profile, 15 year storm, project Alternative 3.

3.4.2. Alternative 6: The Town of Palm Beach Increased Sand Volume Project and County Increased Sand Volume Project without Shoreline Protection Structures

Alternative 6 consists of a wider dune and beach fill templates at profiles R-129-210 through R-134+135. The project prevents scouring at the toe of the seawalls at all locations (Attachment F-8). None of the buried seawalls were exposed as a result of the return interval storms. No seawall failures were observed during the simulations.

The landward limit of erosion was quantified for each grain size and for each alternative to determine the potential impacts to infrastructure and property landward of the Project Area. In general, for a grain size of 0.25 mm, 0.36 mm, and 0.6 mm, the project

provides storm protection against a 15-year storm with little to no impact to the pre-construction profile.

Based on the landward limit of erosion calculation, for alternative 6 with a grain size of 0.25 mm, 0.36 mm, and 0.6 mm damage to property is possible adjacent to profile R-129 as a result of a 100-year return interval or stronger storm (Table 3-5 to Table 3-7). Property along profile T-131 is at risk of damage during the occurrence of a 50-year return interval storm or stronger storm only for a grain size of 0.25 mm and 0.36 mm but not for grain size of 0.6 mm since it is a slightly coarser grain size (Table 3-5 and Table 3-6). Damage of property is also possible adjacent to profile R-132 as a result of a 100 year storm or stronger storm only for a grain size of 0.25 mm.

3.4.3. Alternative 7a: Coalition to Save Our Shoreline, Inc. (SOS) Plan for Offshore Sand Sources.

Alternative 7a consists of placement of sand within the Town of Palm Beach and shoreline protection structures (T-head groins). Two T-head groins were included between R-132 and R-134. The sand fill volumes required for the SOS plan are greater than the volumes for Alternative 6 within the Town of Palm Beach. The sand volume within the Town of Palm Beach was increased by advancing the dune on average 30 feet from R-129-210 to T-131, advancing the beach berm on average 70 feet seaward from R-129-210 to T-131, and including a beach berm with an average width of 135 feet from R-130 to R-134 as compared to Alternative 2. Within the County the sand fill volumes and shoreline protection structures for Alternative 7a were the same as that for Alternative 2.

For the range of grain sizes analyzed, the project provides storm protection against a 15-year storm with little to no impact to the pre-construction profile. Recession into the pre-construction profile increases with increasing magnitude of return interval storm. In general, the landward limit of erosion (Table 3-5 to Table 3-7) was positioned further seaward with increasing in sediment grain size for a given return period storm. Profiles

with coarser grain size experienced less cross-shore redistribution of the sand during model simulations as compared to the profiles with finer grain sizes.

3.4.4. Alternative 7b: Coalition to Save Our Shoreline, Inc. (SOS) Preferred Plan for Upland Sand Sources.

Alternative 7b is the preferred project plan provided by the Coalition to Save Our Shoreline, Inc. for the Town of Palm Beach. The preferred plan places approximately 166,500 CY of high quality beach compatible sand. The preferred project plan has two structures (groins or T-head groins constructed of sheet pile) at the southern end to reduce sand losses from the south end.

Based on the landward limit of erosion calculation, for alternative 7b with a grain size of 0.25 mm, 0.36 mm, and 0.6 mm, damage to property is possible adjacent to profile R-129 as a result of a 100-year return interval or stronger storm (Table 3-5 to Table 3-7).

In general, for the range of grain sizes analyzed, the project provides storm protection against a 15-year storm with little to no impact to the pre-construction profile.

Table 3-5. SBEACH landward limit of storm erosion grain size 0.25 mm.

| FDEP R-Monument ¹ | Simulation ID | Landward Limit of Storm Erosion ³ (feet from seaward edge of maintained property) Given Return Period in Years: | | | | |
|---------------------------------|-----------------------|--|-----|-----|-----------|------------|
| | | 5 | 15 | 25 | 50 | 100 |
| R-129 | Alternative 3 (0.25) | 74 | 51 | 44 | 30 | -13 |
| | Alternative 6 (0.25) | 99 | 85 | 69 | 51 | -16 |
| | Alternative 7a (0.25) | 124 | 110 | 83 | 67 | 34 |
| | Alternative 7b (0.25) | 91 | 76 | 57 | 38 | -1 |
| R-130 | Alternative 3 (0.25) | 80 | 57 | 56 | 41 | 2 |
| | Alternative 6 (0.25) | 92 | 73 | 67 | 52 | 44 |
| | Alternative 7a (0.25) | 138 | 124 | 99 | 70 | 3 |
| | Alternative 7b (0.25) | 89 | 74 | 69 | 27 | 22 |
| T-131 | Alternative 3 (0.25) | 18 | 7 | 3 | -5 | -11 |
| | Alternative 6 (0.25) | 19 | 8 | 3 | -5 | -11 |
| | Alternative 7a (0.25) | 99 | 87 | 83 | 37 | 28 |
| | Alternative 7b (0.25) | 83 | 72 | 68 | 27 | 21 |
| R-132 | Alternative 3 (0.25) | 46 | 37 | 19 | 17 | 7 |
| | Alternative 6 (0.25) | 46 | 37 | 35 | 17 | -2 |
| | Alternative 7a (0.25) | 122 | 110 | 106 | 69 | 58 |
| | Alternative 7b (0.25) | 94 | 71 | 68 | 56 | 31 |
| R-133 | Alternative 3 (0.25) | 23 | 10 | 5 | -9 | -16 |
| | Alternative 6 (0.25) | 59 | 45 | 43 | 18 | 8 |
| | Alternative 7a (0.25) | 100 | 58 | 57 | 37 | 32 |
| | Alternative 7b (0.25) | 80 | 68 | 36 | 21 | 13 |
| R-134 | Alternative 3 (0.25) | 42 | 26 | 28 | 12 | 5 |
| | Alternative 6 (0.25) | 70 | 52 | 45 | 34 | 27 |
| | Alternative 7a (0.25) | 42 | 26 | 25 | 12 | 5 |
| | Alternative 7b (0.25) | 41 | 26 | 26 | 8 | 4 |

¹Profile R-129 does not have a seawall.

²Values bolded in red represent erosion landward of the edge of maintained or improved property or infrastructure.

Table 3-6. SBEACH landward limit of storm erosion grain size 0.36 mm.

| FDEP R-Monument ¹ | Simulation ID | Landward Limit of Storm Erosion ² (feet from seaward edge of maintained property) Given Return Period in Years: | | | | |
|---------------------------------|-----------------------|--|-----|-----|-----------|------------|
| | | 5 | 15 | 25 | 50 | 100 |
| R-129 | Alternative 3 (0.36) | 79 | 56 | 50 | 32 | -17 |
| | Alternative 6 (0.36) | 104 | 91 | 69 | 51 | -21 |
| | Alternative 7a (0.36) | 127 | 115 | 83 | 36 | 34 |
| | Alternative 7b (0.36) | 86 | 74 | 57 | 34 | -15 |
| R-130 | Alternative 3 (0.36) | 82 | 57 | 56 | 42 | 6 |
| | Alternative 6 (0.36) | 95 | 73 | 67 | 51 | 4 |
| | Alternative 7a (0.36) | 142 | 128 | 88 | 69 | 63 |
| | Alternative 7b (0.36) | 102 | 88 | 84 | 38 | 32 |
| T-131 | Alternative 3 (0.36) | 17 | 6 | 3 | -2 | -5 |
| | Alternative 6 (0.36) | 18 | 7 | 3 | -3 | -6 |
| | Alternative 7a (0.36) | 99 | 34 | 89 | 36 | 30 |
| | Alternative 7b (0.36) | 75 | 69 | 65 | 19 | 14 |
| R-132 | Alternative 3 (0.36) | 48 | 41 | 36 | 12 | 9 |
| | Alternative 6 (0.36) | 48 | 41 | 35 | 12 | 8 |
| | Alternative 7a (0.36) | 126 | 115 | 112 | 70 | 60 |
| | Alternative 7b (0.36) | 82 | 56 | 53 | 42 | 13 |
| R-133 | Alternative 3 (0.36) | 29 | 11 | 5 | -9 | -14 |
| | Alternative 6 (0.36) | 59 | 43 | 37 | 23 | 13 |
| | Alternative 7a (0.36) | 105 | 56 | 53 | 44 | 32 |
| | Alternative 7b (0.36) | 72 | 63 | 59 | 15 | 4 |
| R-134 | Alternative 3 (0.36) | 43 | 30 | 27 | 16 | 11 |
| | Alternative 6 (0.36) | 62 | 53 | 44 | 37 | 31 |
| | Alternative 7a (0.36) | 43 | 28 | 24 | 15 | 7 |
| | Alternative 7b (0.36) | 43 | 30 | 23 | 11 | 6 |

¹Profile R-129 does not have a seawall.

²Values bolded in red represent erosion landward of the edge of maintained or improved property or infrastructure.

Table 3-7. SBEACH landward limit of storm erosion grain size 0.60 mm.

| FDEP R-Monument ¹ | Simulation ID | Landward Limit of Storm Erosion ² (feet from seaward edge of maintained property) Given Return Period in Years: | | | | |
|---------------------------------|----------------------|--|-----|-----|-----------|------------|
| | | 5 | 15 | 25 | 50 | 100 |
| R-129 | Alternative 3 (0.6) | 90 | 57 | 52 | 34 | -20 |
| | Alternative 6 (0.6) | 111 | 105 | 101 | 49 | -21 |
| | Alternative 7a (0.6) | 134 | 124 | 69 | 36 | -15 |
| | Alternative 7b (0.6) | 99 | 87 | 84 | 80 | -11 |
| R-130 | Alternative 3 (0.6) | 89 | 58 | 58 | 49 | 5 |
| | Alternative 6 (0.6) | 104 | 80 | 73 | 57 | 4 |
| | Alternative 7a (0.6) | 150 | 88 | 78 | 65 | 5 |
| | Alternative 7b (0.6) | 94 | 87 | 84 | 29 | 25 |
| T-131 | Alternative 3 (0.6) | 18 | 9 | 9 | 6 | 0 |
| | Alternative 6 (0.6) | 9 | 8 | 9 | 6 | 0 |
| | Alternative 7a (0.6) | 108 | 32 | 30 | 31 | 31 |
| | Alternative 7b (0.6) | 95 | 29 | 29 | 28 | 26 |
| R-132 | Alternative 3 (0.6) | 53 | 14 | 41 | 13 | 12 |
| | Alternative 6 (0.6) | 53 | 14 | 41 | 13 | 8 |
| | Alternative 7a (0.6) | 133 | 124 | 124 | 79 | 16 |
| | Alternative 7b (0.6) | 107 | 81 | 70 | 33 | 30 |
| R-133 | Alternative 3 (0.6) | 29 | 17 | 11 | -1 | -9 |
| | Alternative 6 (0.6) | 65 | 21 | 37 | 25 | 13 |
| | Alternative 7a (0.6) | 113 | 38 | 48 | 35 | 33 |
| | Alternative 7b (0.6) | 88 | 83 | 81 | 29 | 21 |
| R-134 | Alternative 3 (0.6) | 40 | 32 | 28 | 21 | 16 |
| | Alternative 6 (0.6) | 61 | 59 | 52 | 43 | 31 |
| | Alternative 7a (0.6) | 40 | 31 | 30 | 21 | 16 |
| | Alternative 7b (0.6) | 40 | 31 | 28 | 20 | 15 |

¹Profile R-129 does not have a seawall.

²Values bolded in red represent erosion landward of the edge of maintained or improved property or infrastructure.

4.0 CONCLUSIONS

Additional SBEACH modeling was performed for the Town of Palm Beach project area (R-129-210 to R-134+135) that considered the 2014 beach profiles surveys obtained from the FDEP website for the project area. Storm vulnerability analysis was conducted for the no action scenario using a grain size of 0.36 mm for the five synthetic storms with return intervals of 5, 15, 25, 50, and 100 years developed under the initial work. The performance within the project area with the project alternatives 3, 6, 7a and 7b in place was also evaluated for a range of grain sizes (0.25 mm, 0.36 mm and 0.60 mm) for the same five synthetic storms. Model calibration setup described in Sub-Appendix G-1 was used.

The following conclusions were made based on the results of this additional modeling study:

- The 2014 survey were used as existing conditions for the additional modeling study, which represented a more eroded beach condition as compared to the 2011/2012 survey used in the previous modeling study. This eroded conditions provided less storm protection and damages to property landward of the Project Area were simulated at profiles T-131, R-133 and R-134 for storms with 15 return periods and greater for all alternatives evaluated.
- Model simulations with sediment grain size of 0.25 mm showed small difference in erosion as compared to simulations with grain sizes of 0.36 mm and 0.60 mm. Beach profiles simulated with a finer fill sediment grain size of 0.25 mm experienced higher erosion above MLW (-0.73 feet, NGVD) than the beach profiles simulate with a coarser grain size of 0.36 and 0.6 mm. Profiles with coarser grain size experienced less cross-shore redistribution of the sand during model simulations as compared to the profiles with finer grain sizes.
- Landward limit of erosion was positioned further seaward with increasing in sediment grain size for a given return period storm.

5.0 LITERATURE CITED

Coastal Planning & Engineering. 2007. Town of Palm Beach Reach 8 Beach Nourishment Project Response to RAI #3 JCP File Number 0250572-001-JC; July 2007; CPE02829-03802.

Coastal Planning & Engineering. 2013. Central Palm Beach County Comprehensive Erosion Control Project Reformulated Shore Protection Alternatives. Prepared for Palm Beach County. 109 p.

Florida Department of Environmental Protection. 2013. Palm Beach Island Beach Management Agreement. <http://www.dep.state.fl.us/BEACHES/pb-bma/index.htm>

Palm Beach County, 1993. Environmental Assessment of Coastal Resources in Palm Beach, Lake Worth, South Palm Beach, Lantana, and Manalapan, Palm Beach County, Florida, Palm Beach County, West Palm Beach, FL.

U.S. Army Corps of Engineers (USACE). 1995. Coast of Florida Erosion and Storm Effects Study Region III, U.S. Army Corps of Engineers, Jacksonville, FL.

SUB-APPENDIX G-1
SBEACH ANALYSIS REPORT

ATTACHMENT F
SBEACH MODEL RESULTS

SUB-APPENDIX G-1
SBEACH ANALYSIS REPORT

ATTACHMENT F-1
EXISTING CONDITIONS (2014 SURVEY) GRAIN SIZE 0.36

SUB-APPENDIX G-1
SBEACH ANALYSIS REPORT

ATTACHMENT F-2
EXISTING CONDITIONS (2014 SURVEY) GRAIN SIZE 0.36
NO SEAWALL/SEAWALL FAILURE

SUB-APPENDIX G-1
SBEACH ANALYSIS REPORT

ATTACHMENT F-3
ALTERNATIVE 3 (APPLICANTS' PREFERRED) GRAIN SIZE 0.25

SUB-APPENDIX G-1
SBEACH ANALYSIS REPORT

ATTACHMENT F-4
ALTERNATIVE 3 (APPLICANTS' PREFERRED) GRAIN SIZE 0.36

SUB-APPENDIX G-1
SBEACH ANALYSIS REPORT

ATTACHMENT F-5
ALTERNATIVE 3 (APPLICANTS' PREFERRED) GRAIN SIZE 0.6

SUB-APPENDIX G-1
SBEACH ANALYSIS REPORT

ATTACHMENT F-6
ALTERNATIVE 6 (LARGER FILL DESIGN) GRAIN SIZE 0.25

SUB-APPENDIX G-1
SBEACH ANALYSIS REPORT

ATTACHMENT F-7
ALTERNATIVE 6 (LARGER FILL DESIGN) GRAIN SIZE 0.36

SUB-APPENDIX G-1
SBEACH ANALYSIS REPORT

ATTACHMENT F-8
ALTERNATIVE 6 (LARGER FILL DESIGN) GRAIN SIZE 0.6

SUB-APPENDIX G-1
SBEACH ANALYSIS REPORT

ATTACHMENT F-9
ALTERNATIVE 7a (SOS) GRAIN SIZE 0.25

SUB-APPENDIX G-1
SBEACH ANALYSIS REPORT

ATTACHMENT F-10
ALTERNATIVE 7a (SOS) GRAIN SIZE 0.36

SUB-APPENDIX G-1
SBEACH ANALYSIS REPORT

ATTACHMENT F-11
ALTERNATIVE 7a (SOS) GRAIN SIZE 0.6

SUB-APPENDIX G-1
SBEACH ANALYSIS REPORT

ATTACHMENT F-12
ALTERNATIVE 7b (SOS) GRAIN SIZE 0.25

SUB-APPENDIX G-1
SBEACH ANALYSIS REPORT

ATTACHMENT F-13
ALTERNATIVE 7b (SOS) GRAIN SIZE 0.36

SUB-APPENDIX G-1
SBEACH ANALYSIS REPORT

ATTACHMENT F-14
ALTERNATIVE 7a (SOS) GRAIN SIZE 0.6

SUB-APPENDIX G-1
SBEACH ANALYSIS REPORT

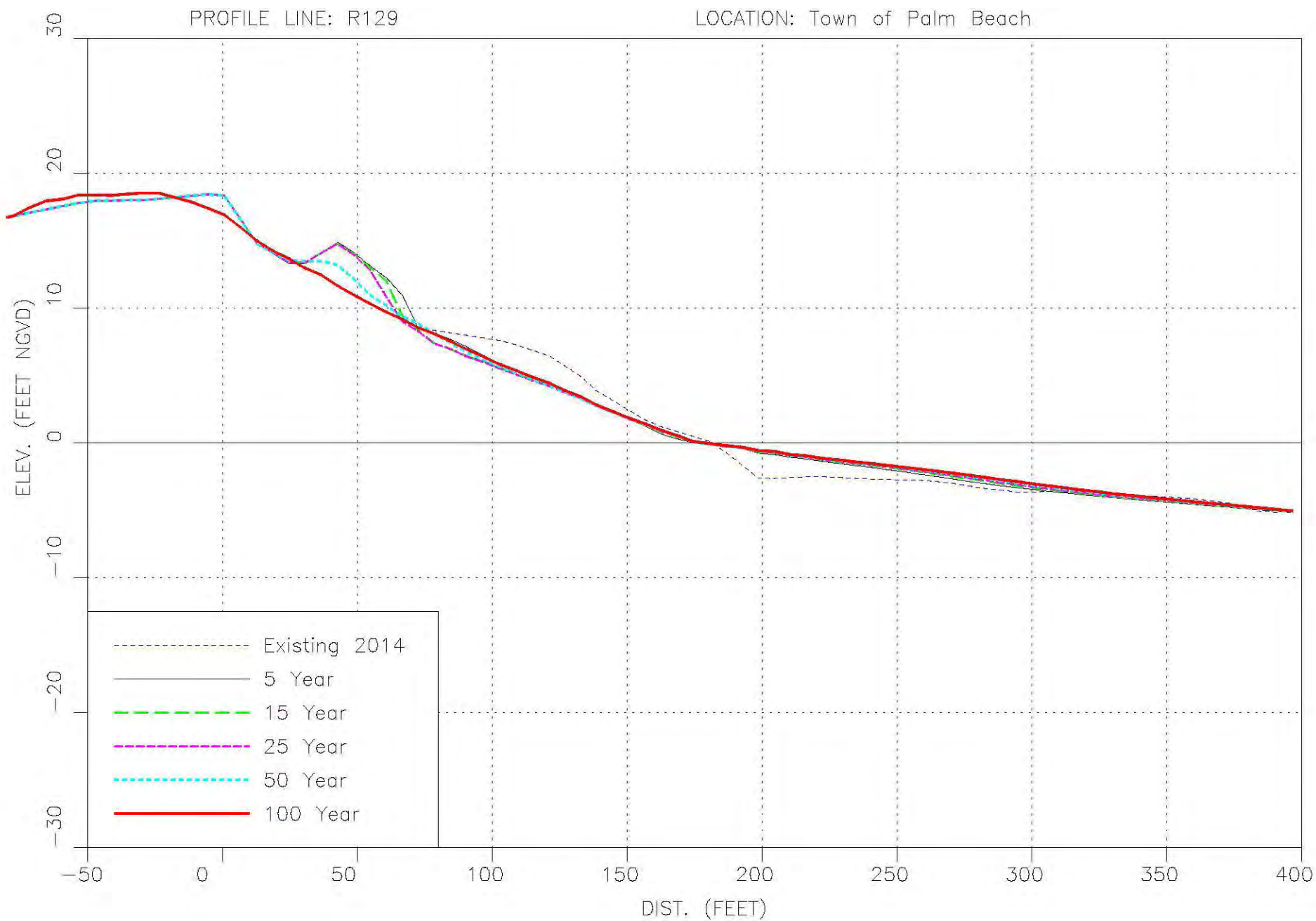
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FUTURE SCENARIO (WITHOUT PROJECT CONDITIONS)

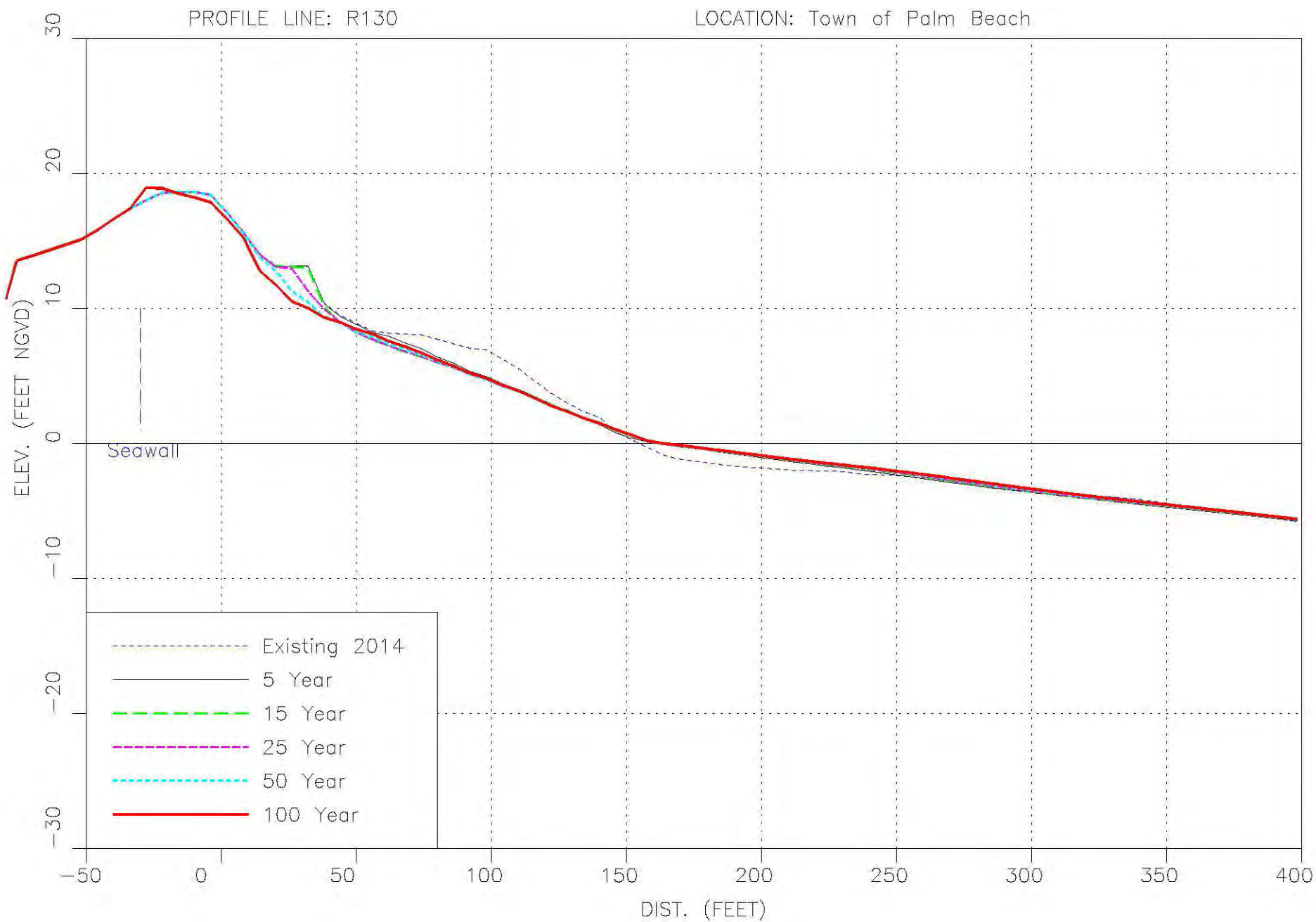
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SBEACH ANALYSIS REPORT

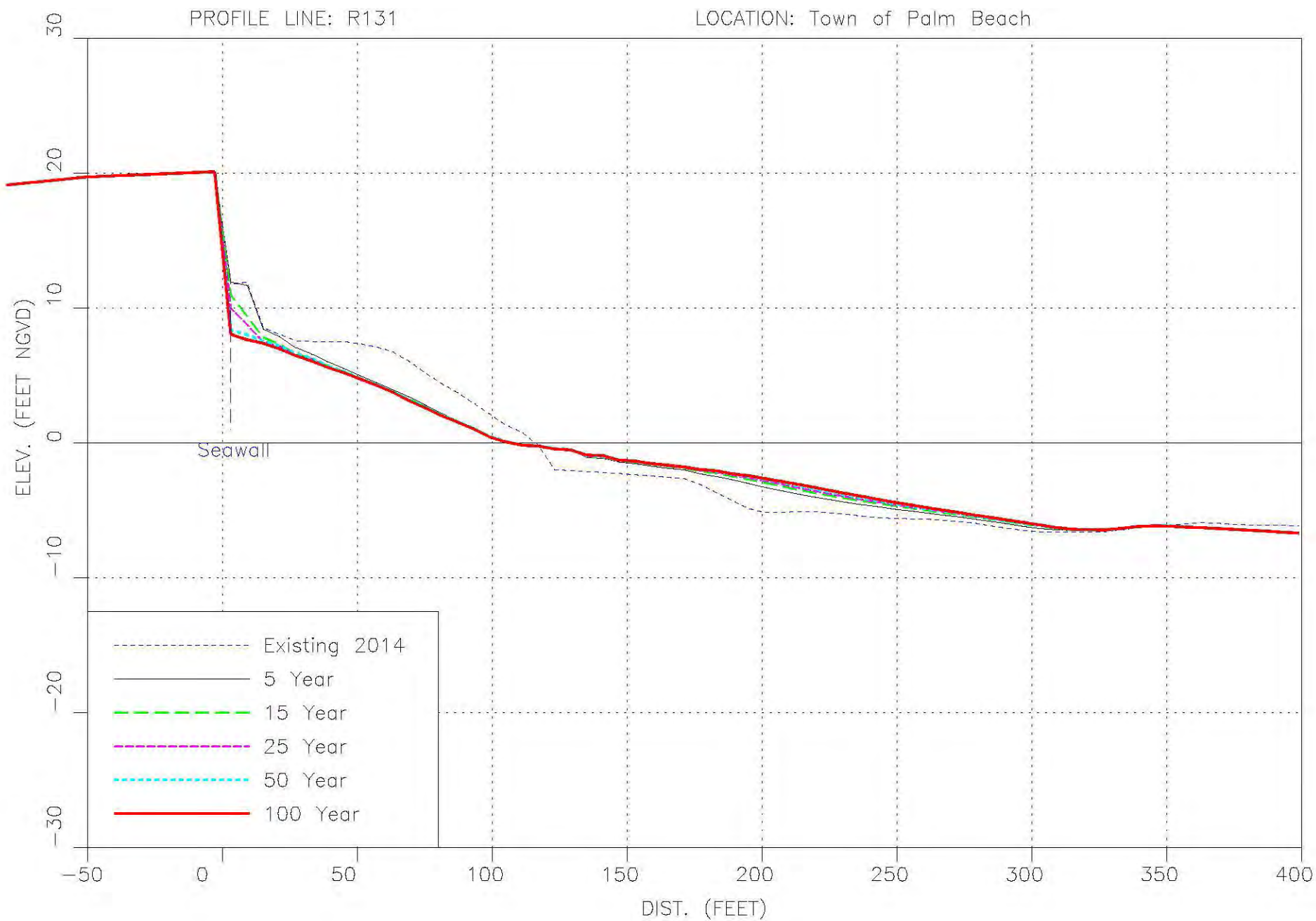
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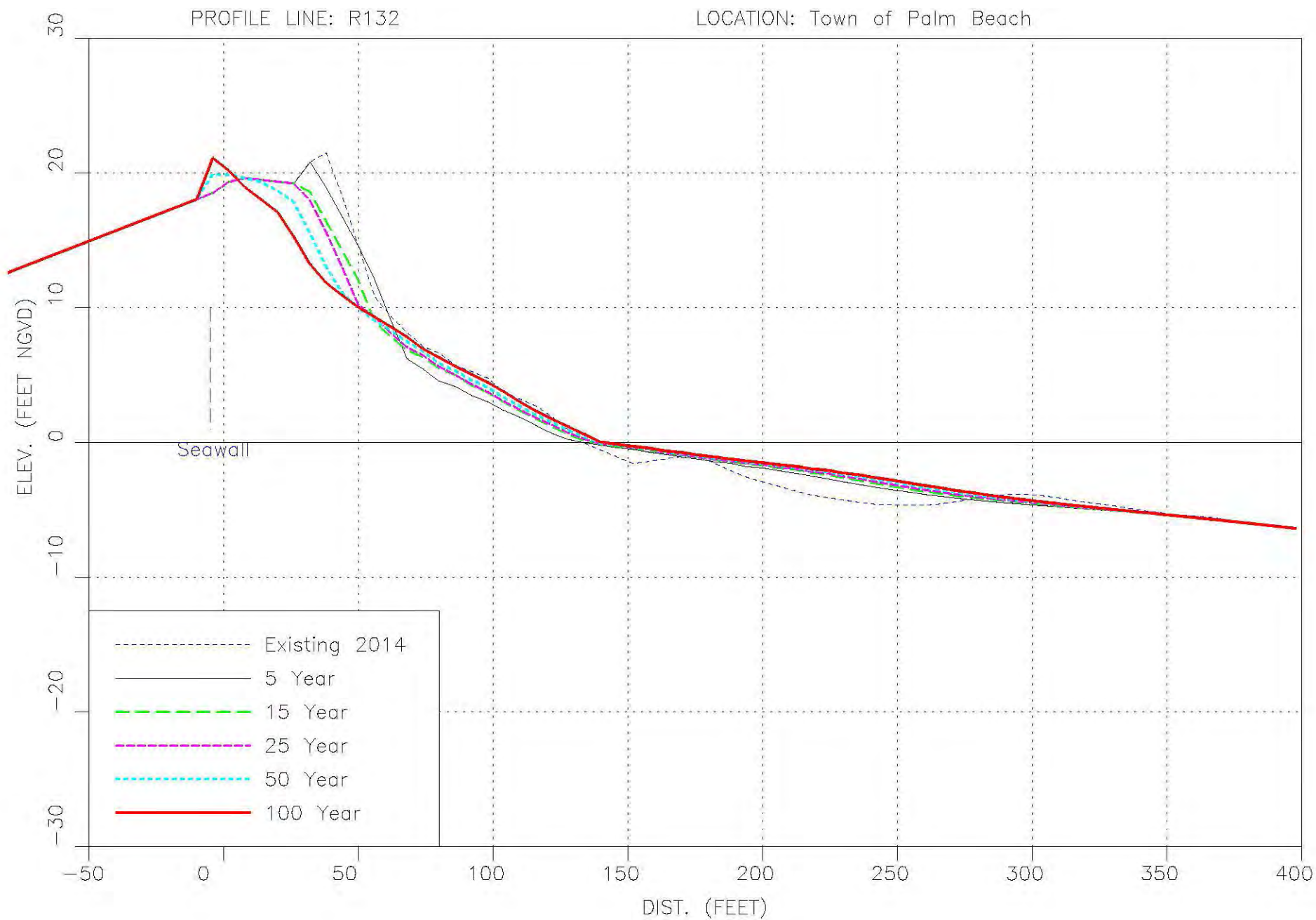
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SBEACH ANALYSIS REPORT

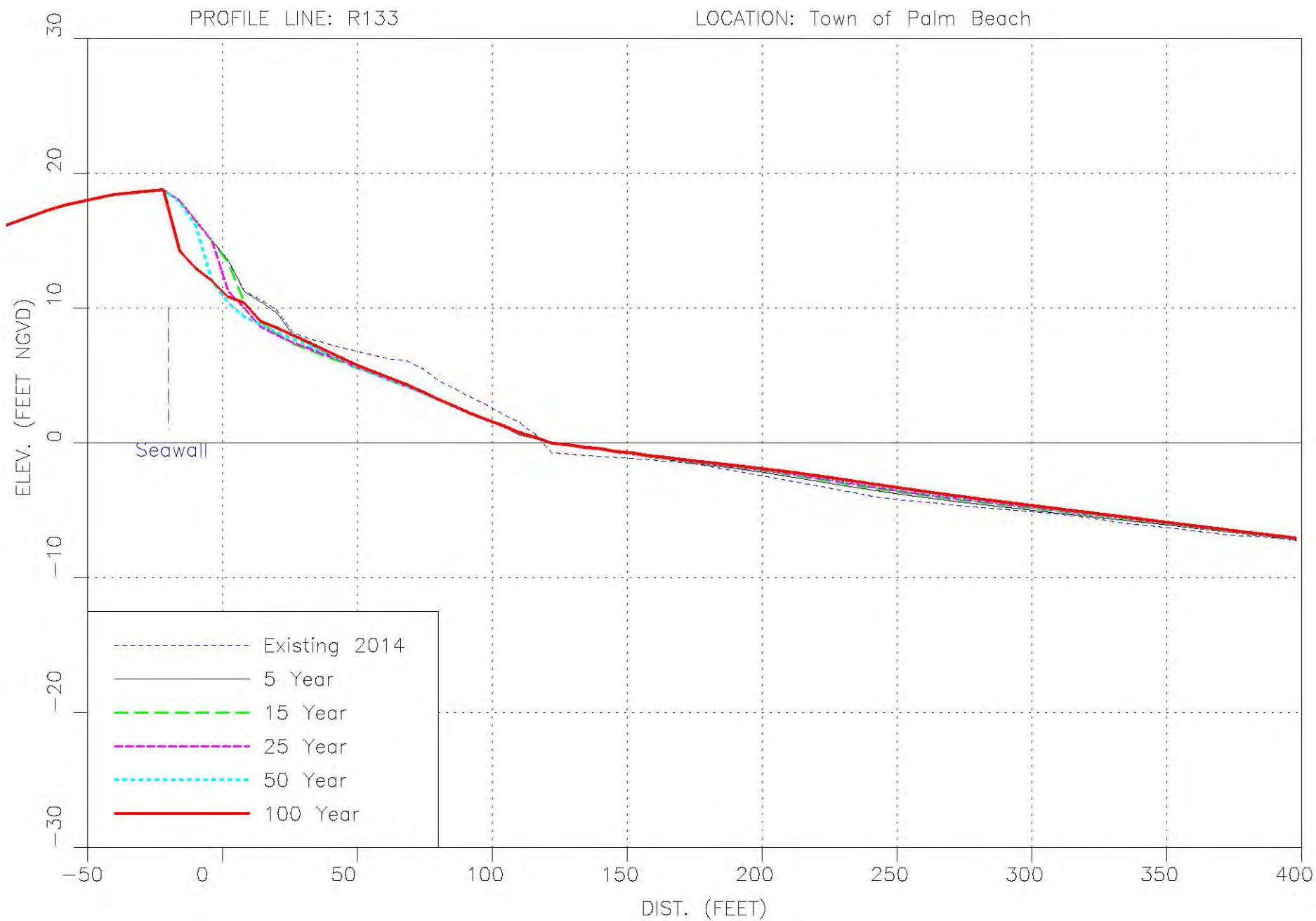
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EXISTING CONDITIONS (2014 SURVEY) GRAIN SIZE 0.36

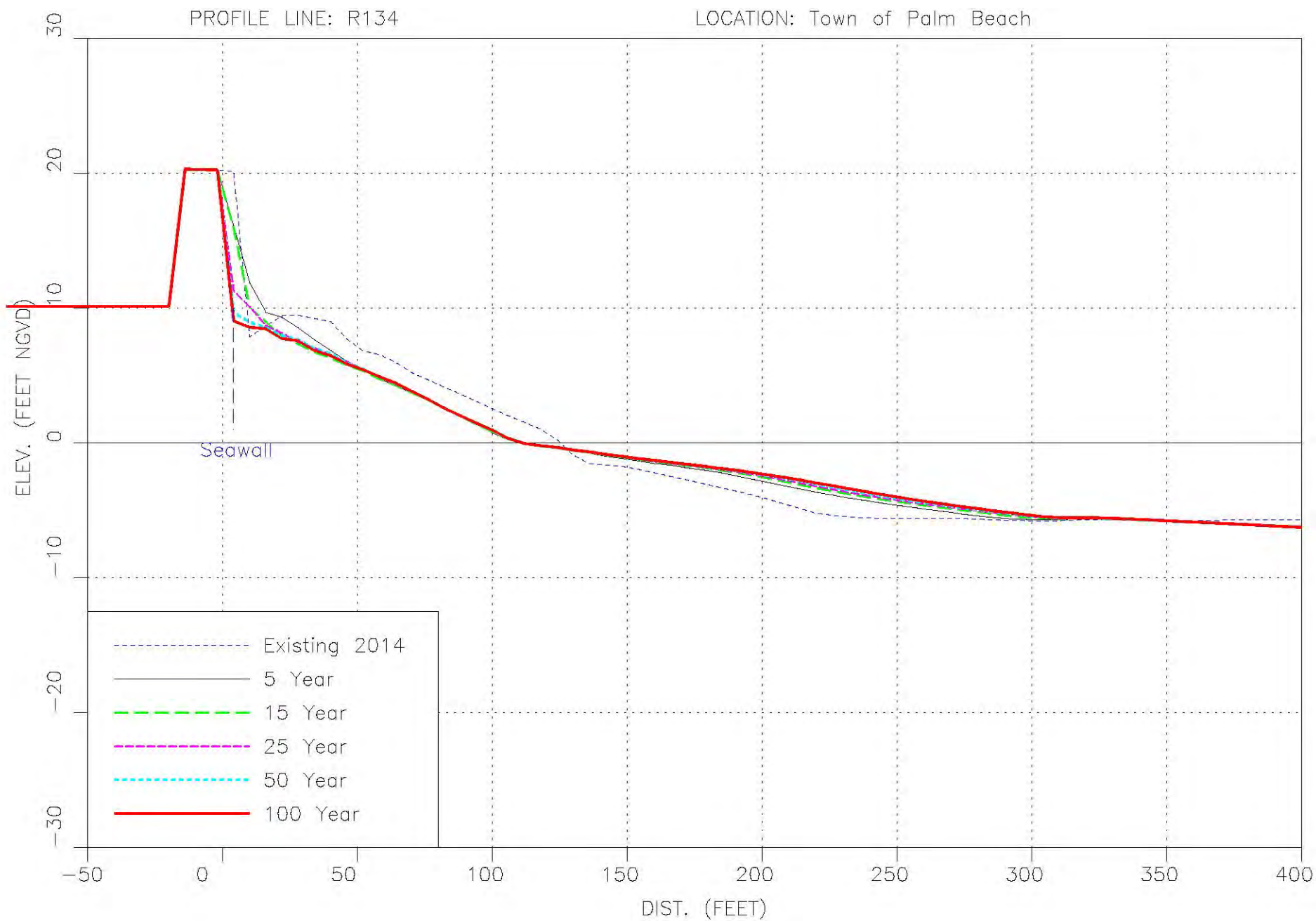






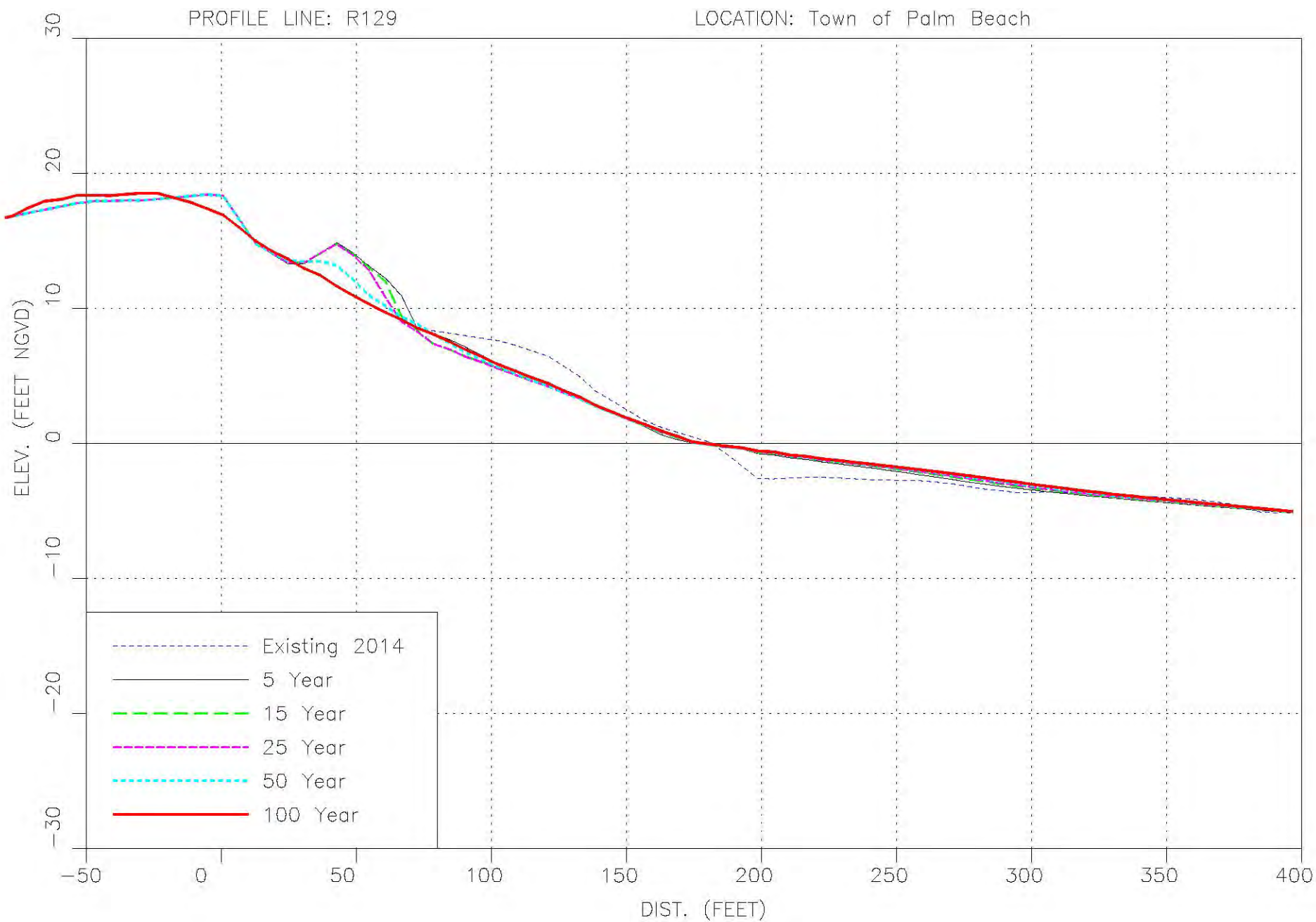


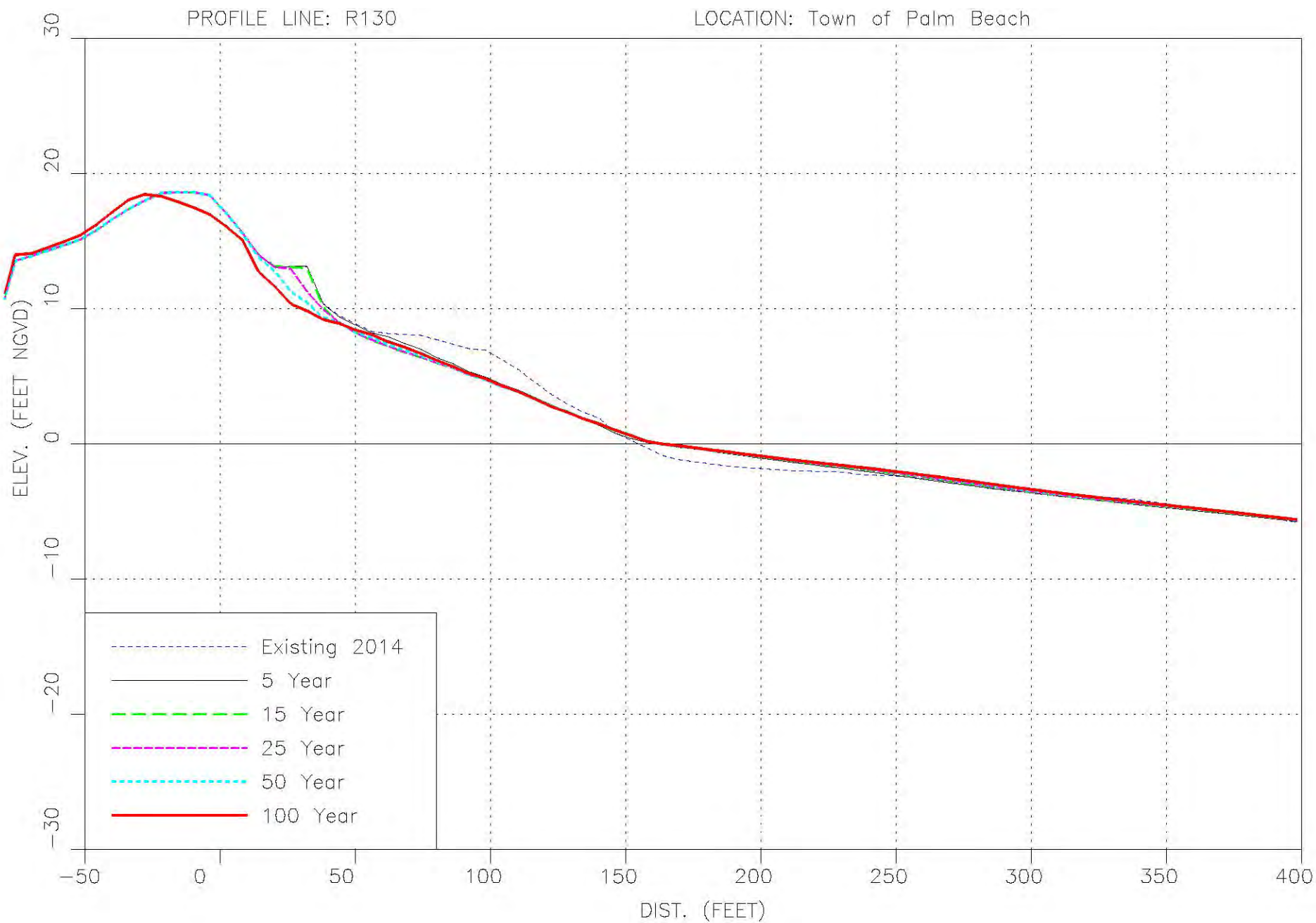


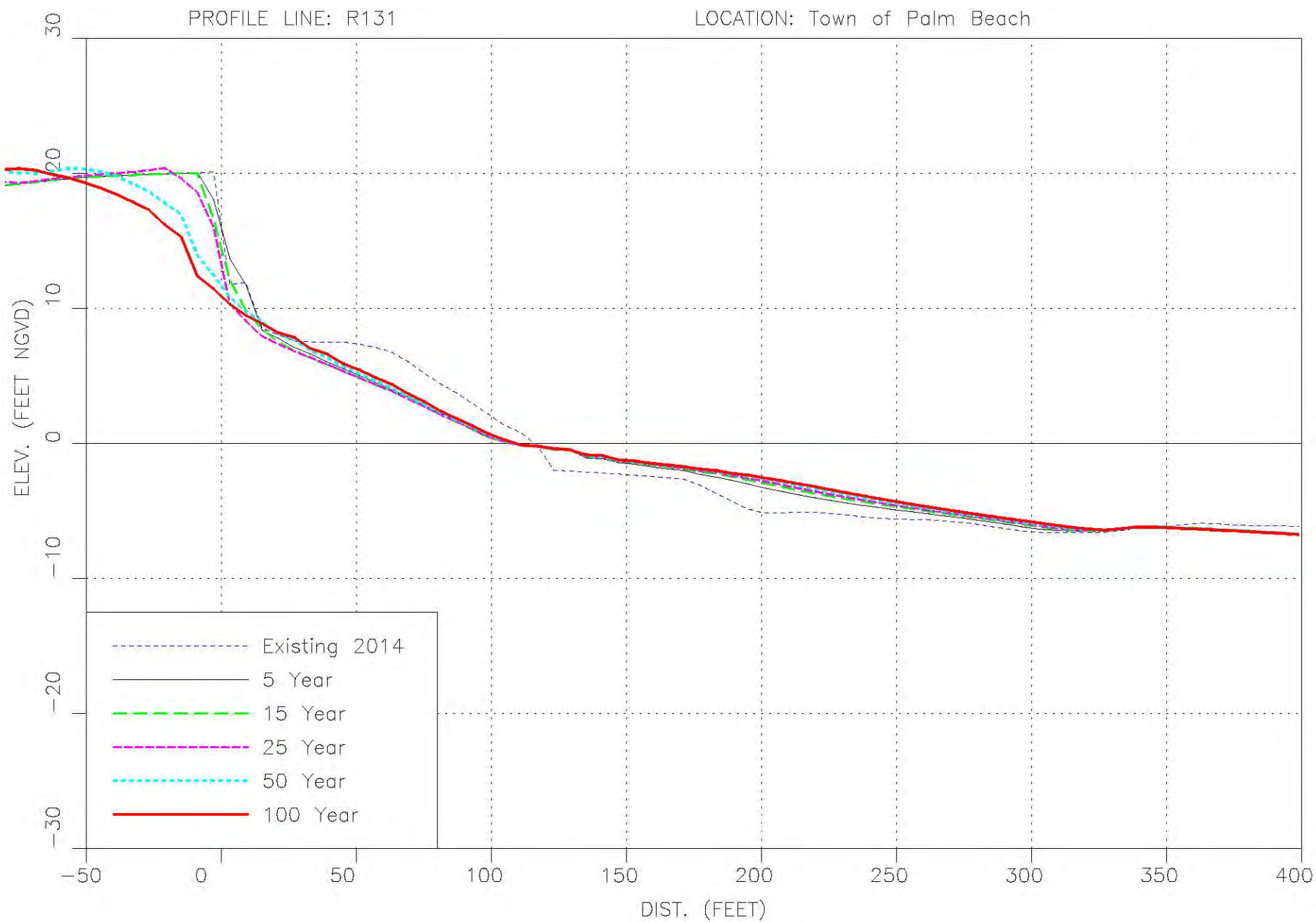


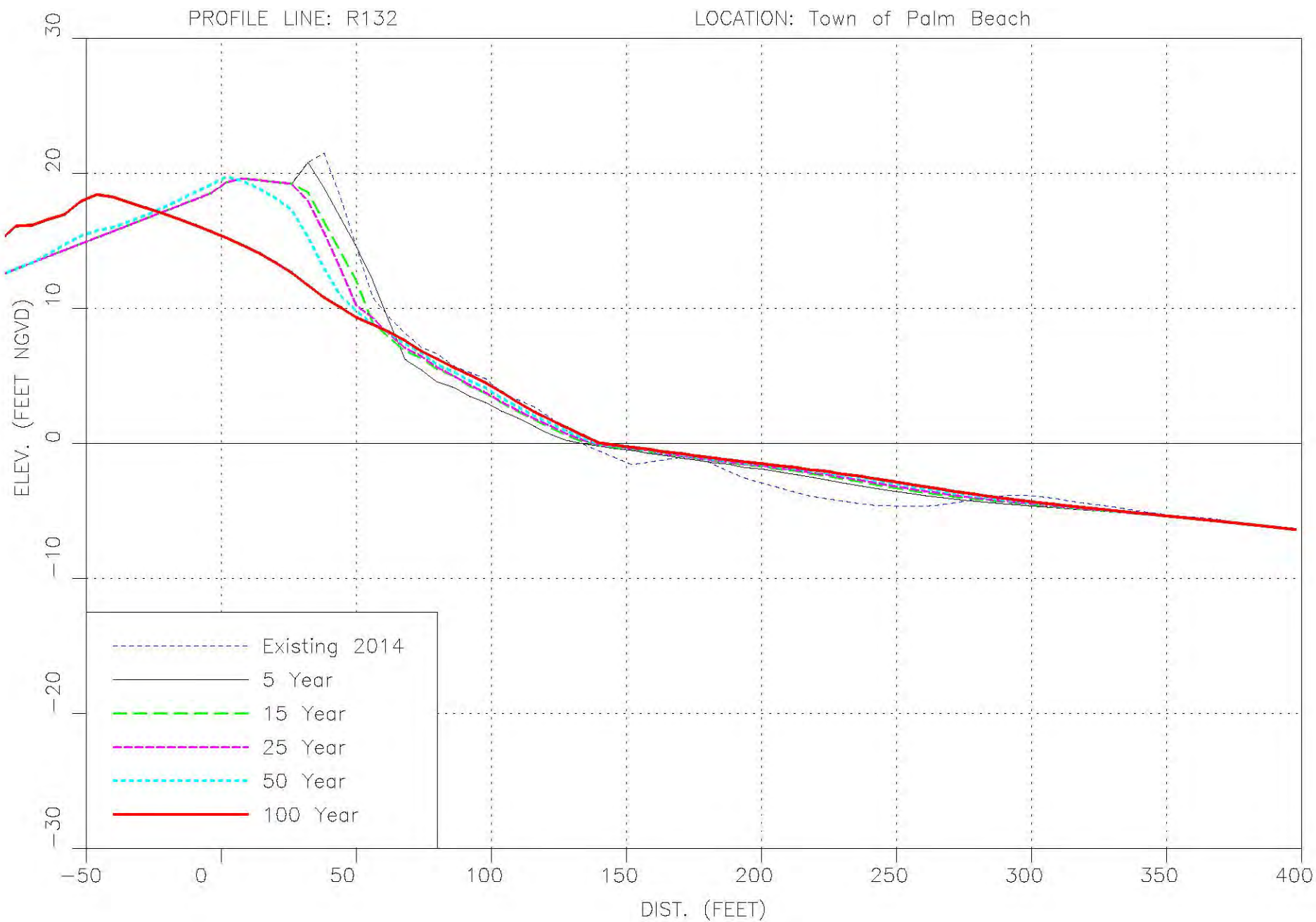
SUB-APPENDIX G-1
SBEACH ANALYSIS REPORT

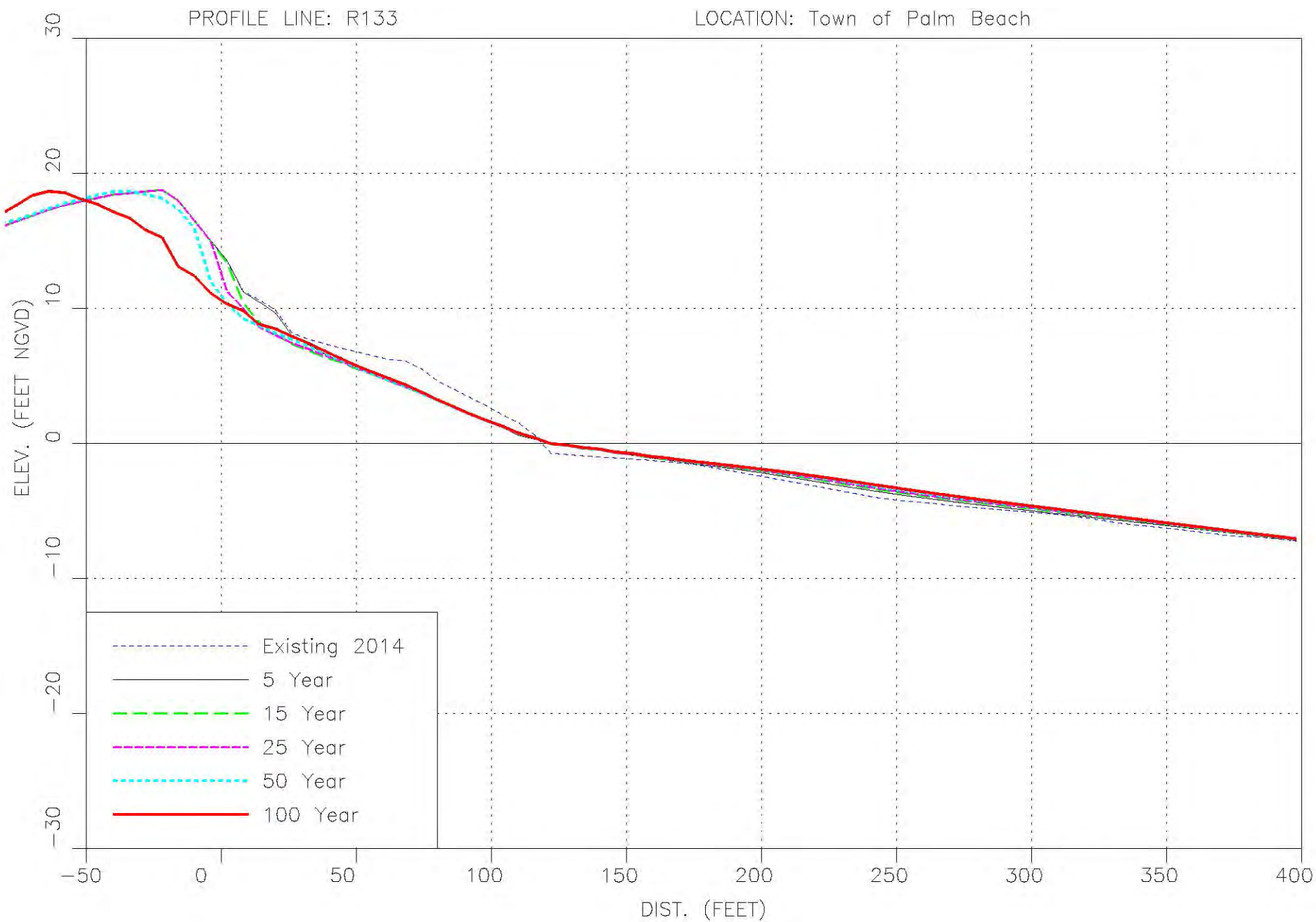
ATTACHMENT F-2
EXISTING CONDITIONS (2014 SURVEY) GRAIN SIZE 0.36
NO SEAWALL/SEAWALL FAILURE

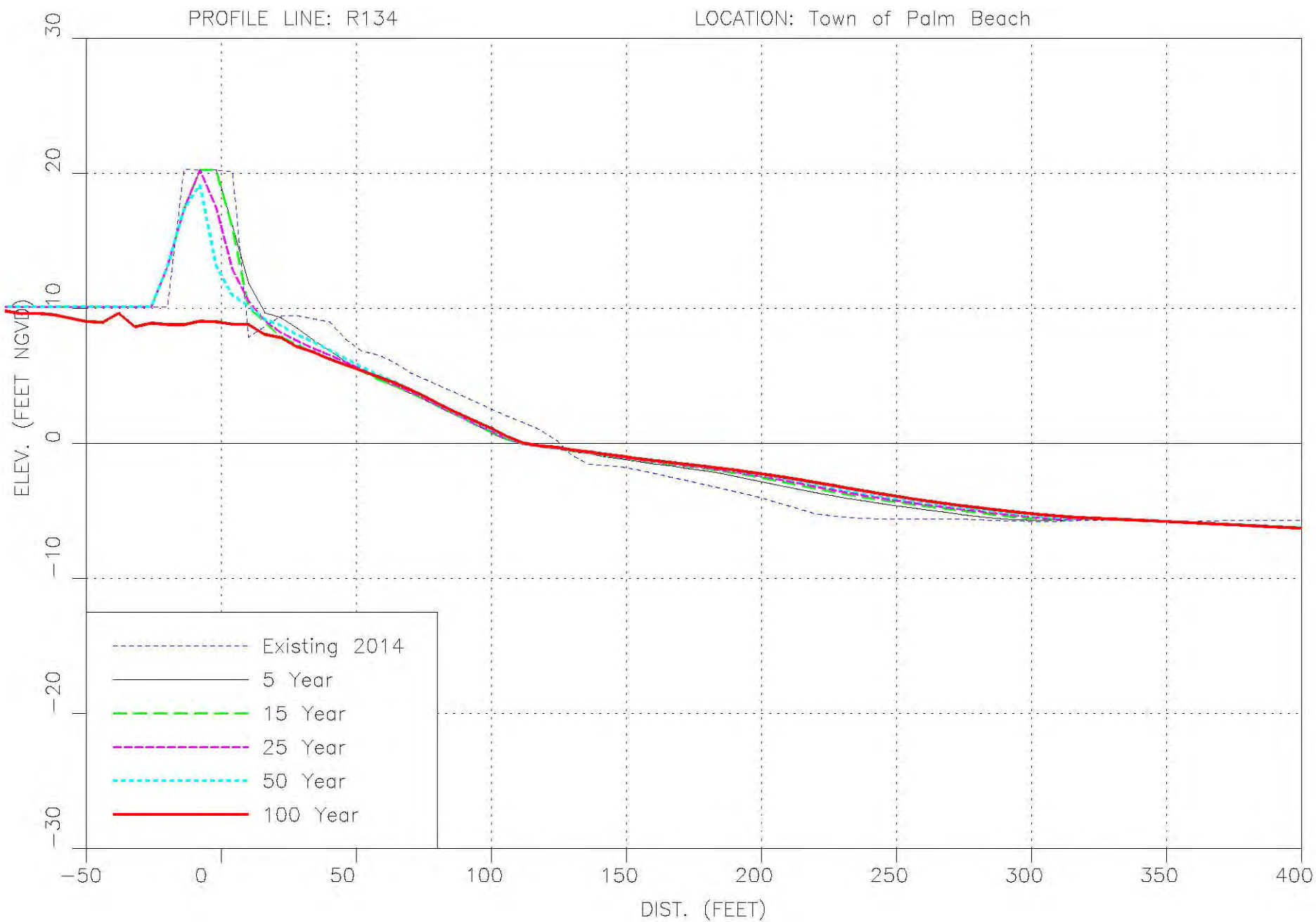






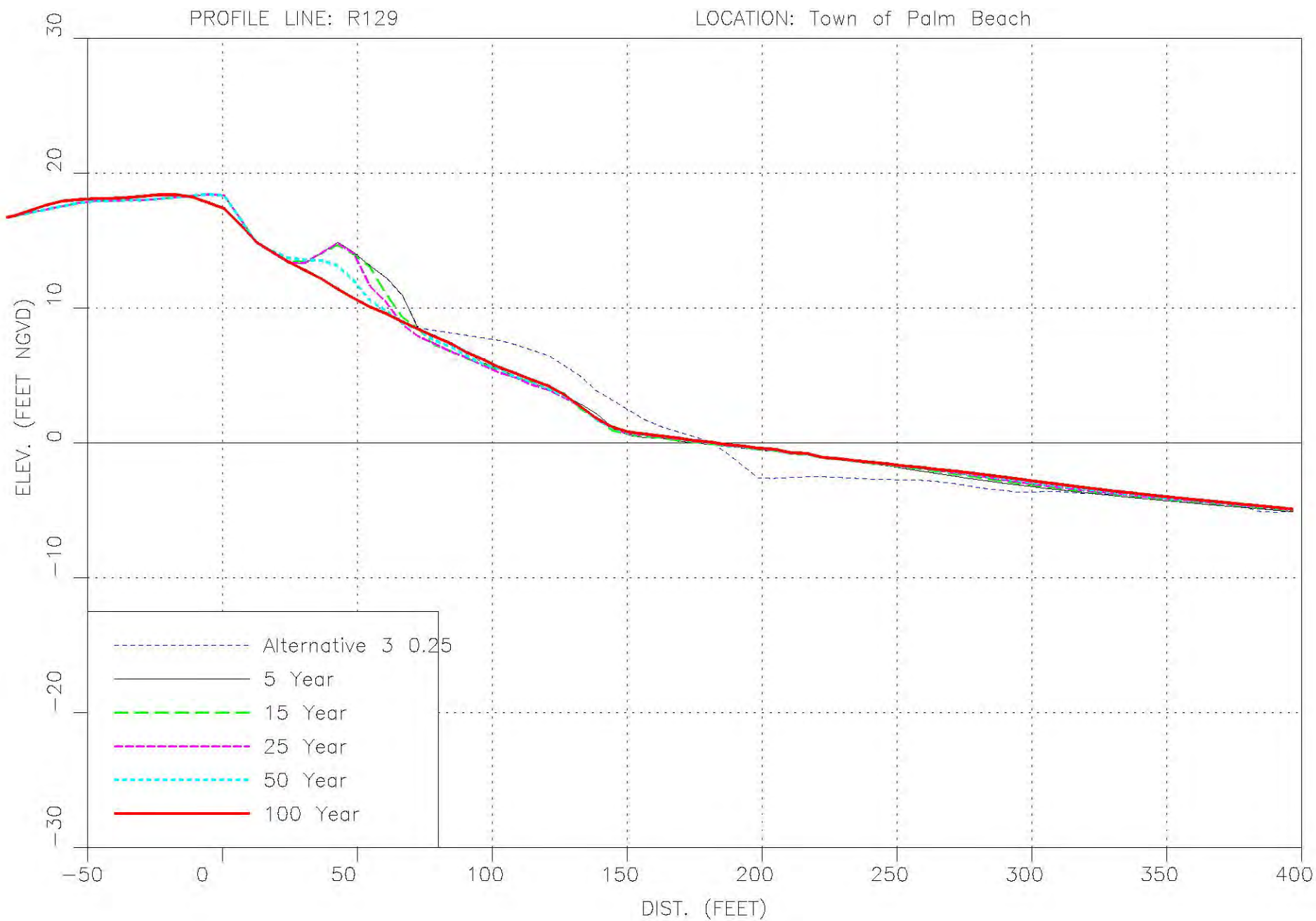


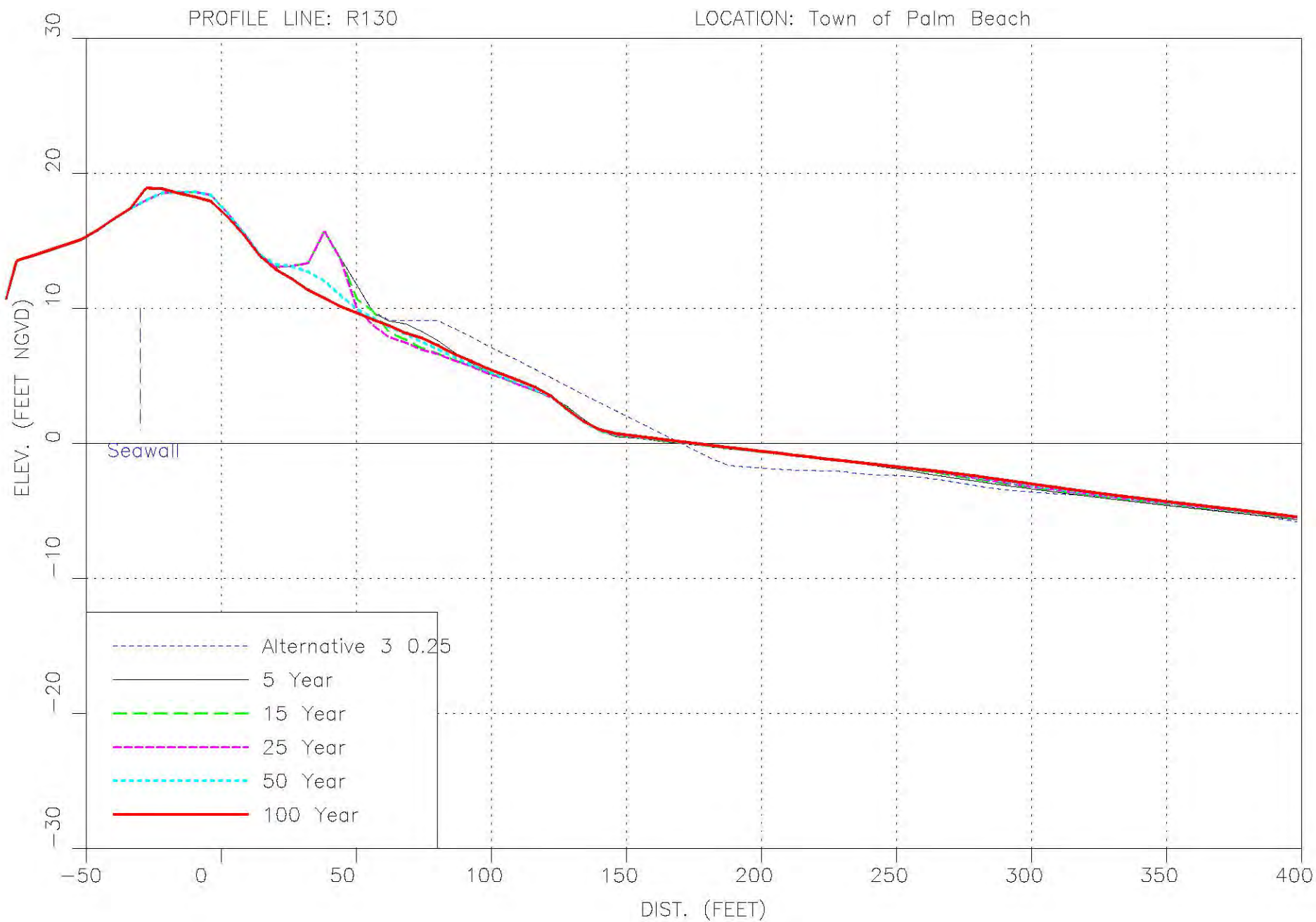


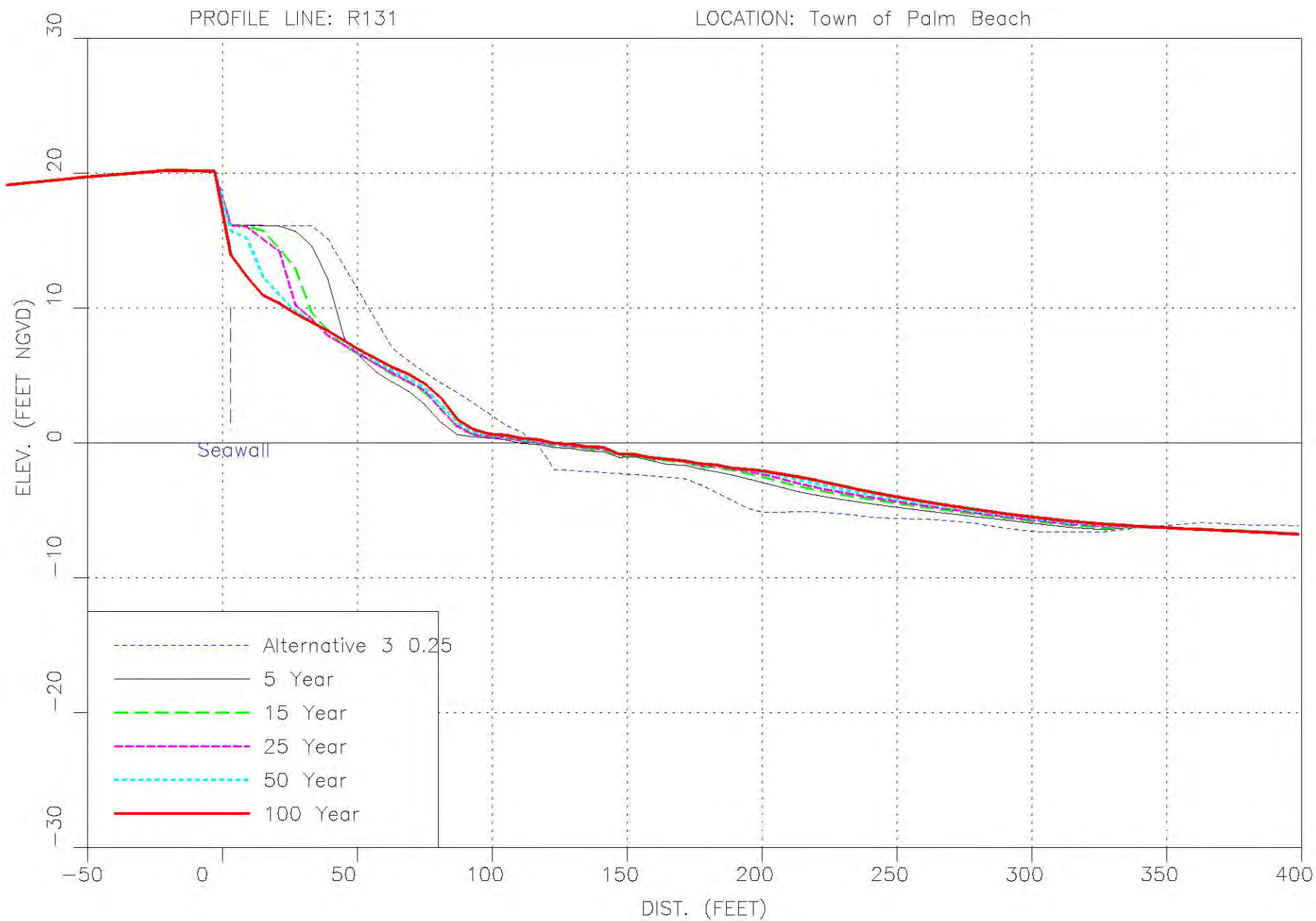


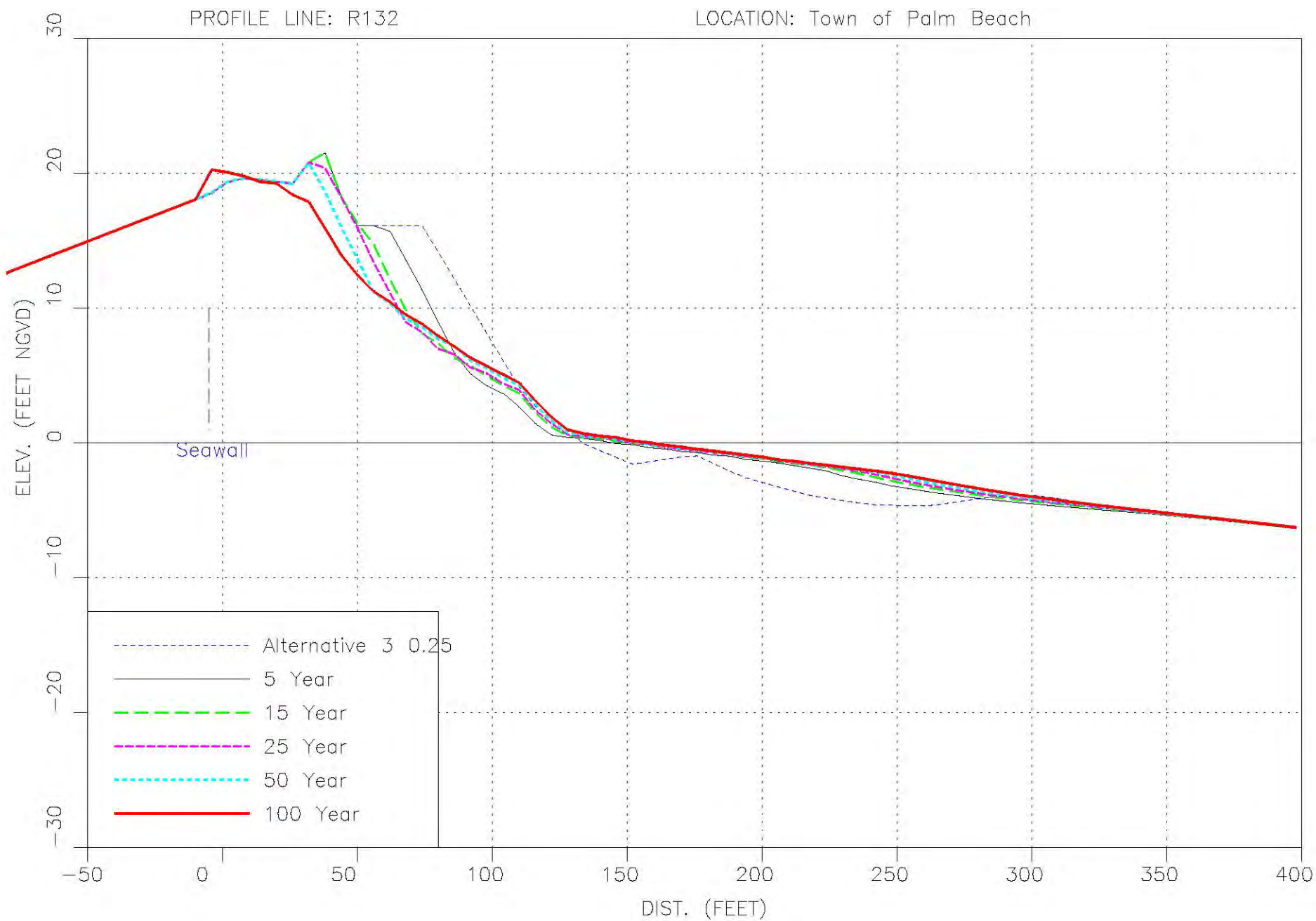
SUB-APPENDIX G-1
SBEACH ANALYSIS REPORT

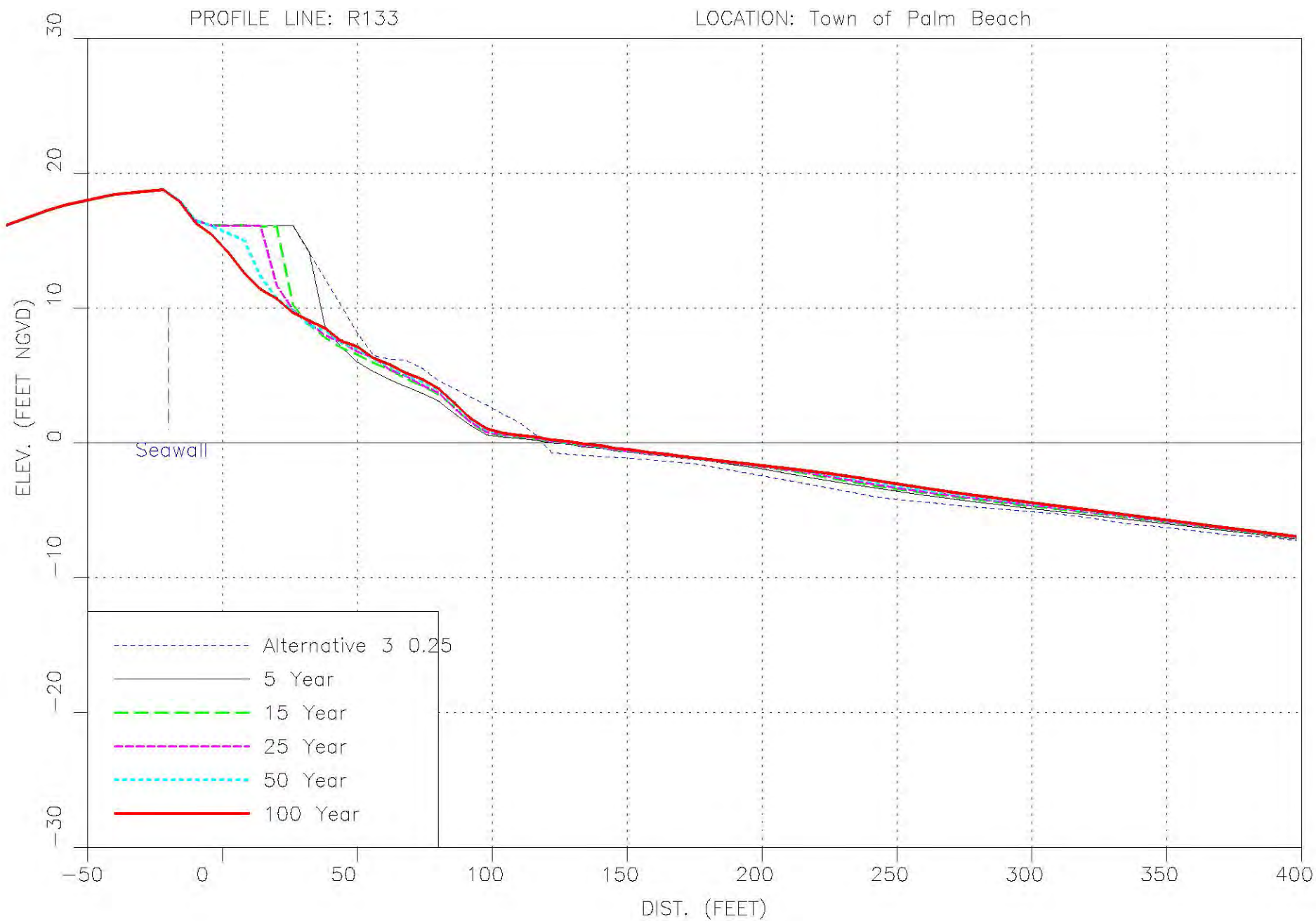
ATTACHMENT F-3
ALTERNATIVE 3 (APPLICANTS' PREFERRED) GRAIN SIZE 0.25

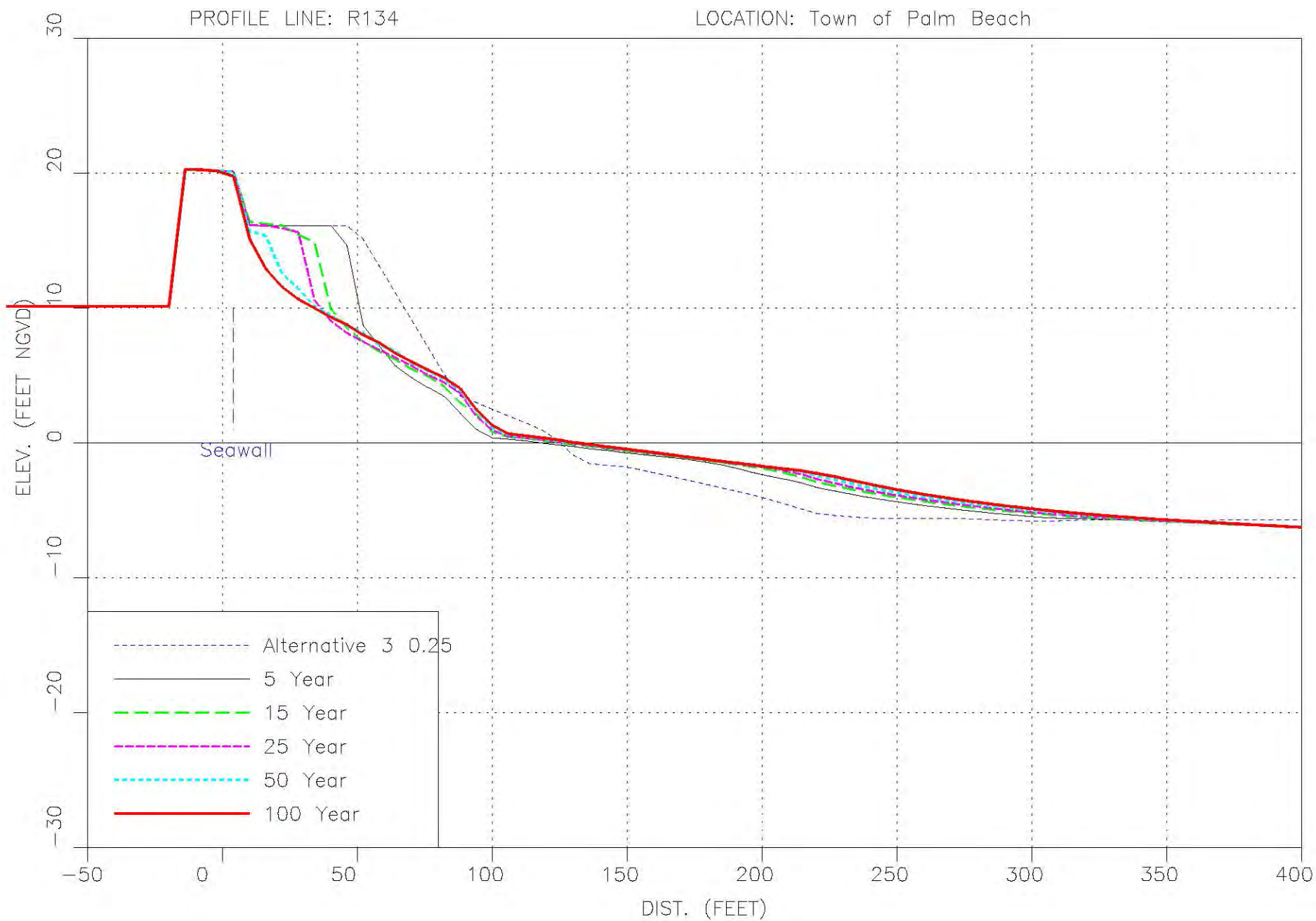






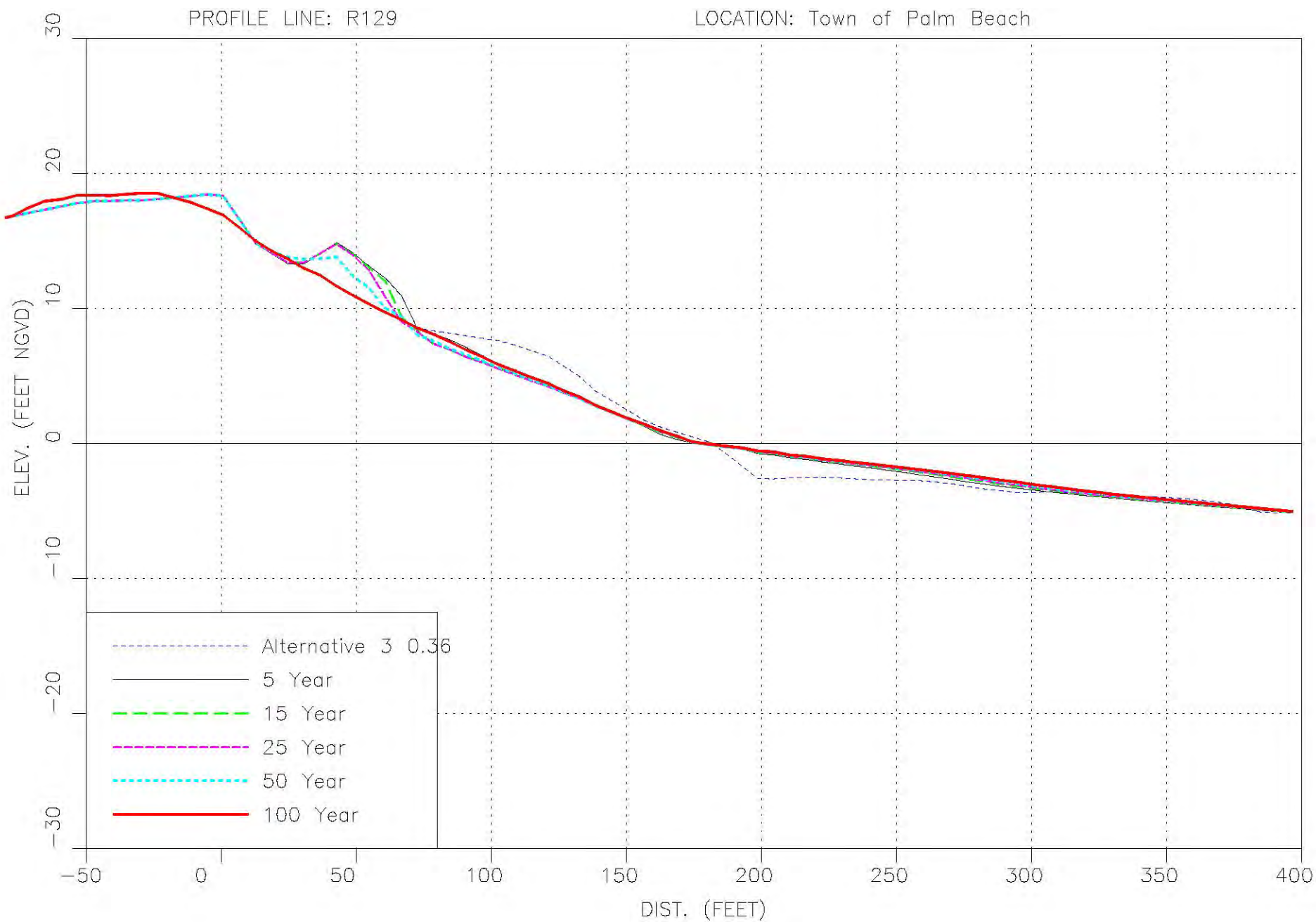


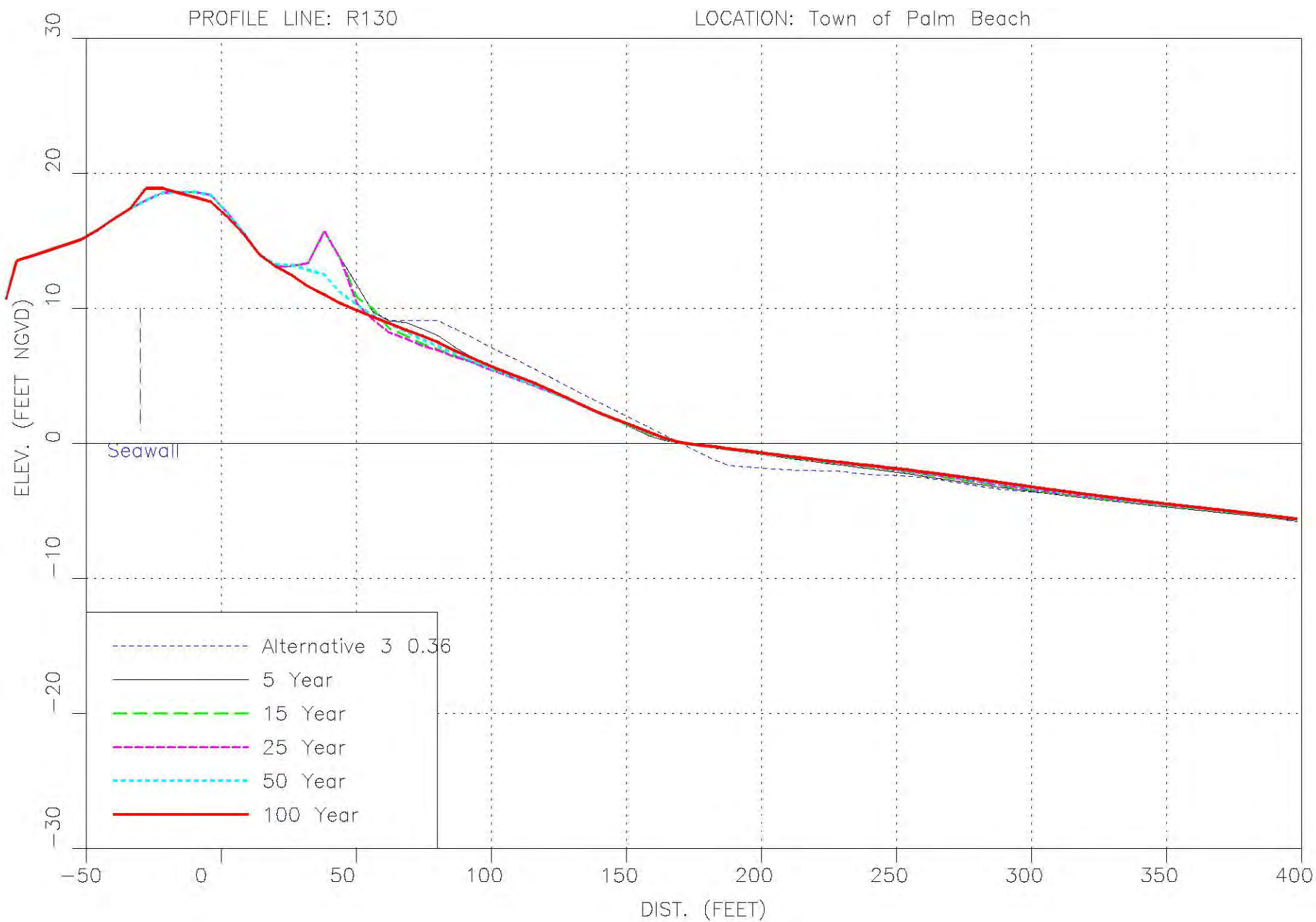


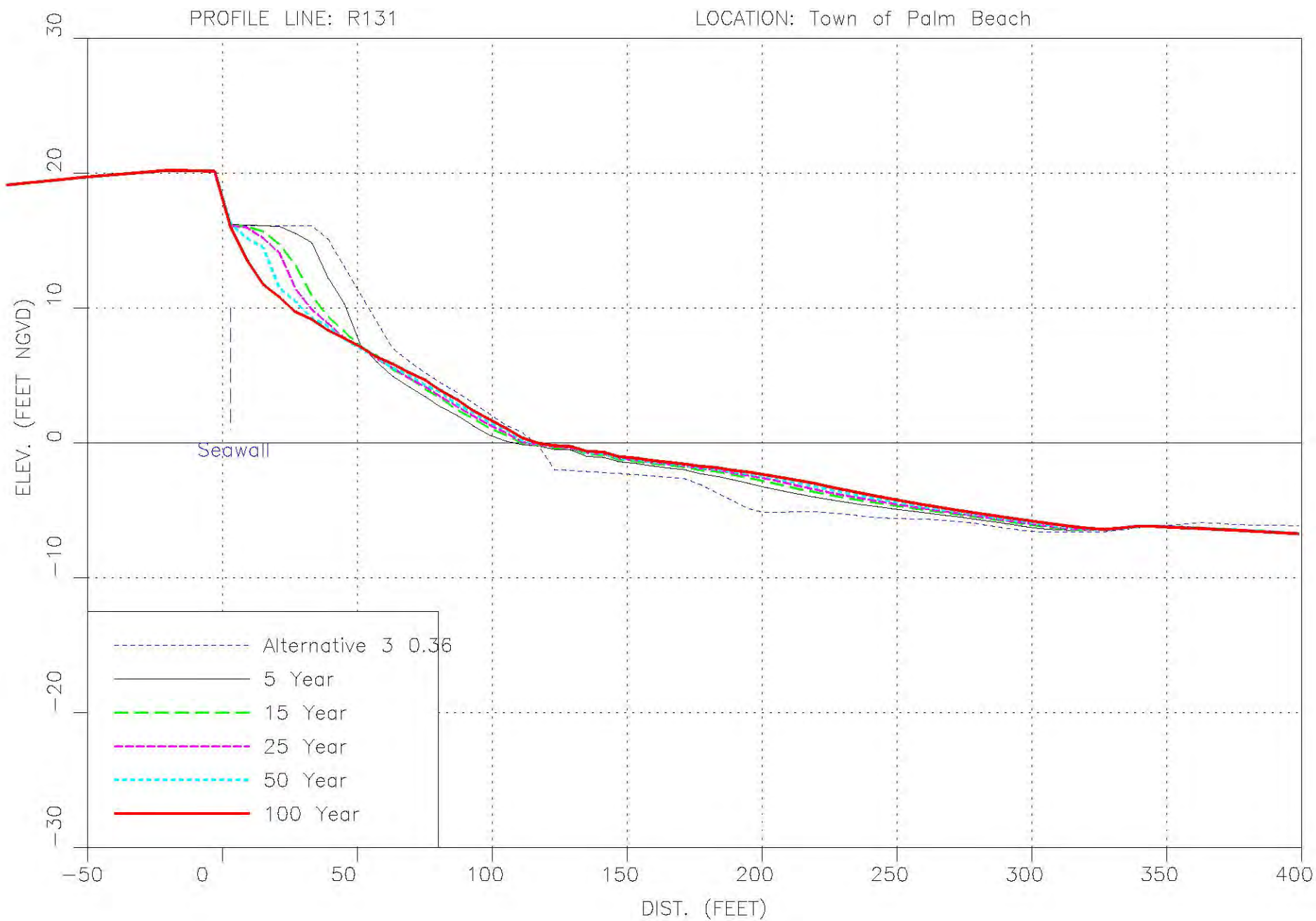


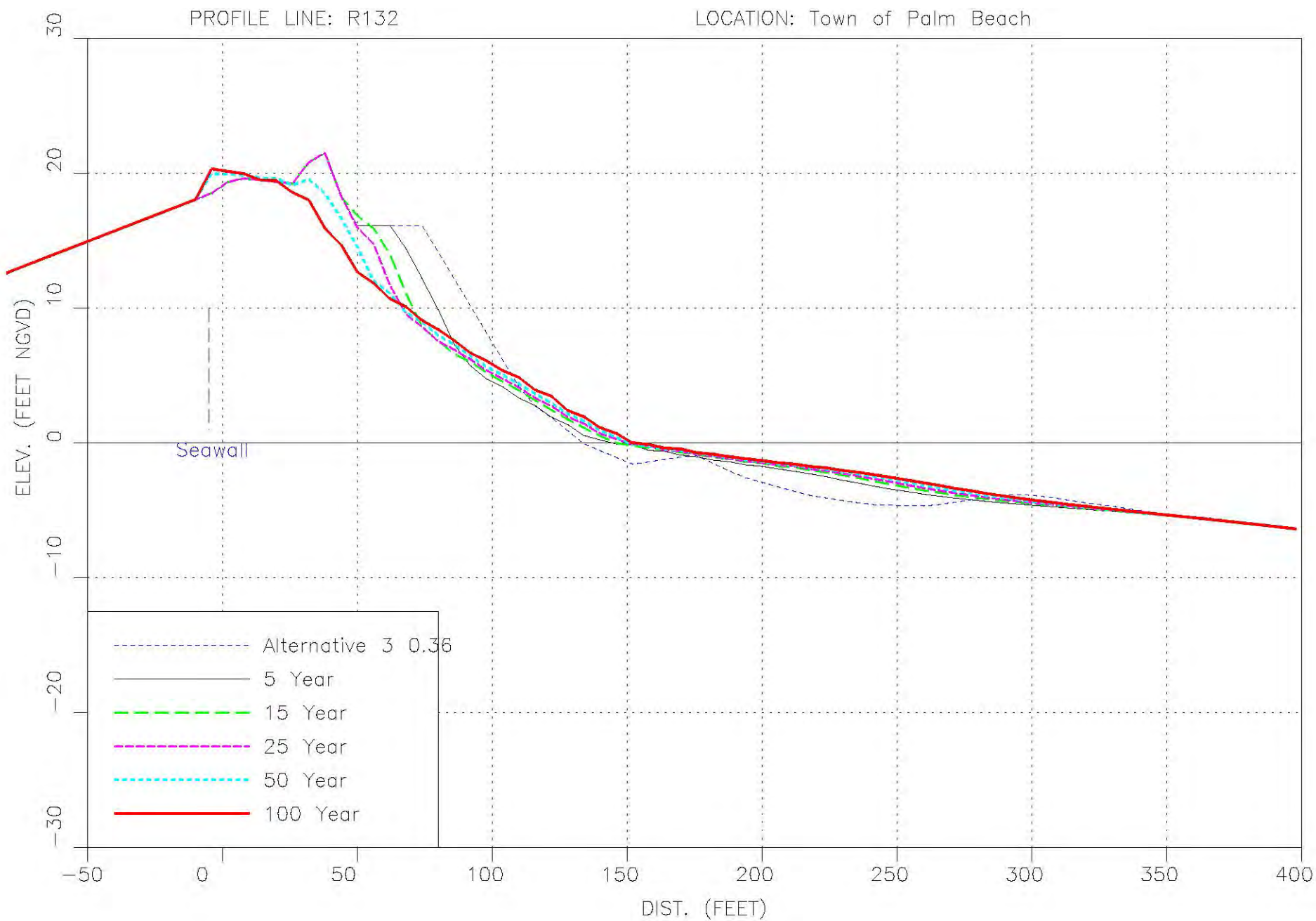
SUB-APPENDIX G-1
SBEACH ANALYSIS REPORT

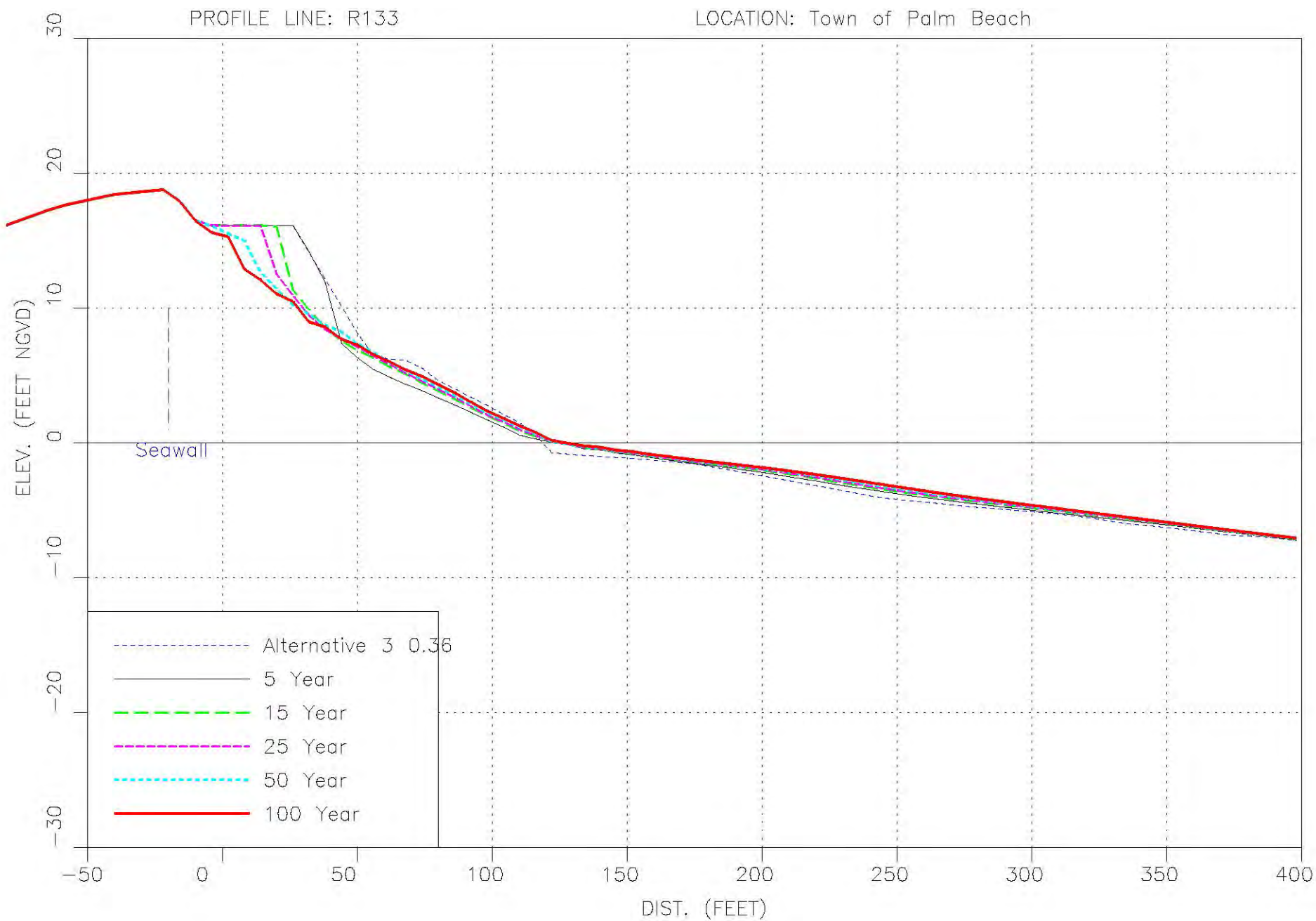
ATTACHMENT F-4
ALTERNATIVE 3 (APPLICANTS' PREFERRED) GRAIN SIZE 0.36

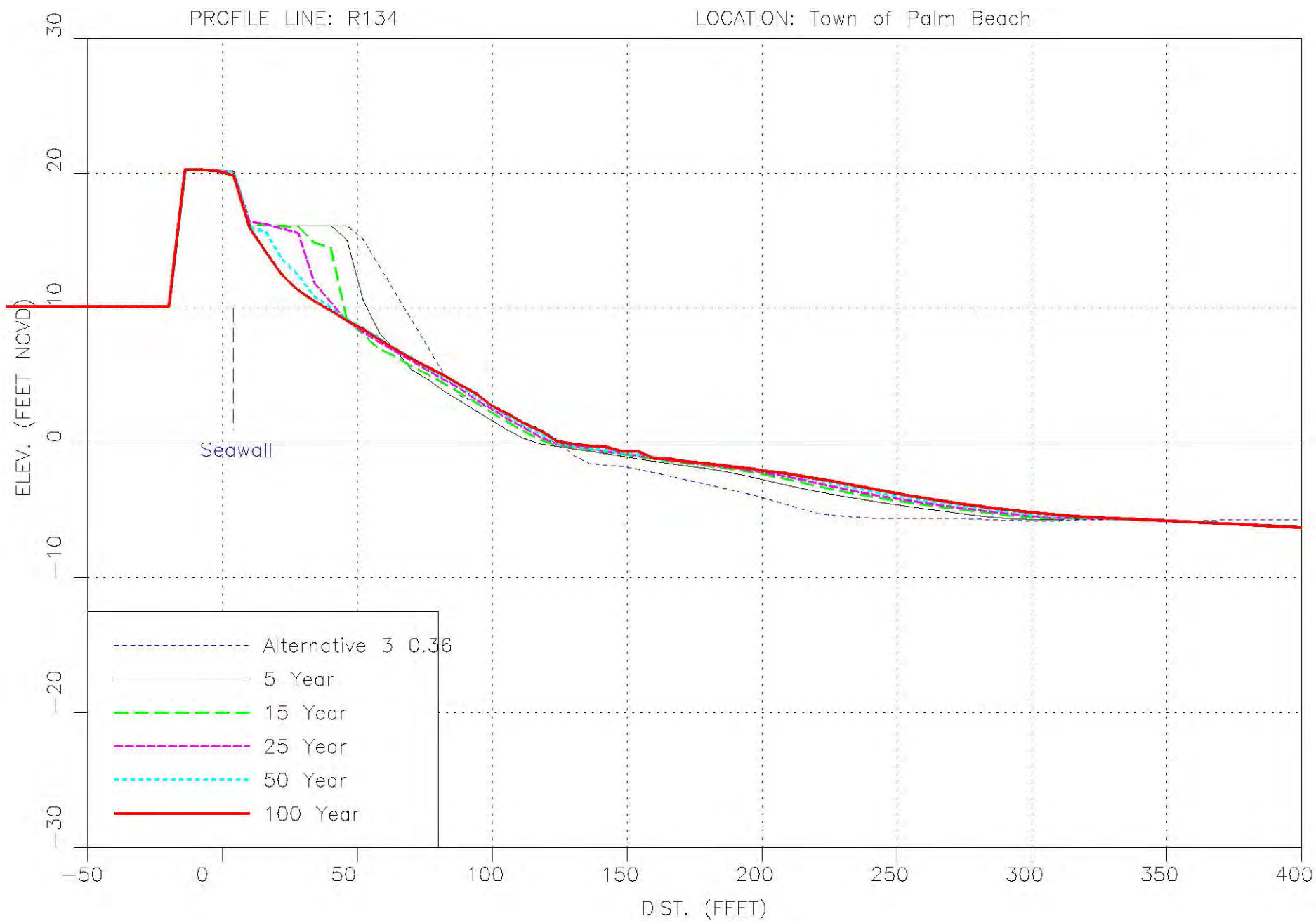






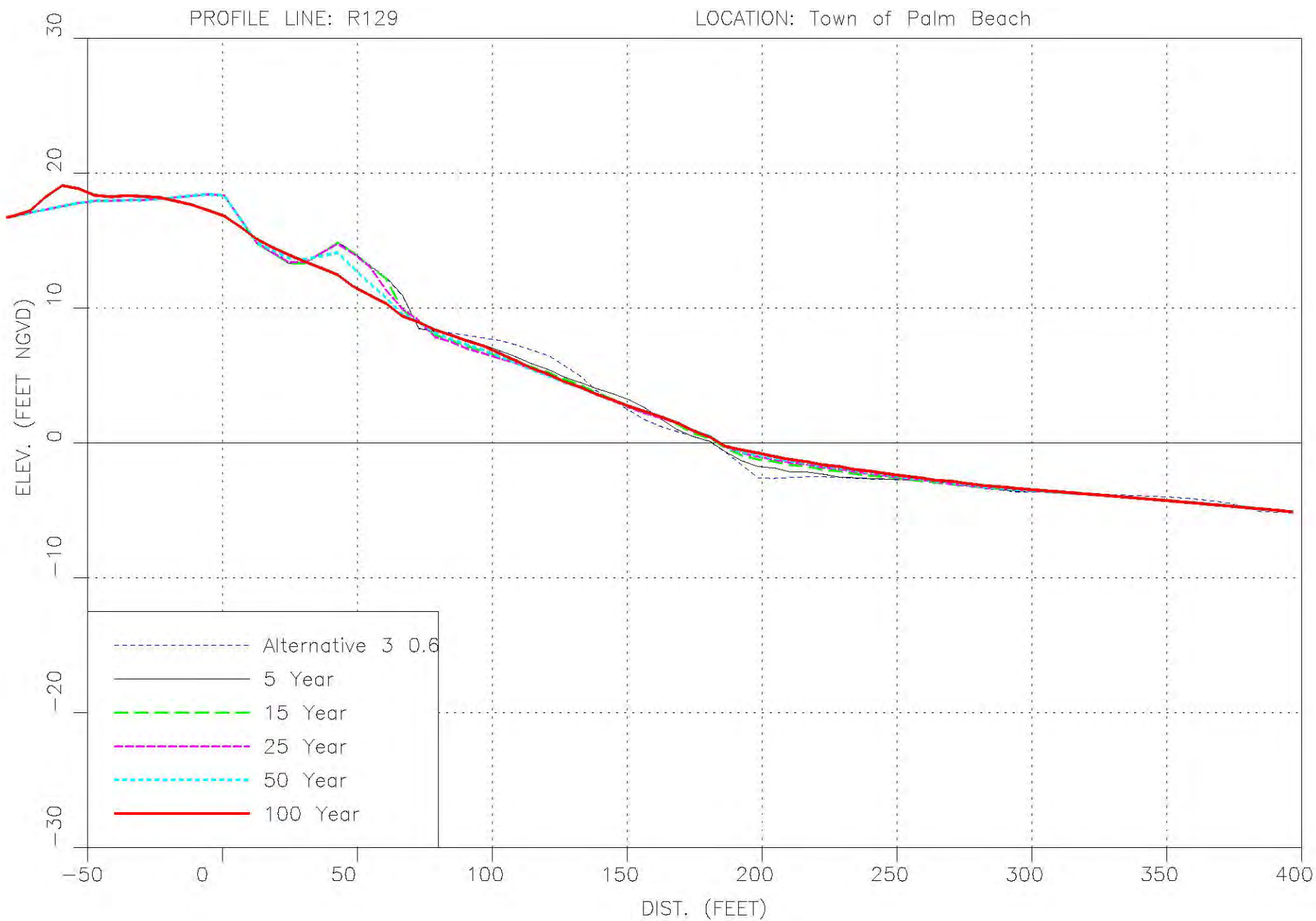


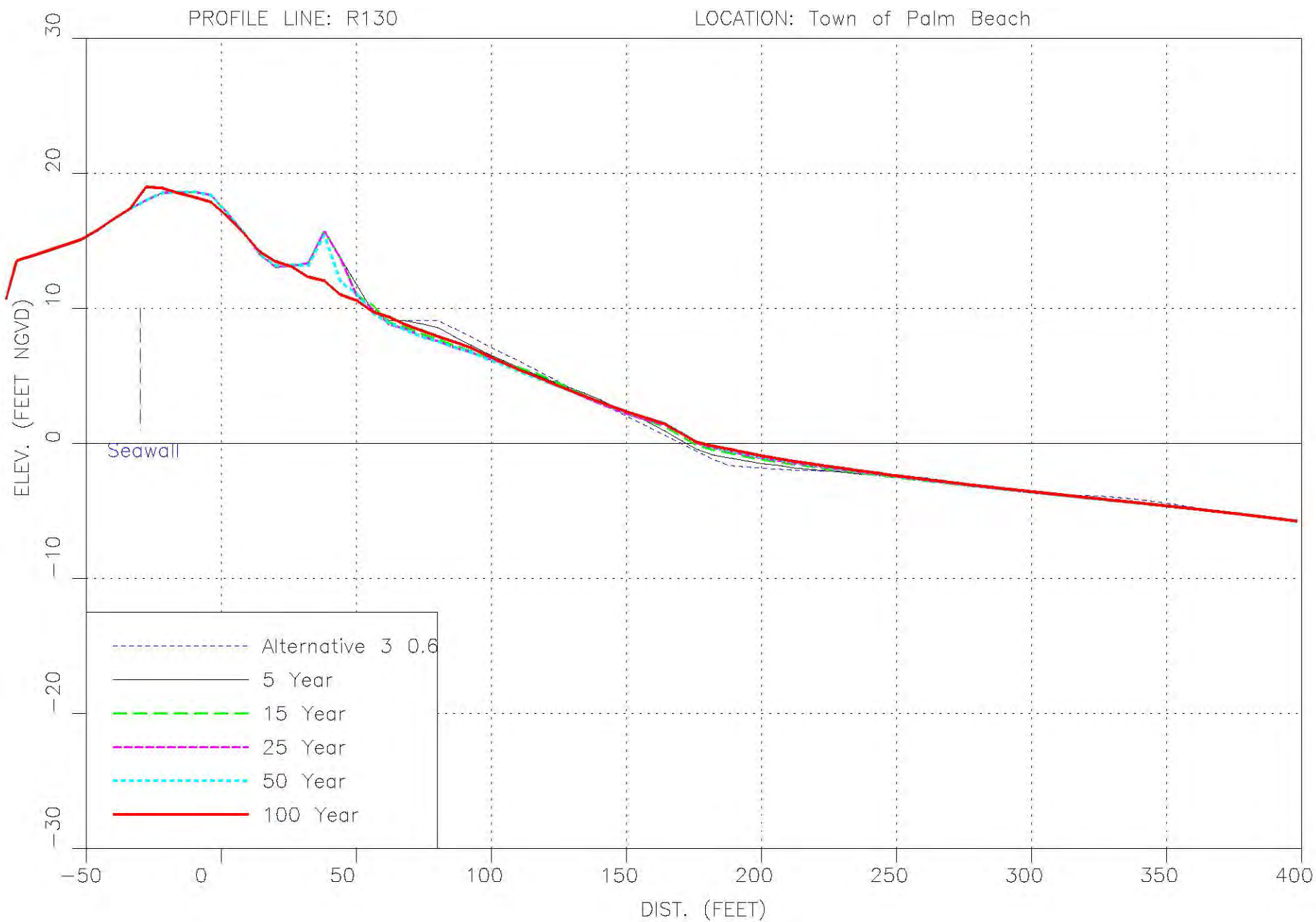


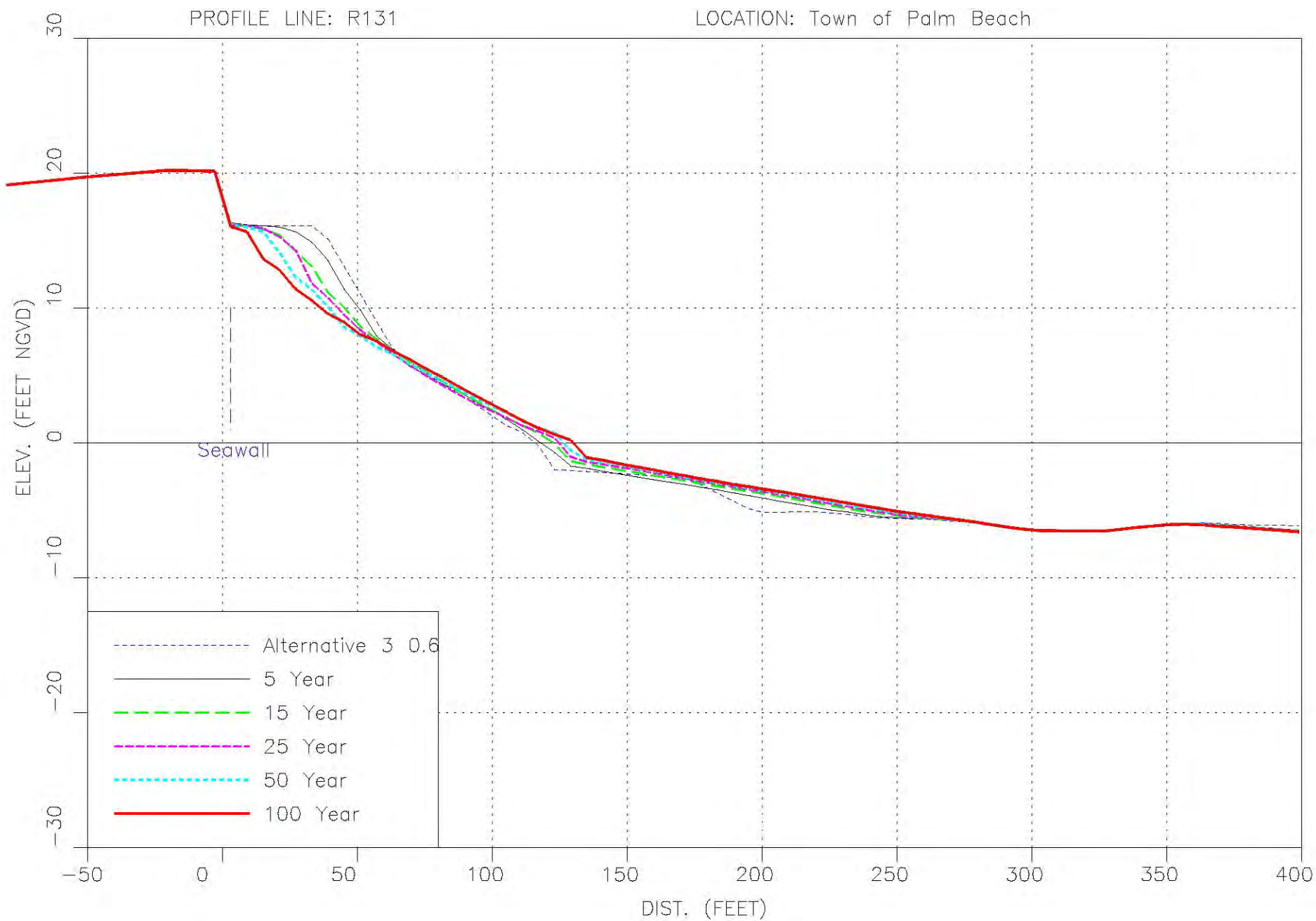


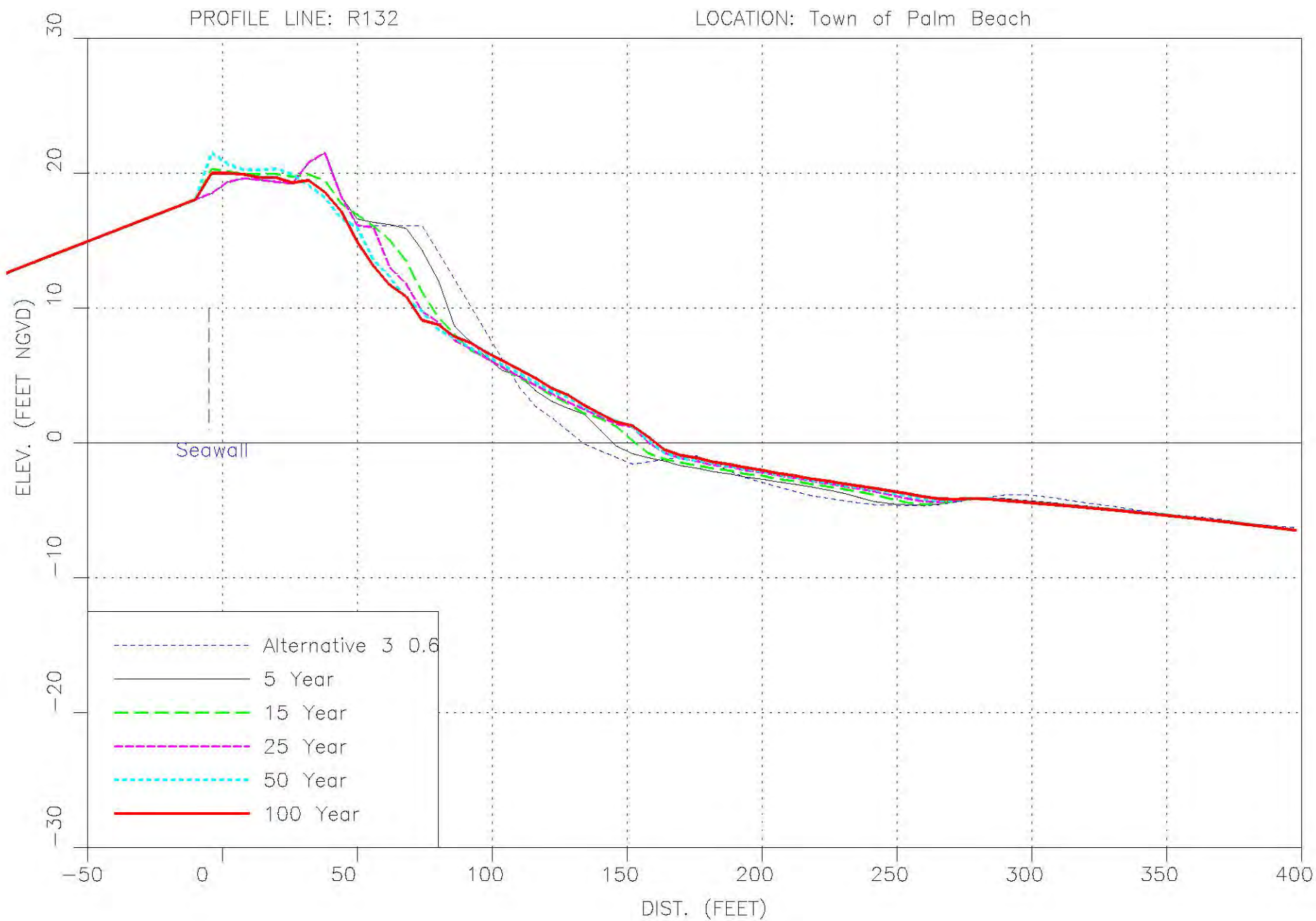
SUB-APPENDIX G-1
SBEACH ANALYSIS REPORT

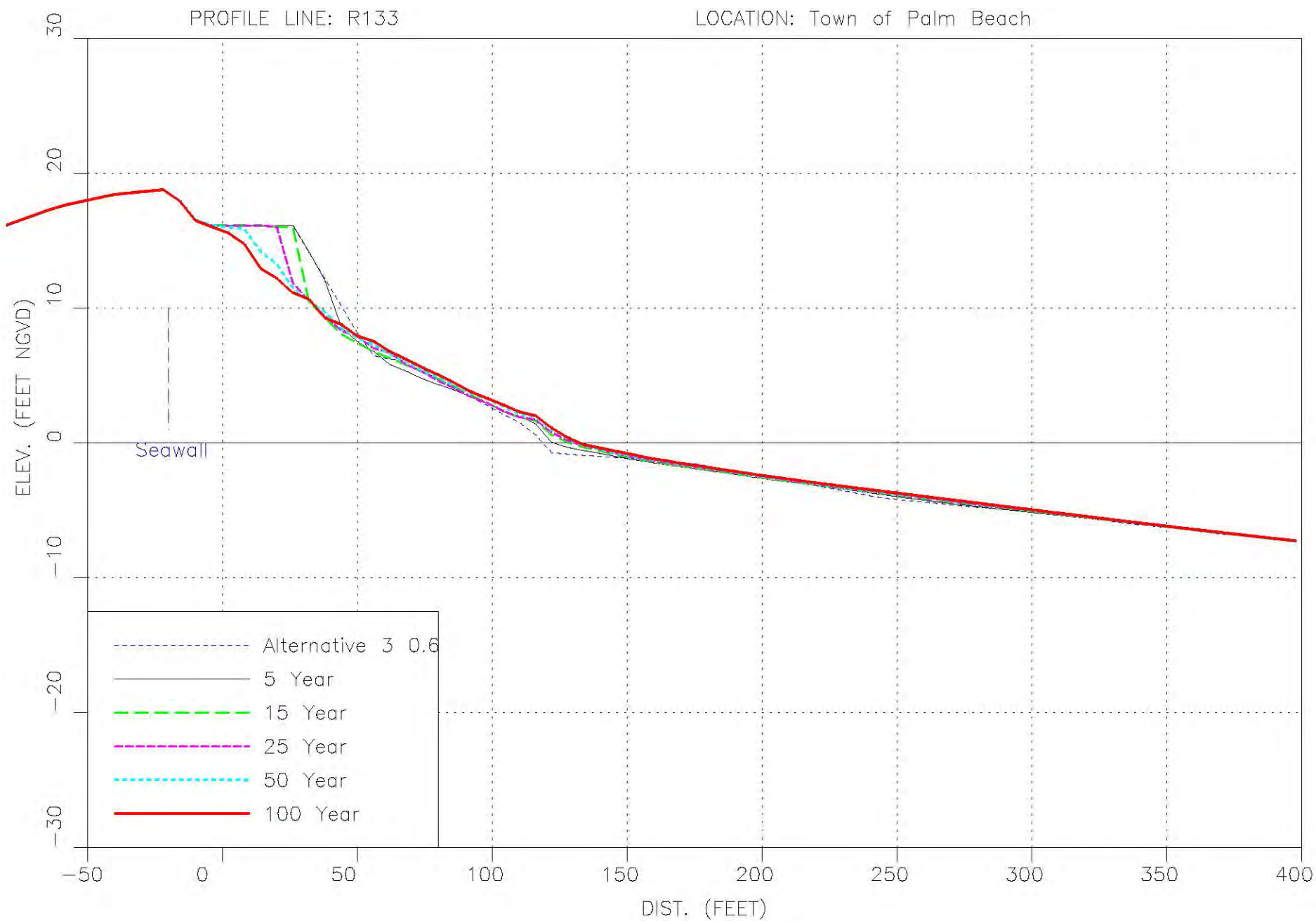
ATTACHMENT F-5
ALTERNATIVE 3 (APPLICANTS' PREFERRED) GRAIN SIZE 0.6

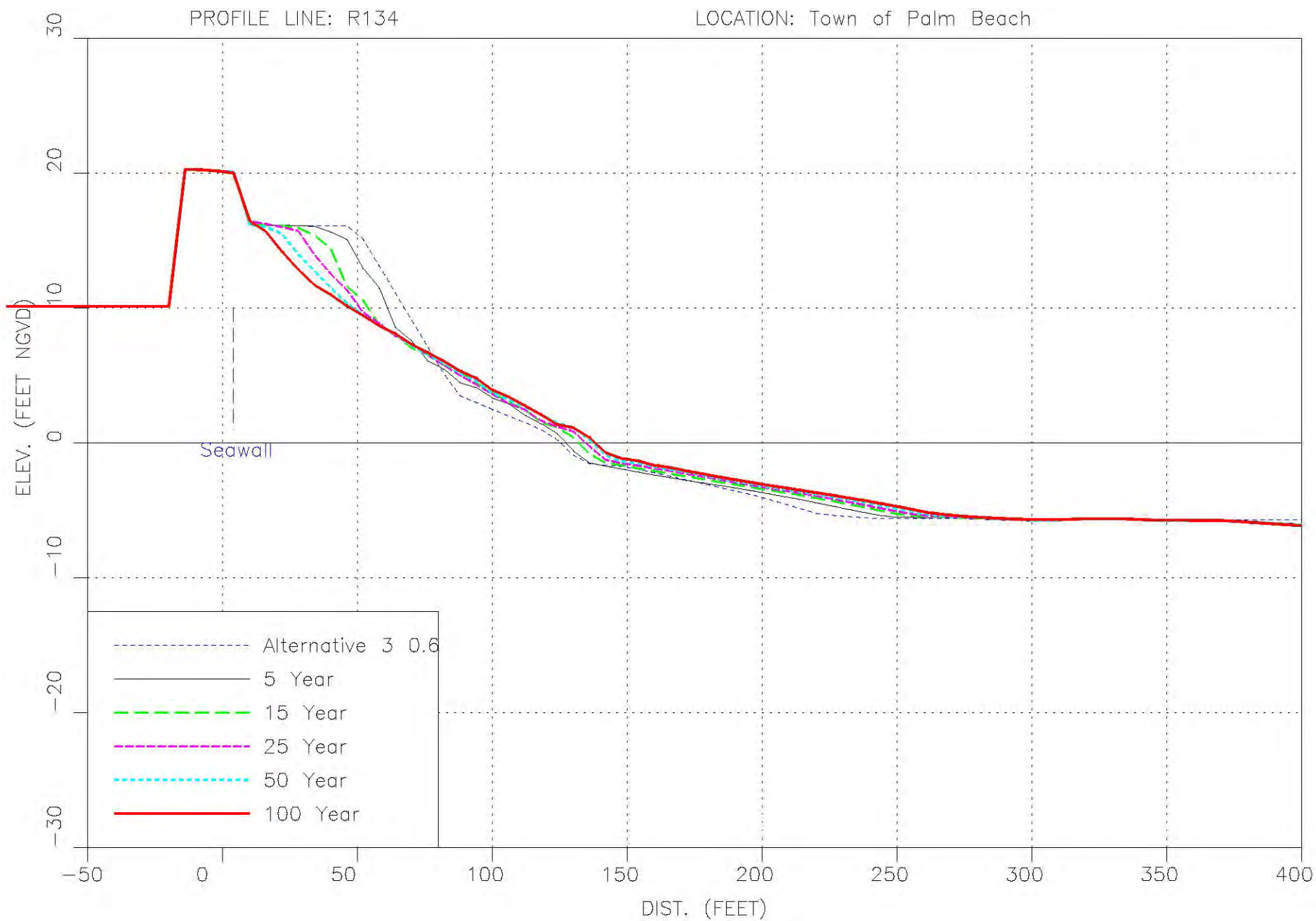






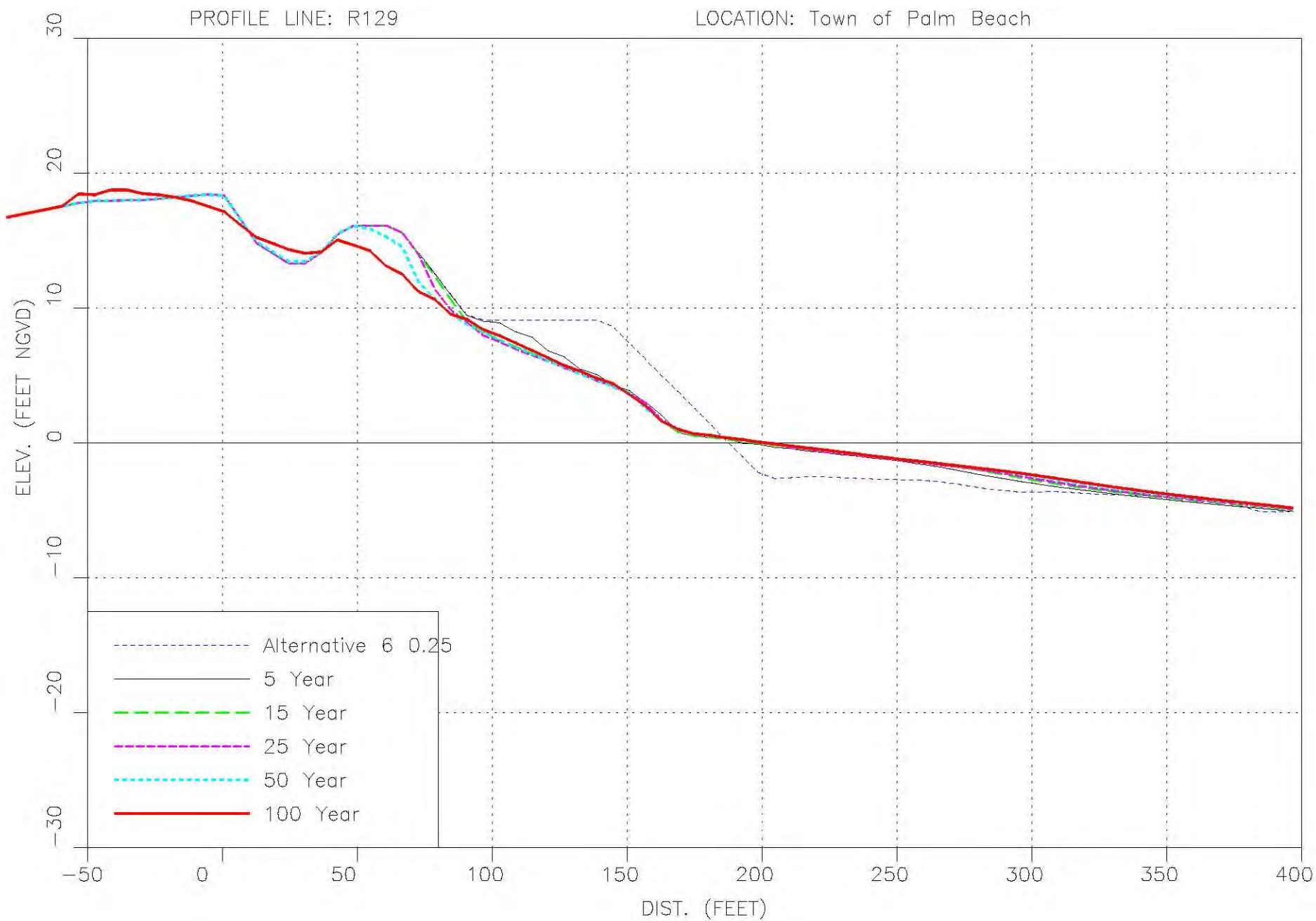


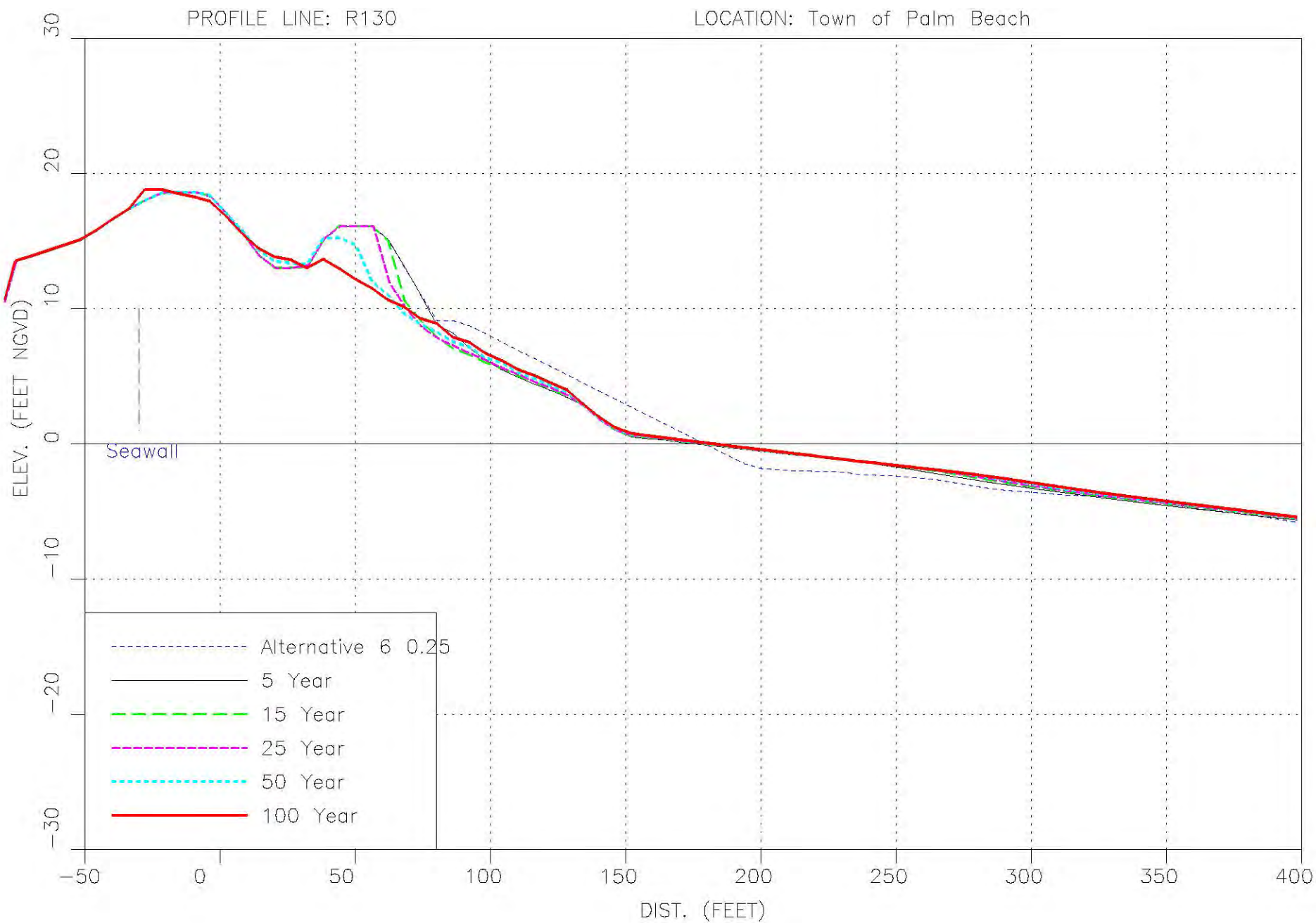


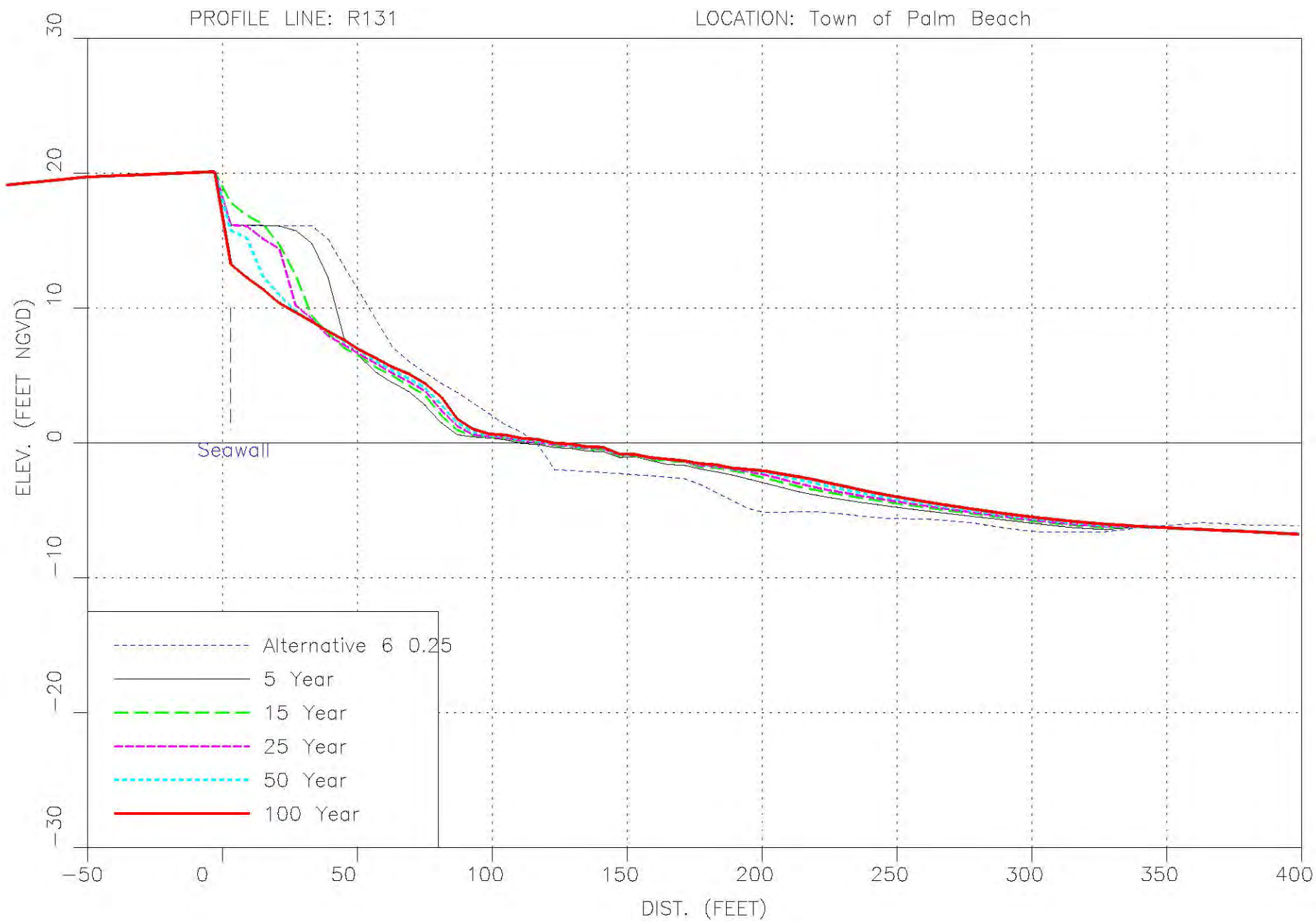


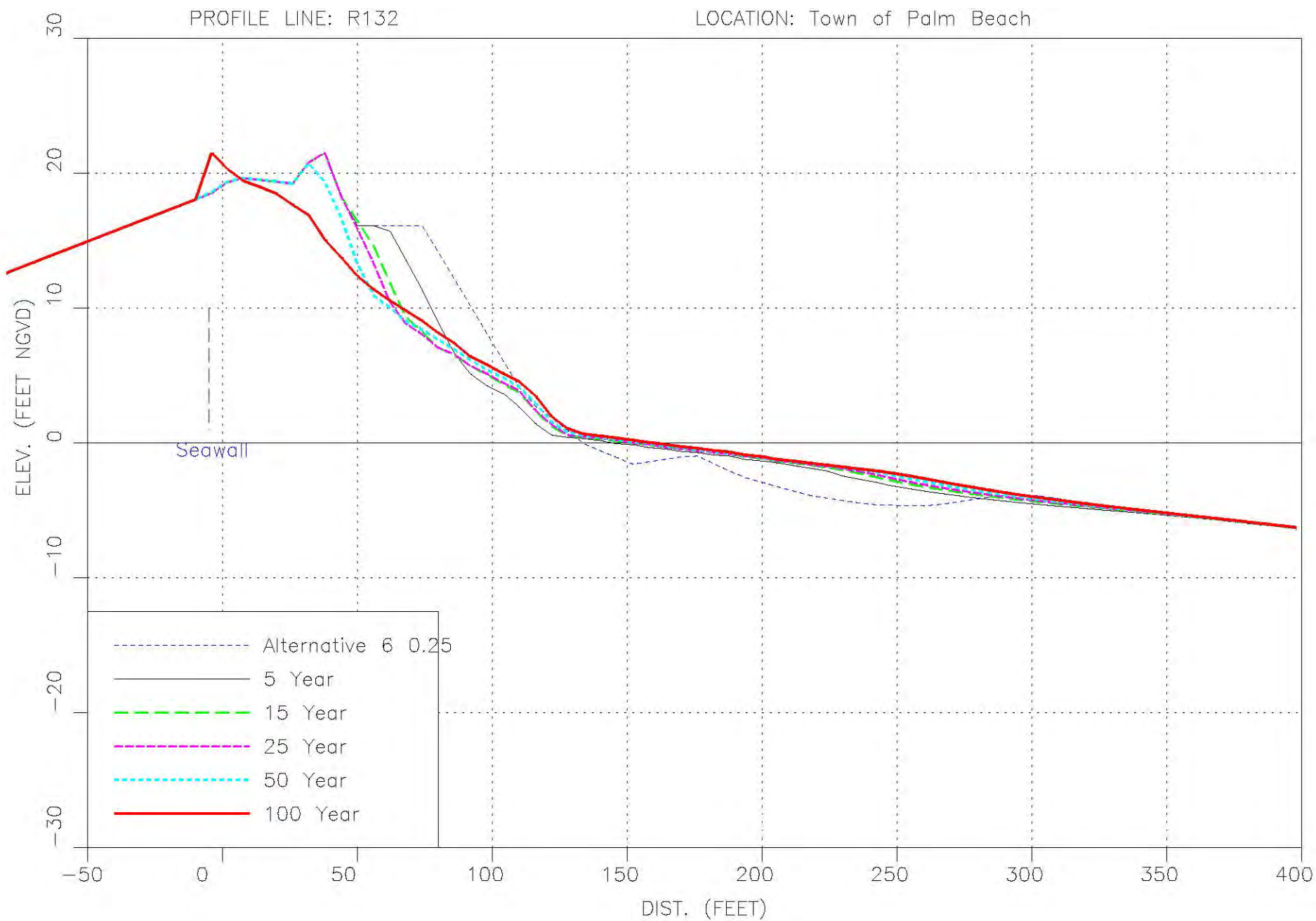
SUB-APPENDIX G-1
SBEACH ANALYSIS REPORT

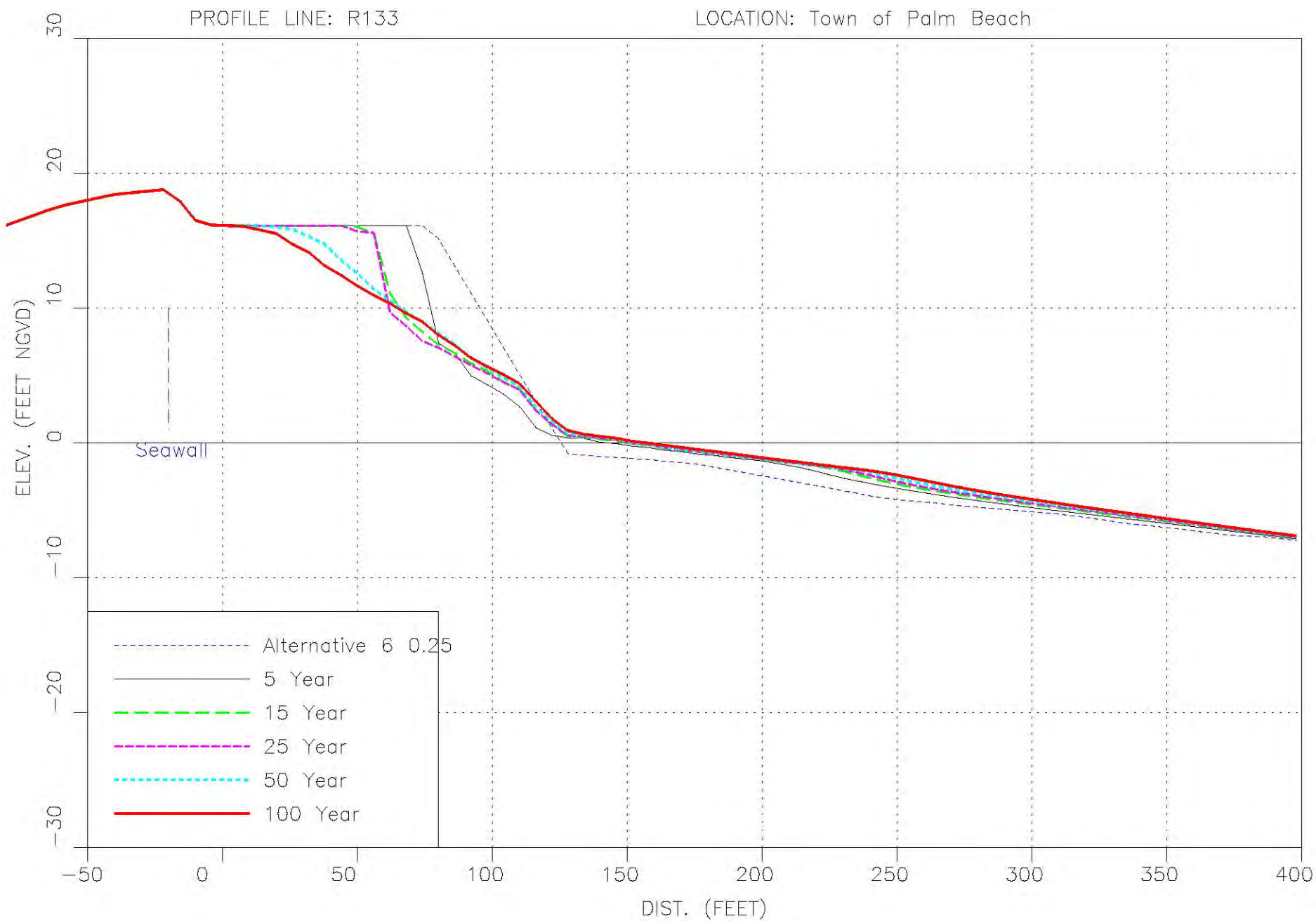
ATTACHMENT F-6
ALTERNATIVE 6 (LARGER FILL DESIGN) GRAIN SIZE 0.25

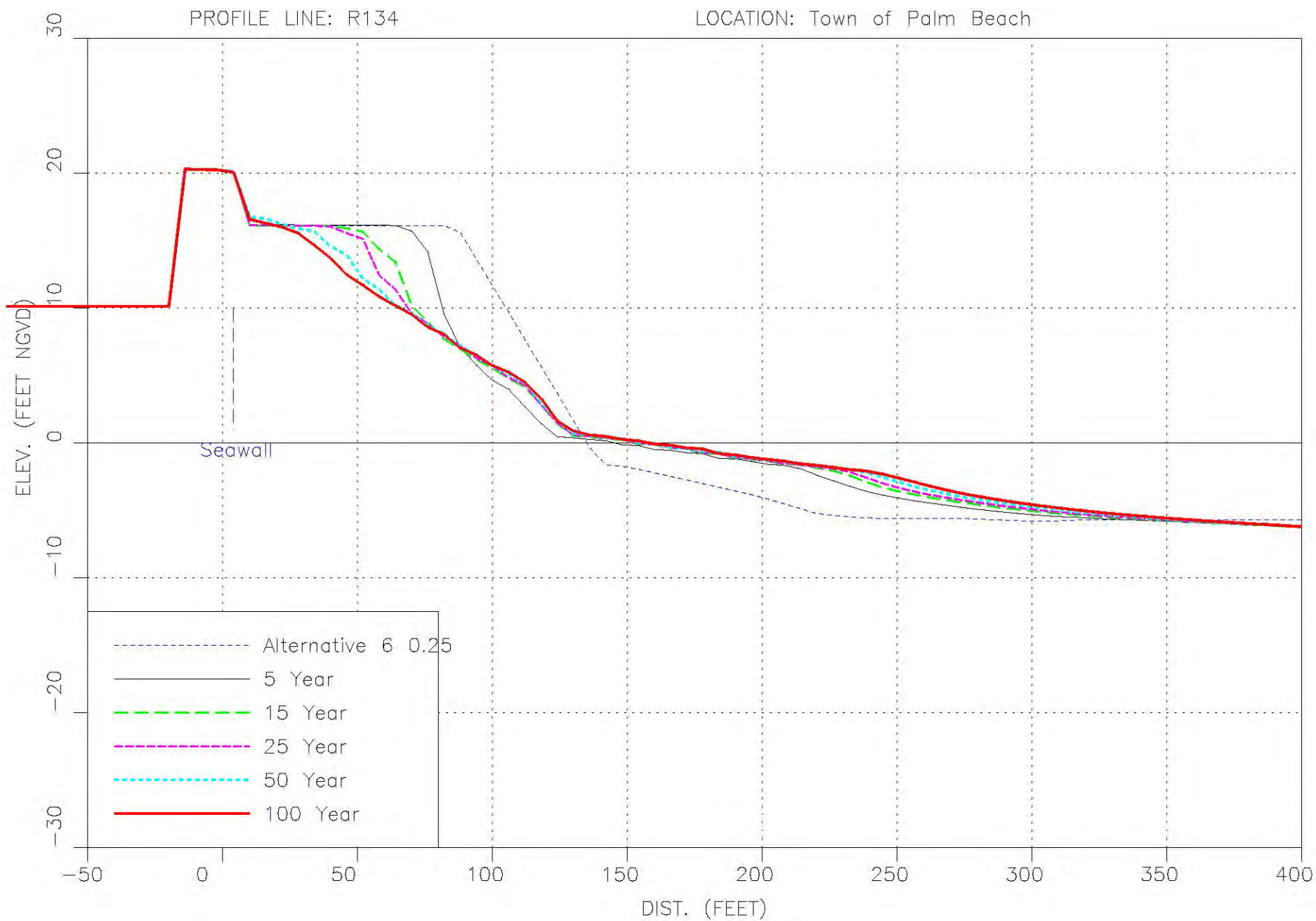






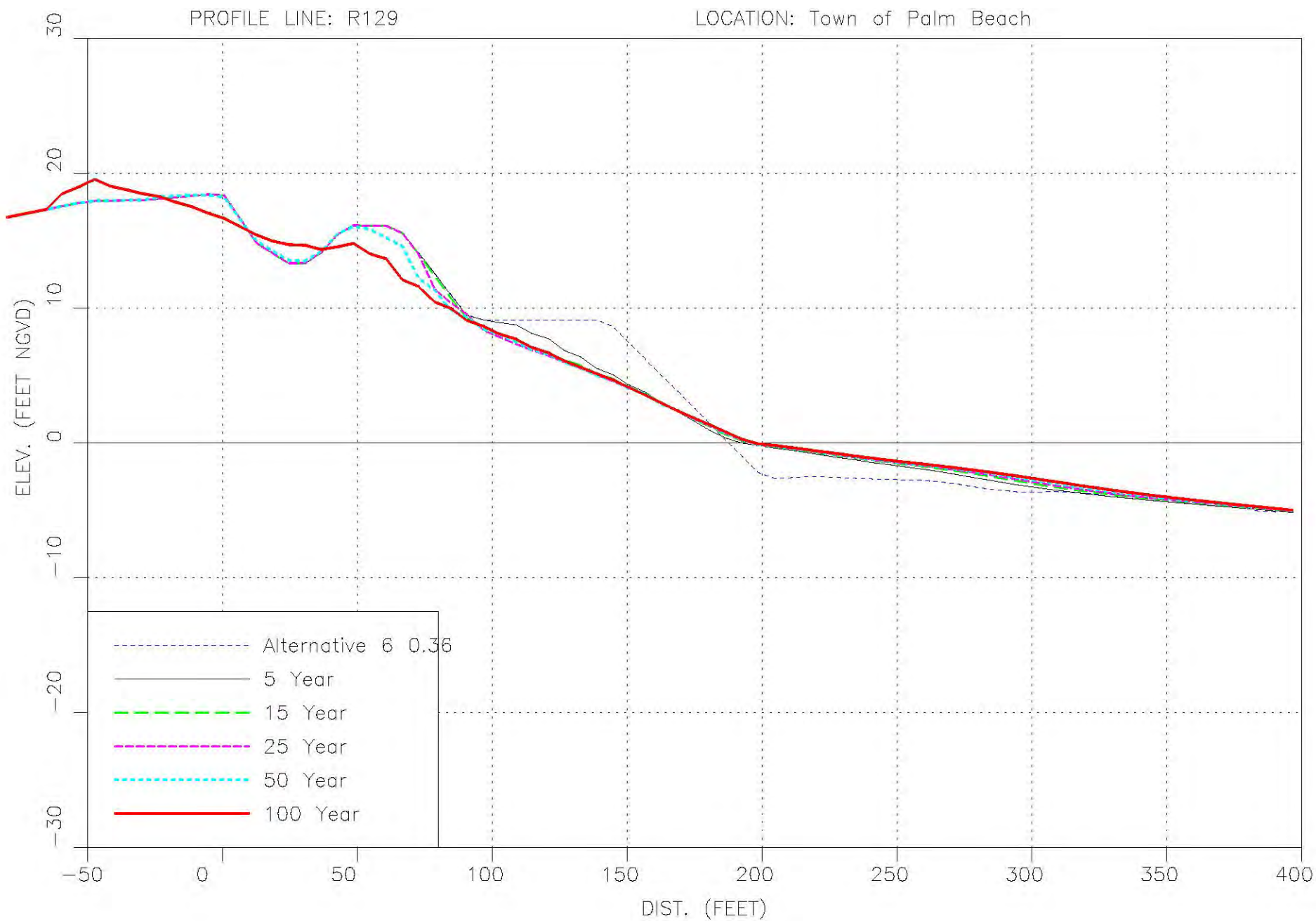


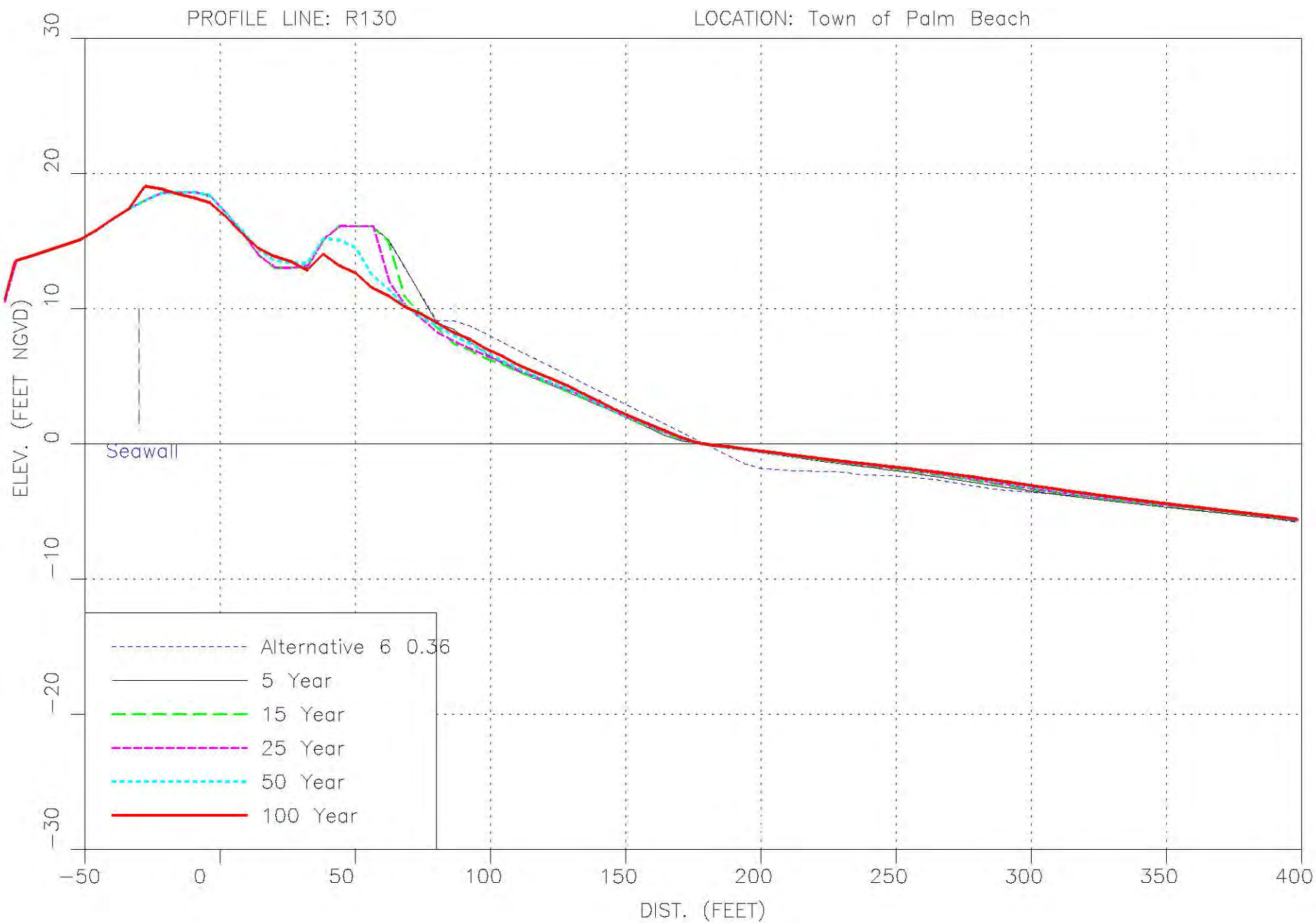


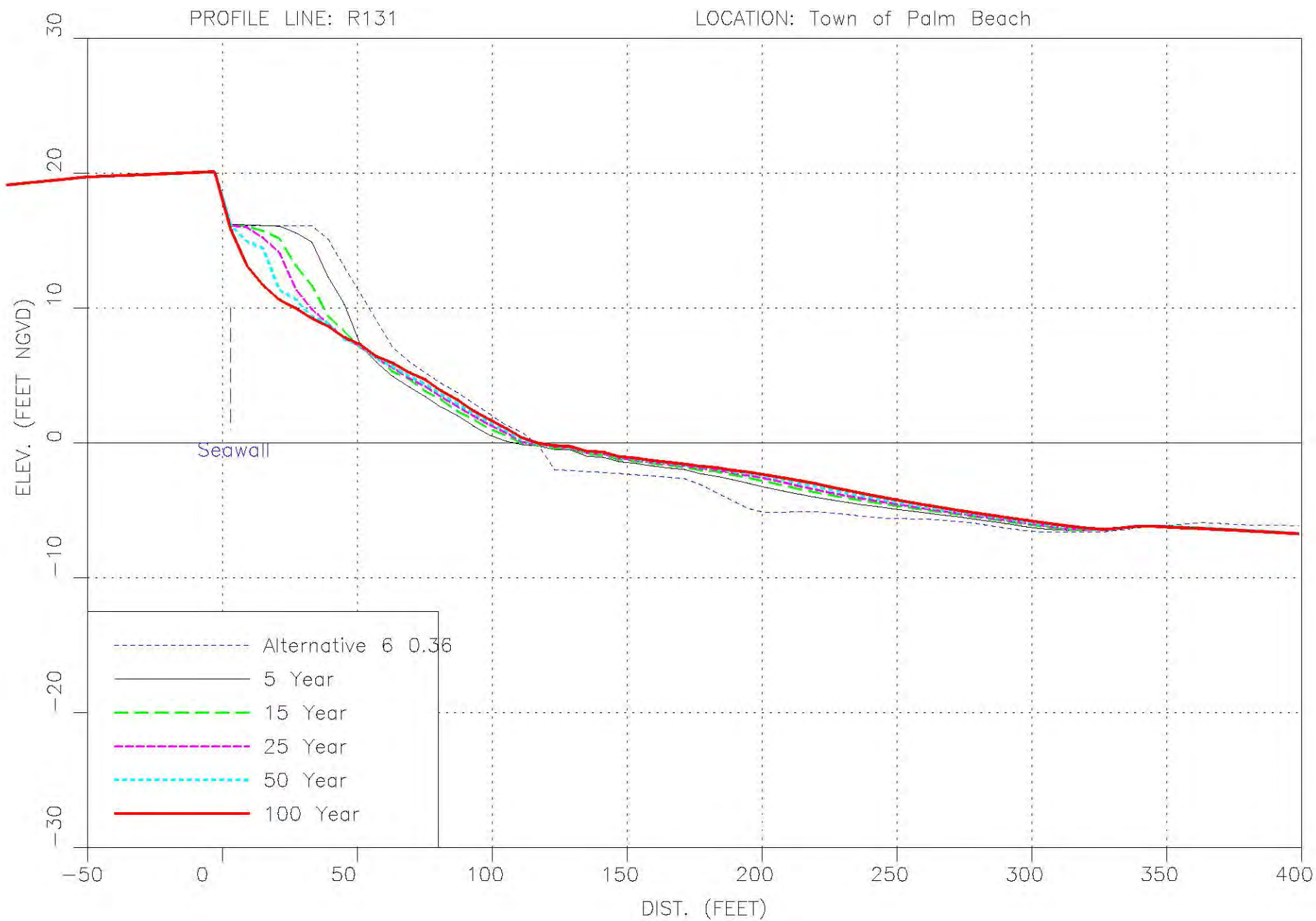


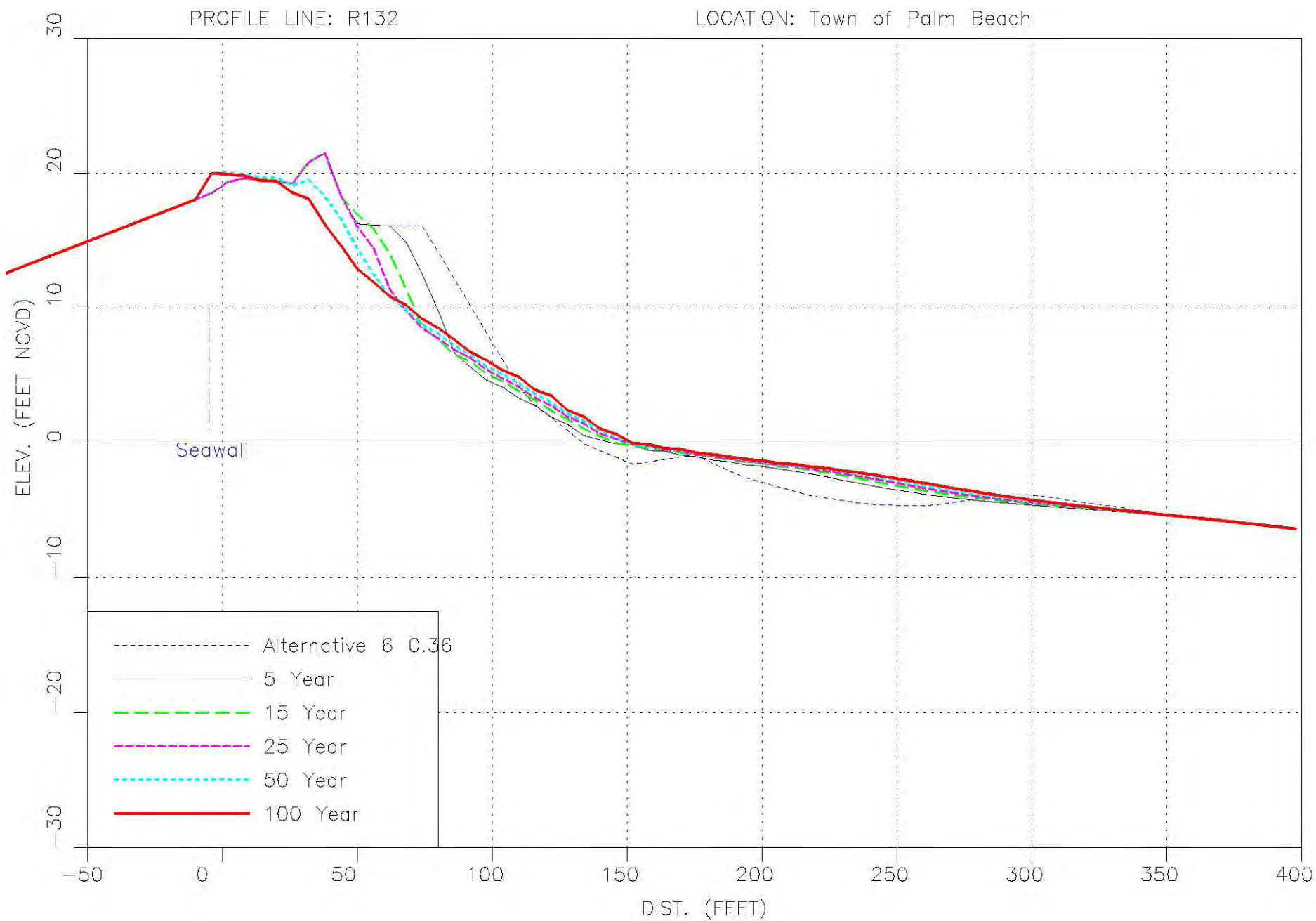
SUB-APPENDIX G-1
SBEACH ANALYSIS REPORT

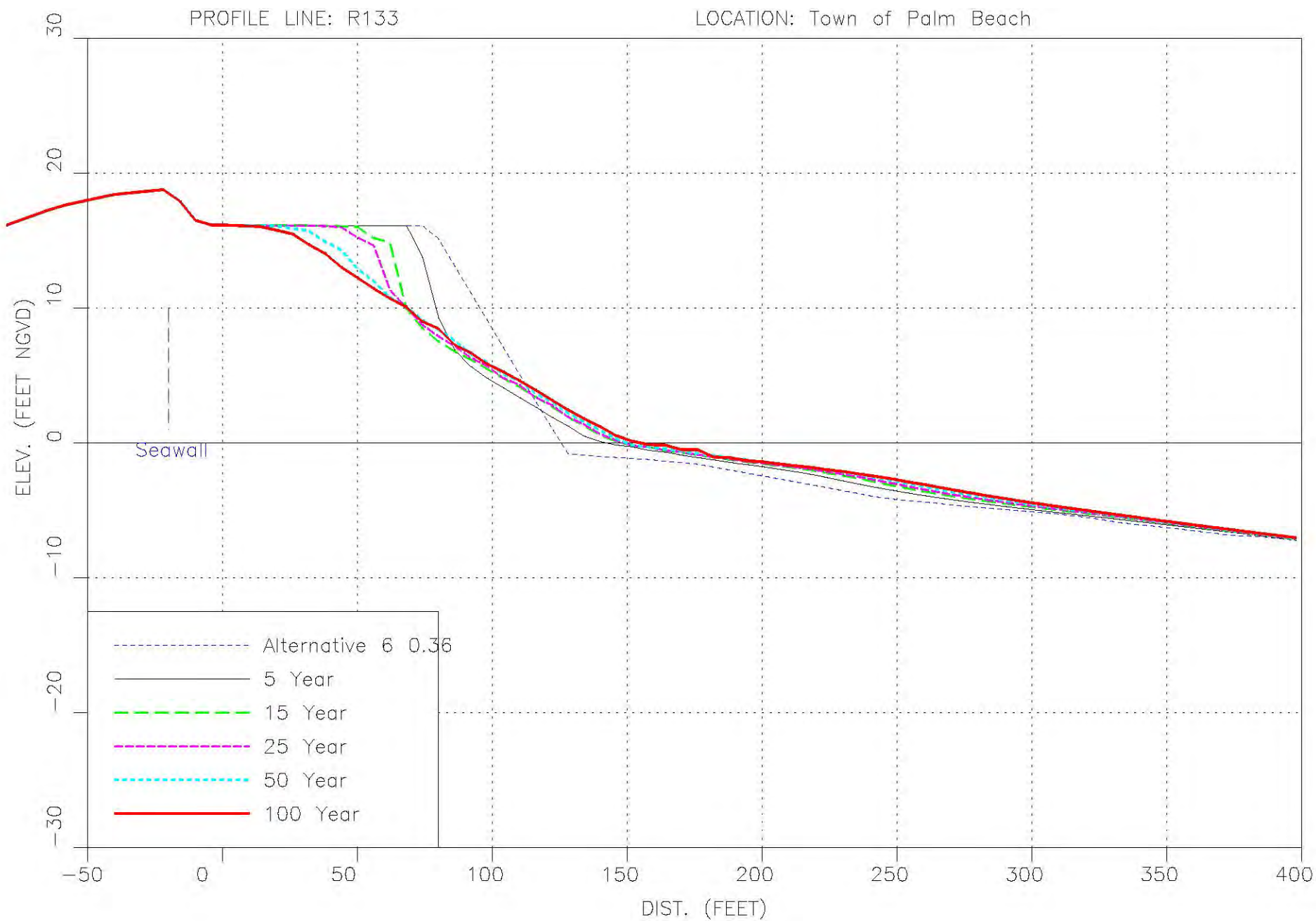
ATTACHMENT F-7
ALTERNATIVE 6 (LARGER FILL DESIGN) GRAIN SIZE 0.36

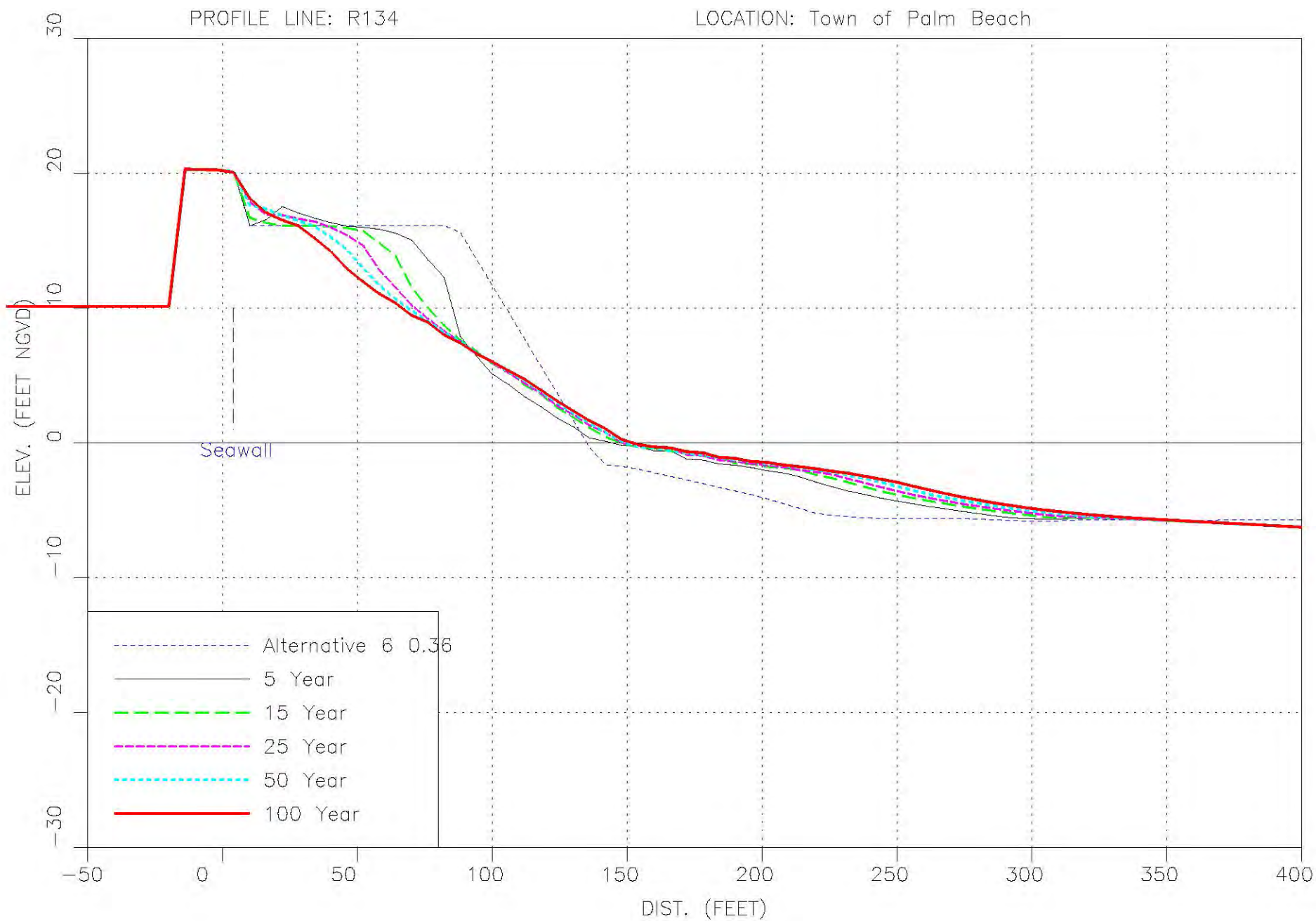






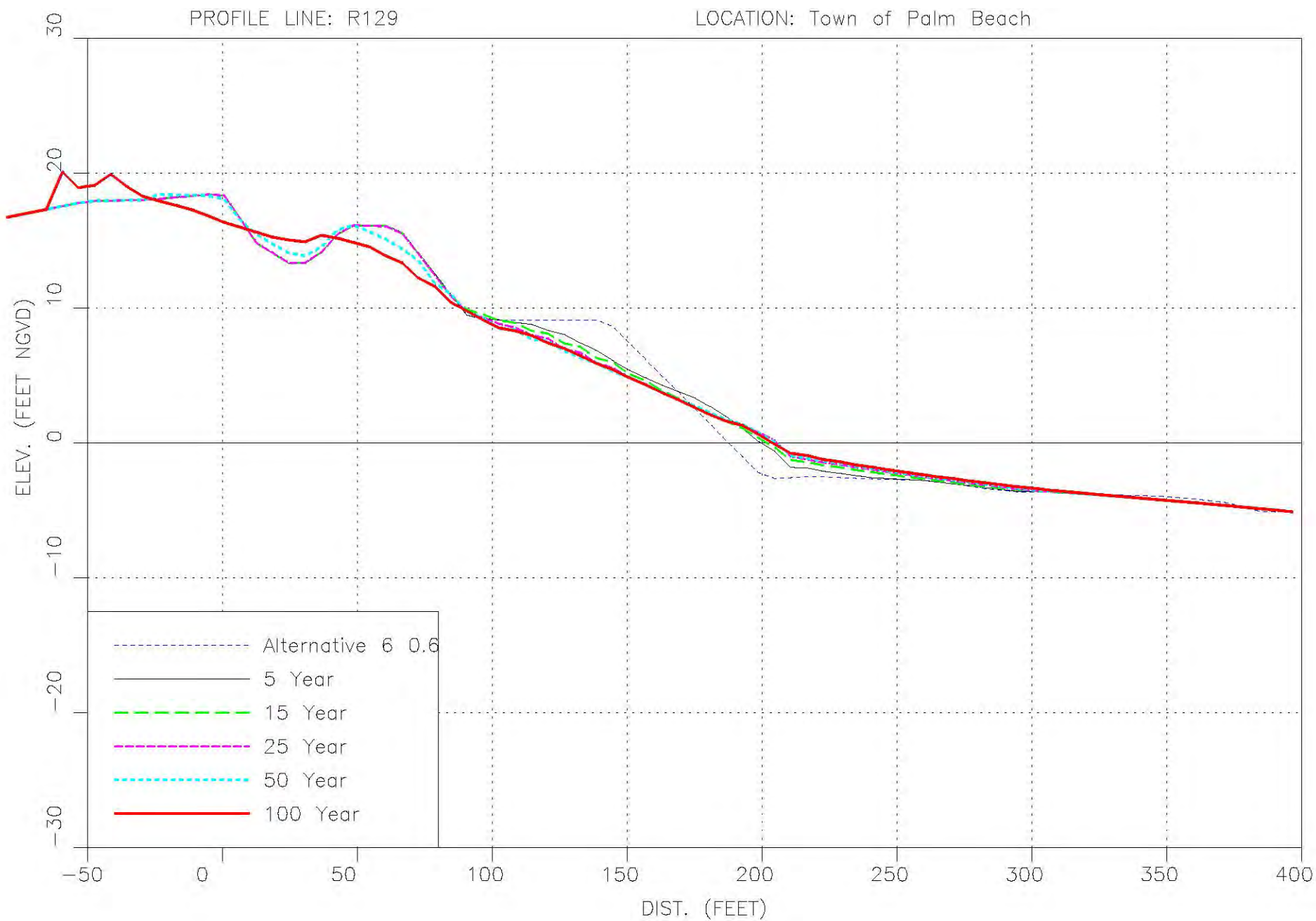


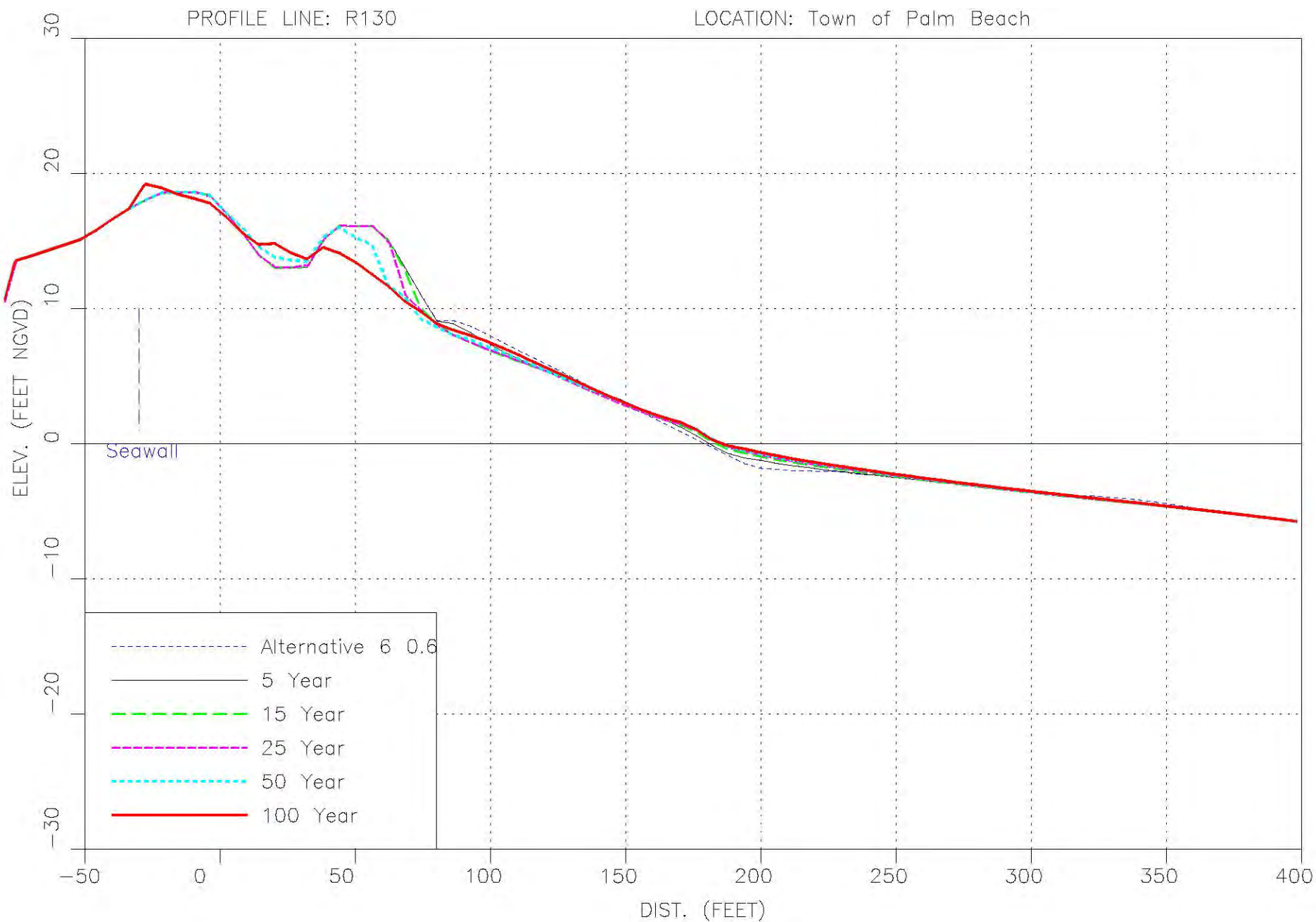


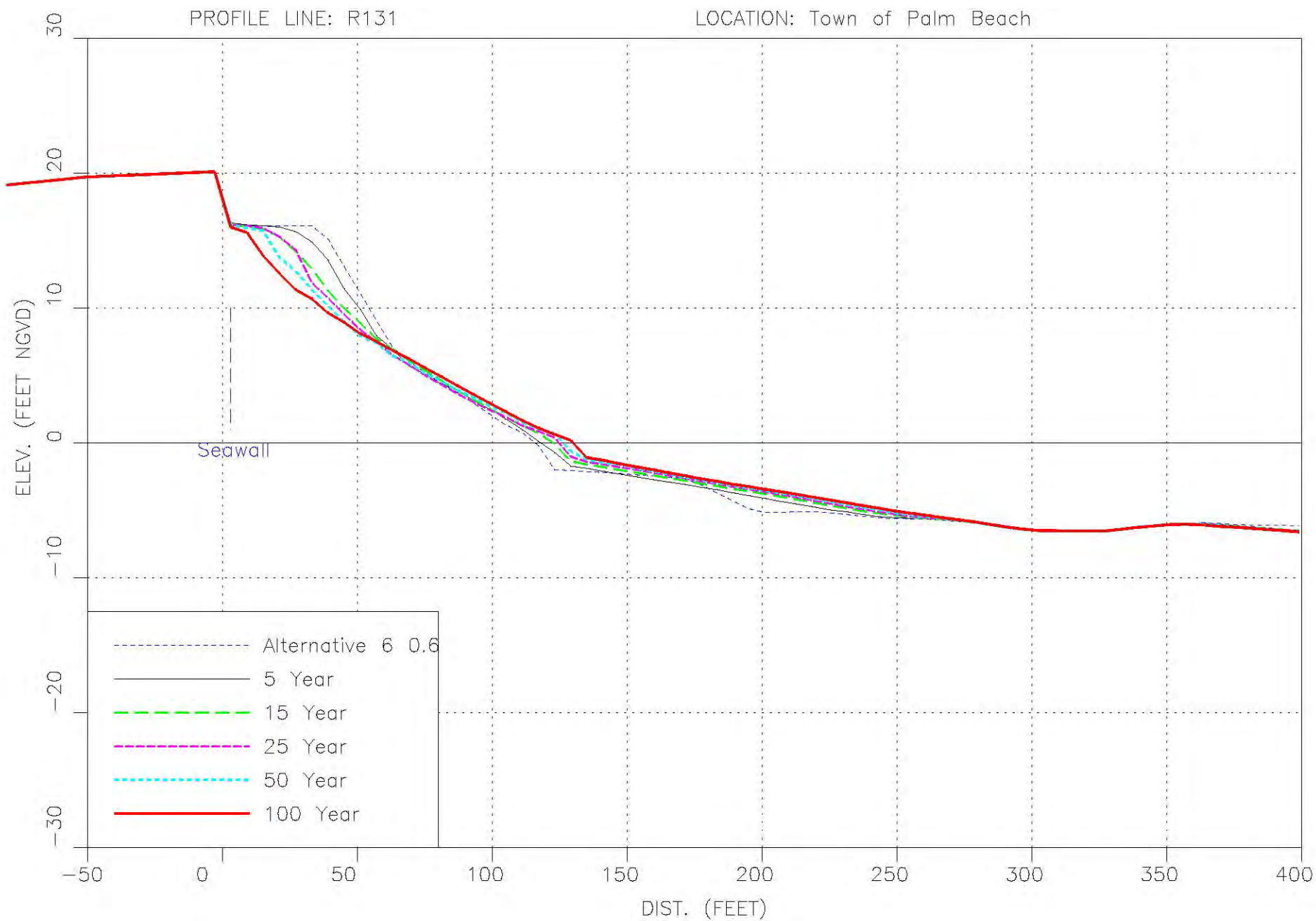


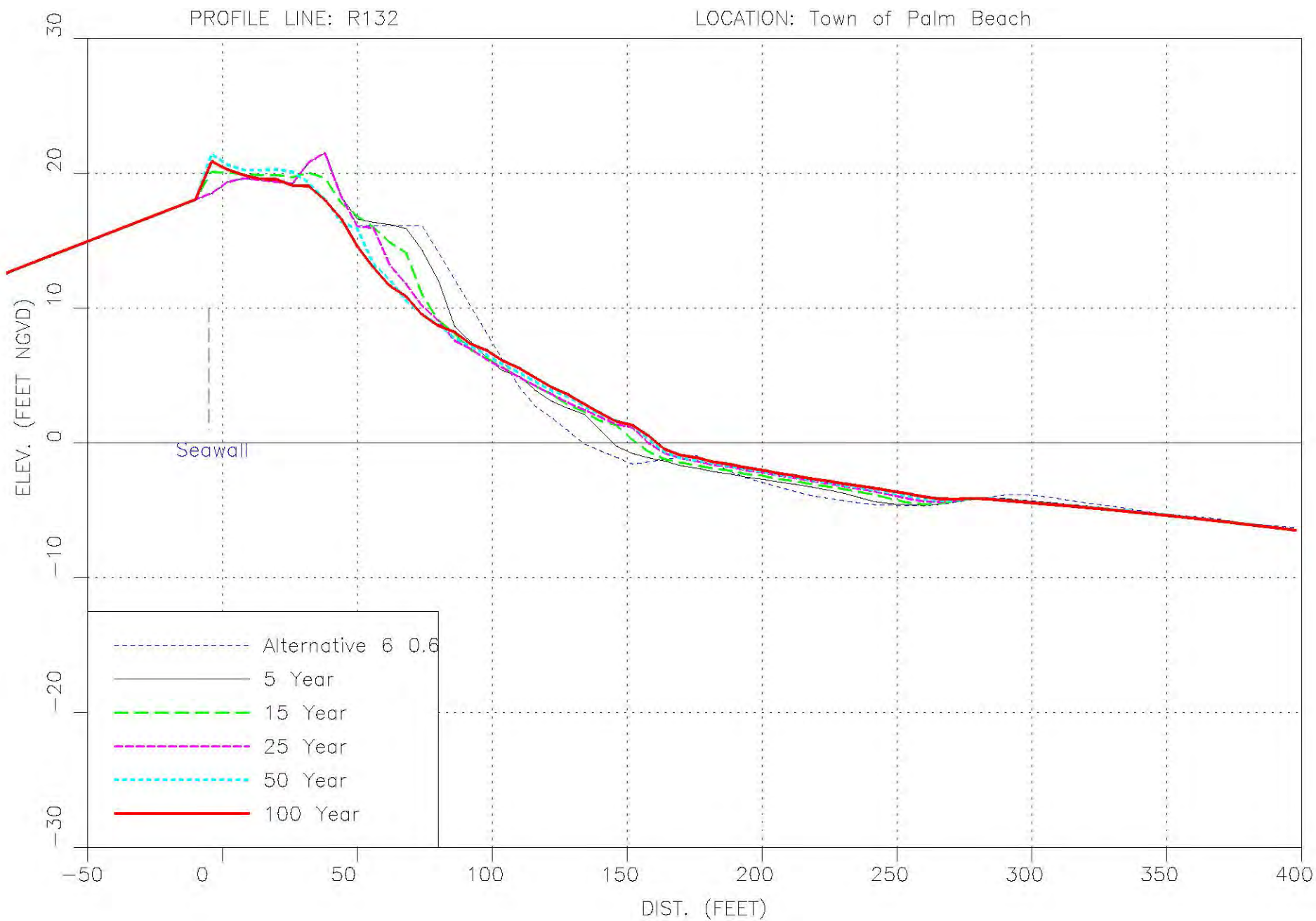
SUB-APPENDIX G-1
SBEACH ANALYSIS REPORT

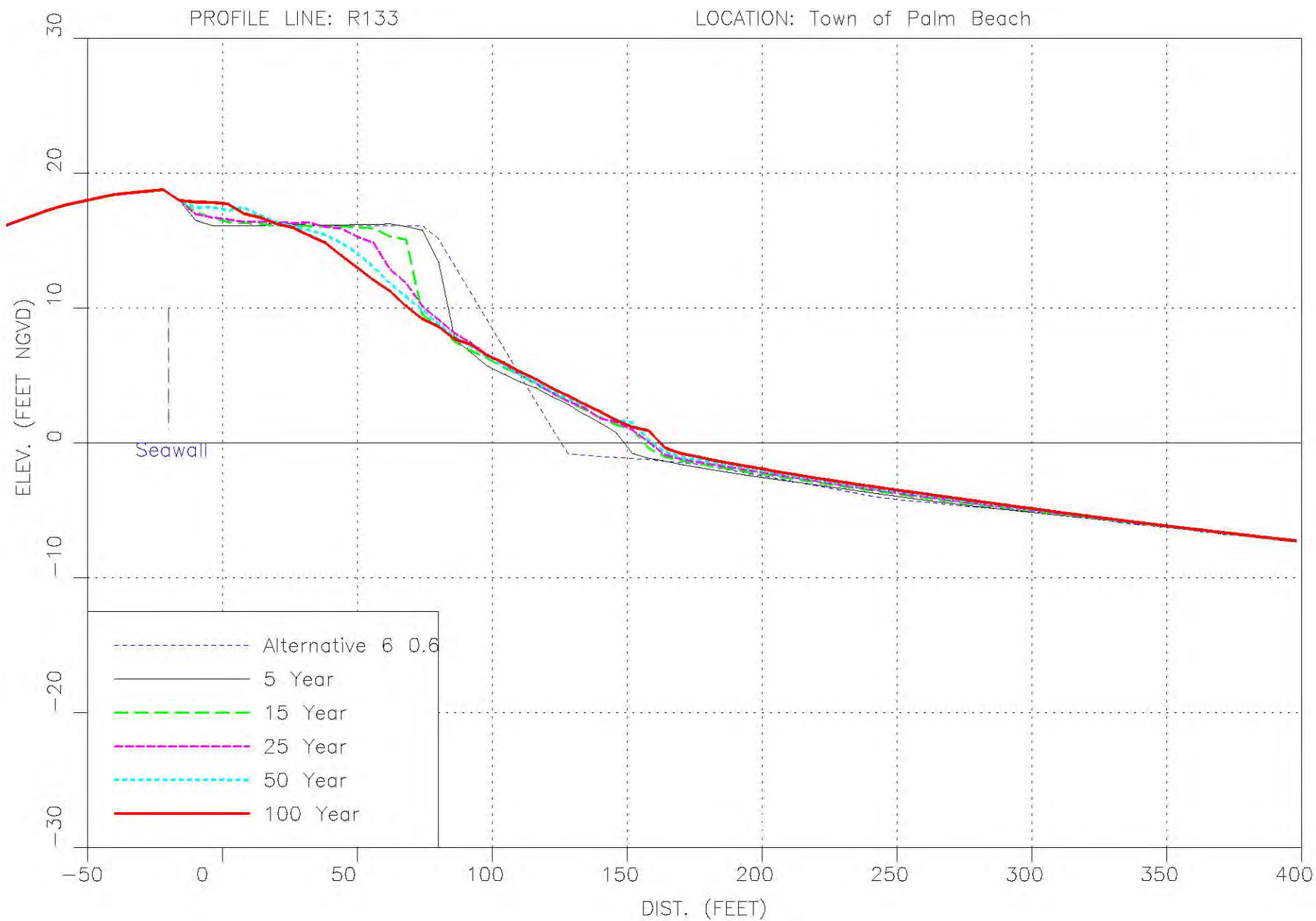
ATTACHMENT F-8
ALTERNATIVE 6 (LARGER FILL DESIGN) GRAIN SIZE 0.6

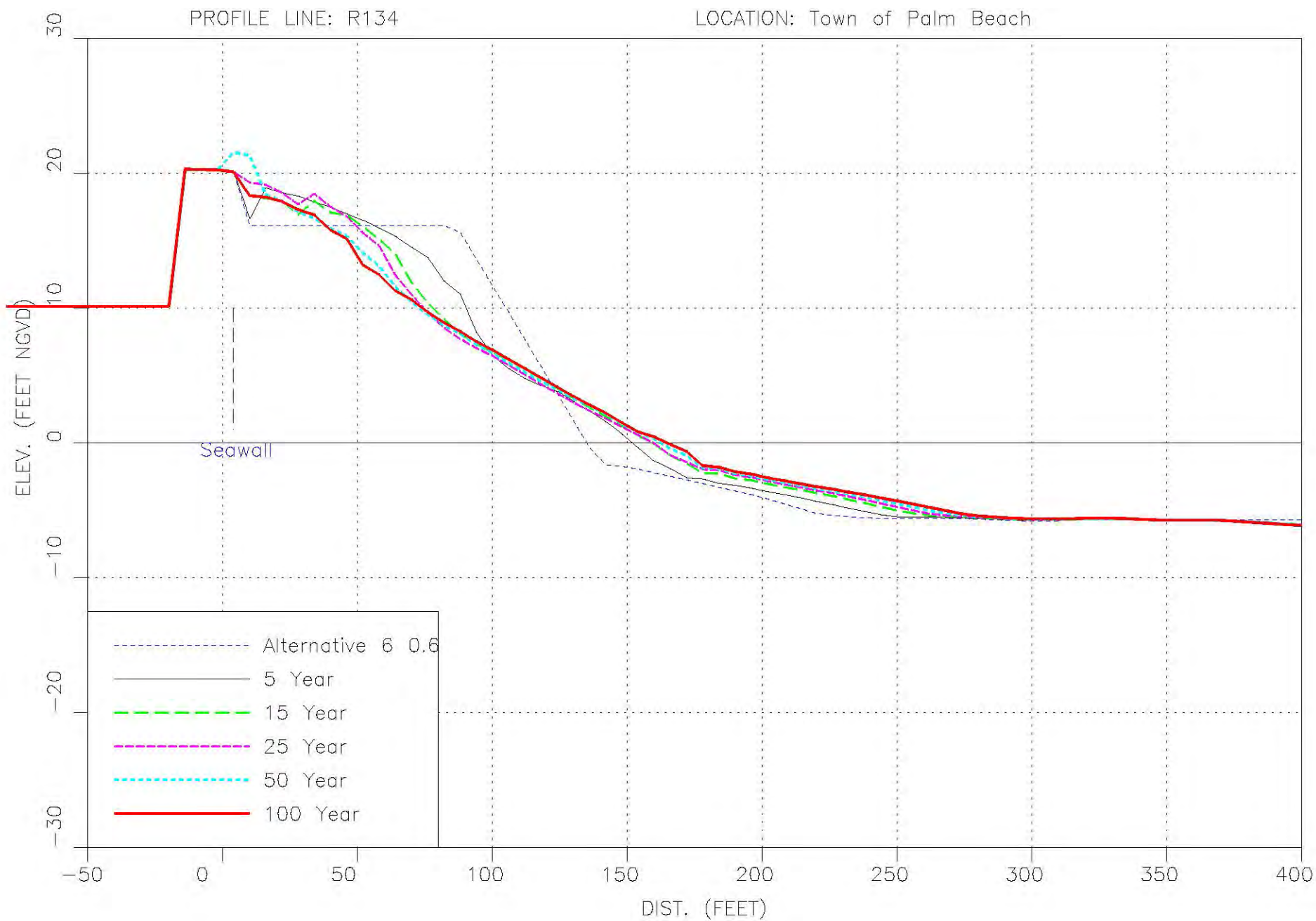






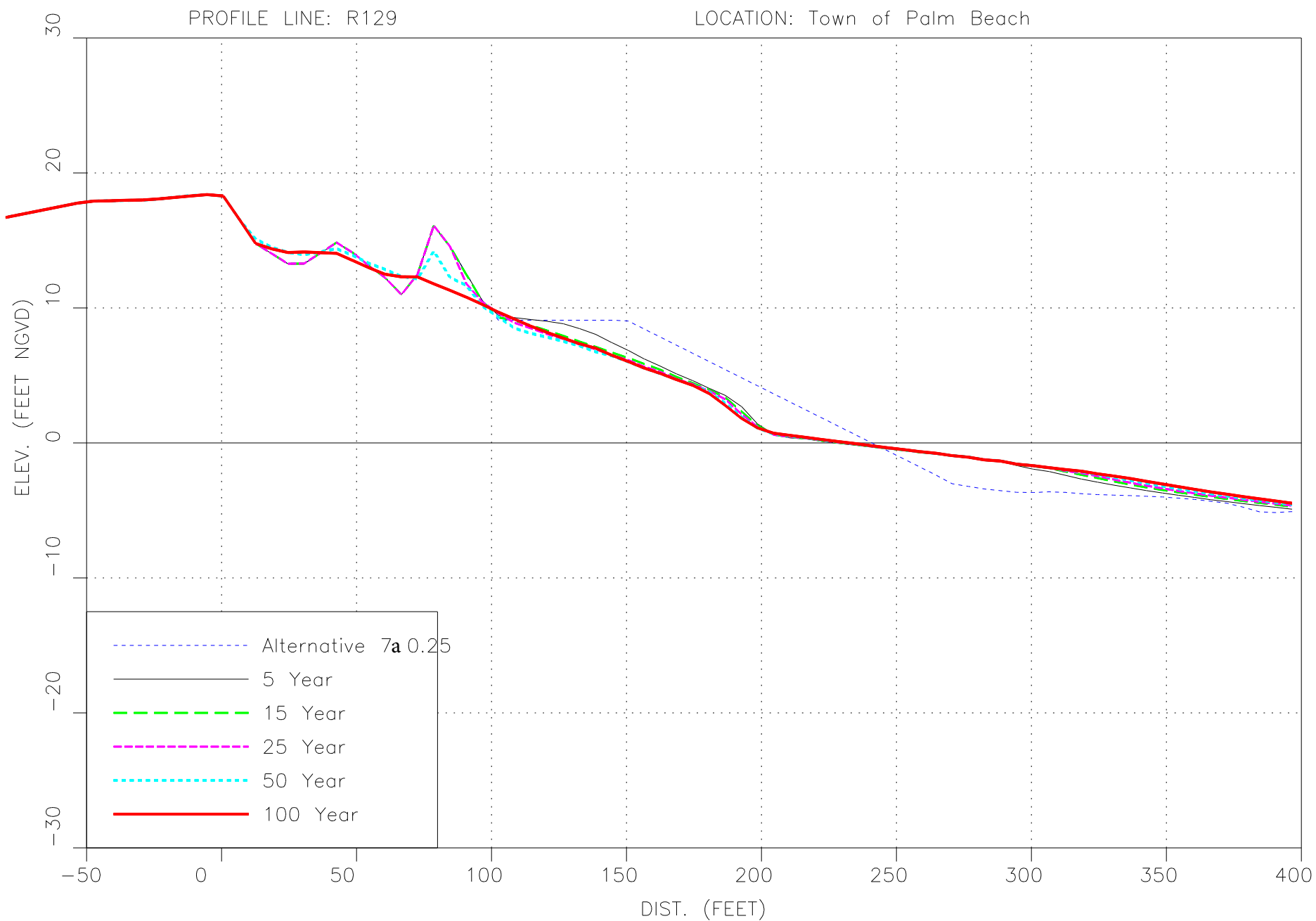


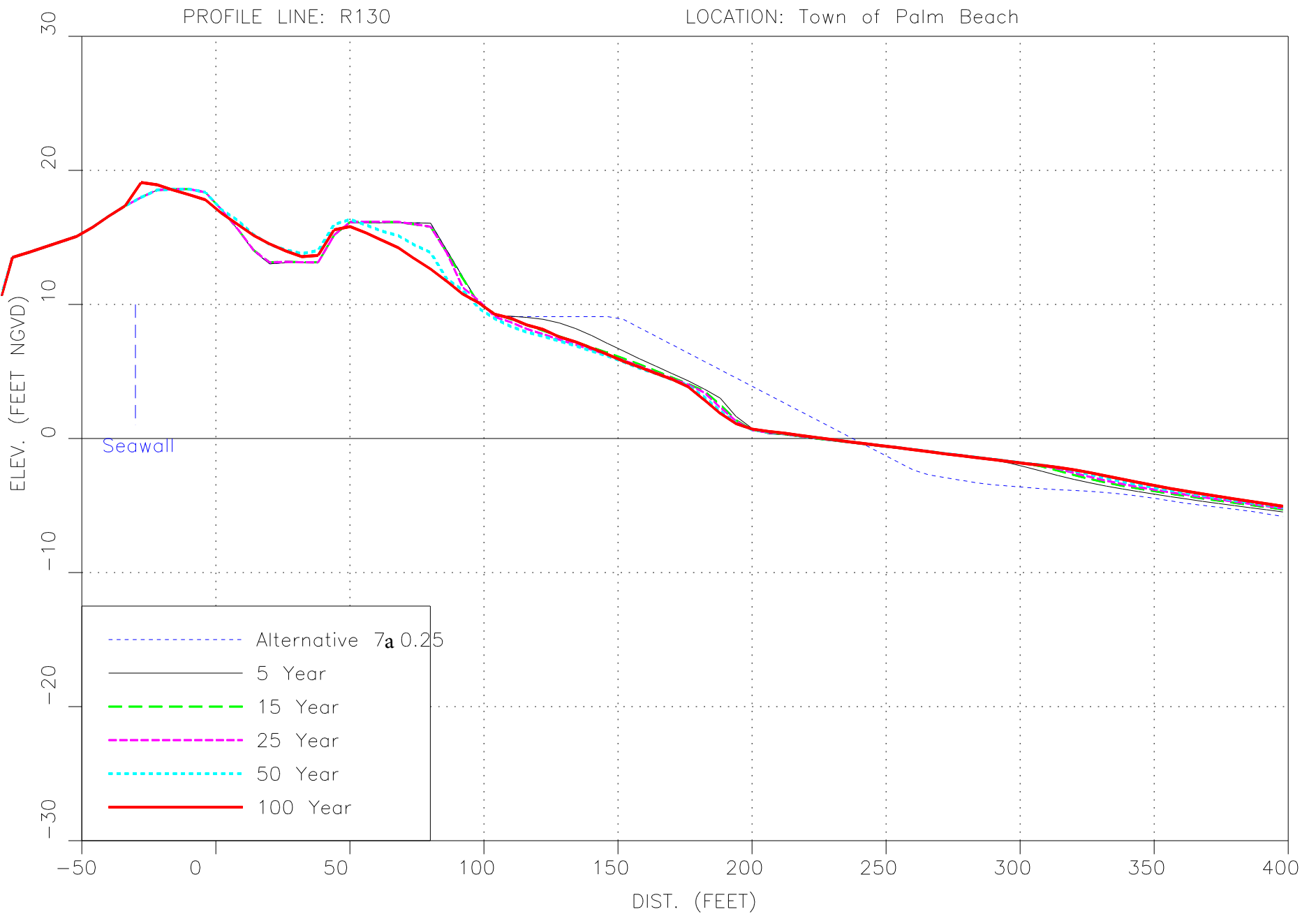


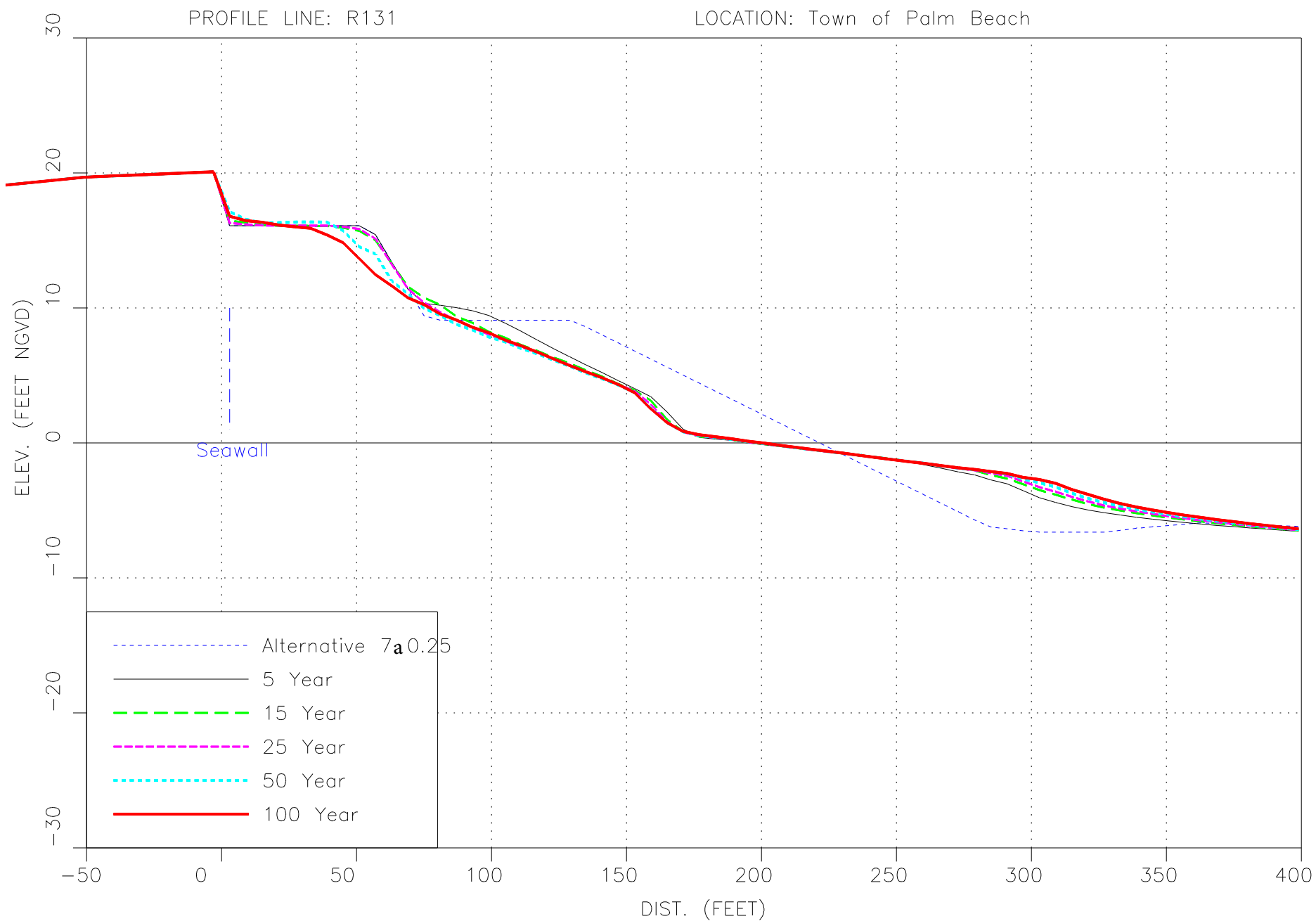


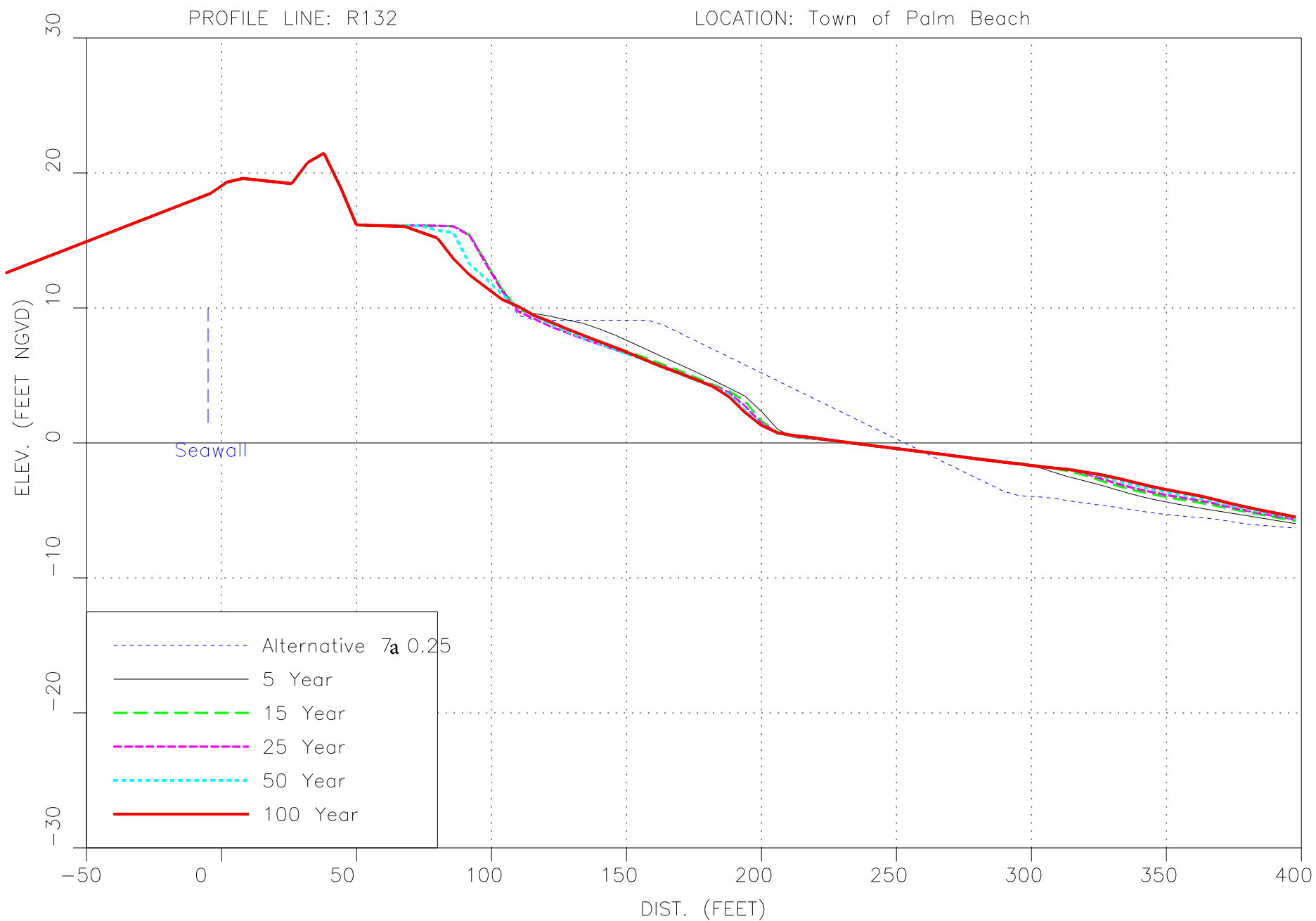
SUB-APPENDIX G-1
SBEACH ANALYSIS REPORT

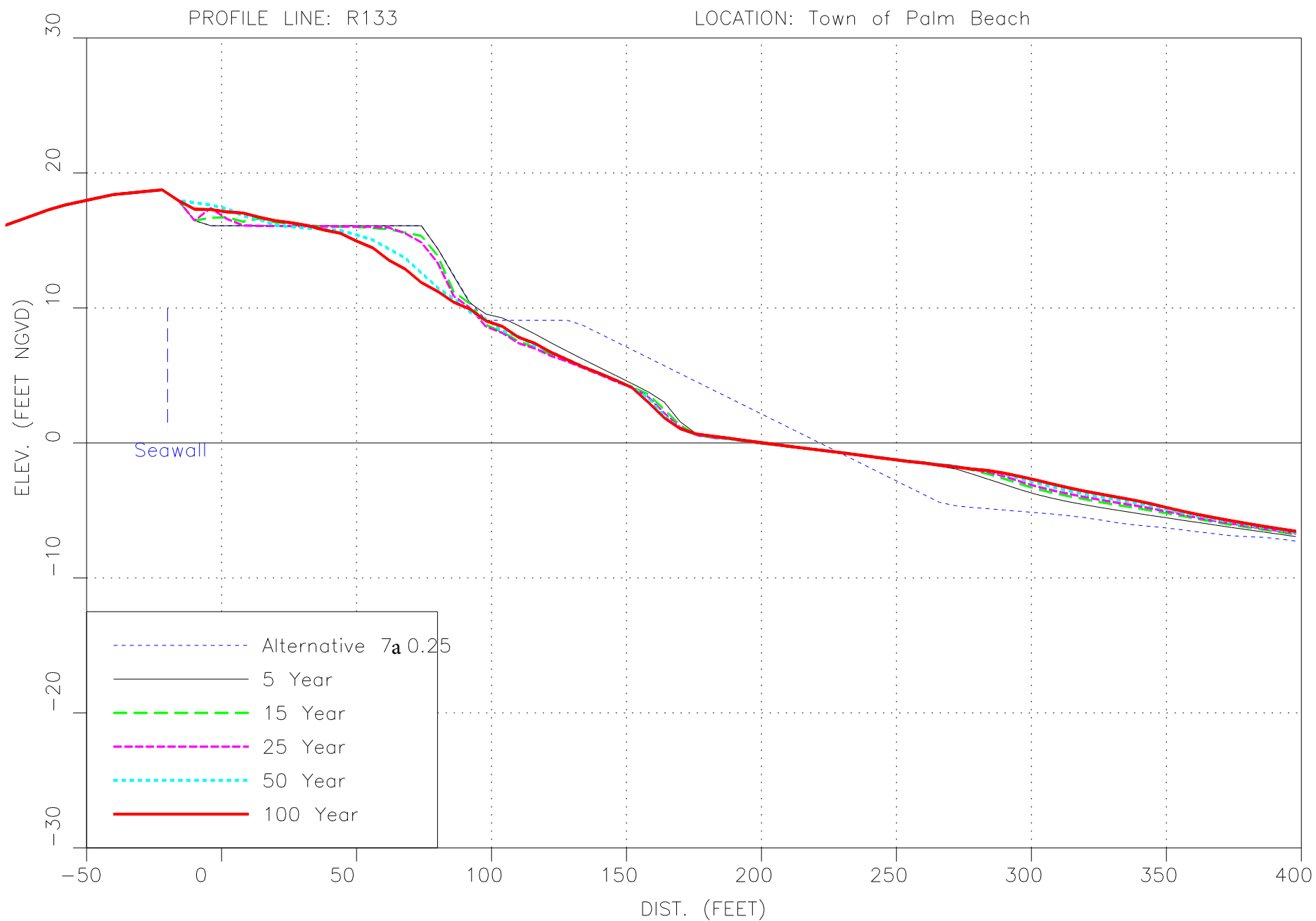
ATTACHMENT F-9
ALTERNATIVE 7a (SOS) GRAIN SIZE 0.25

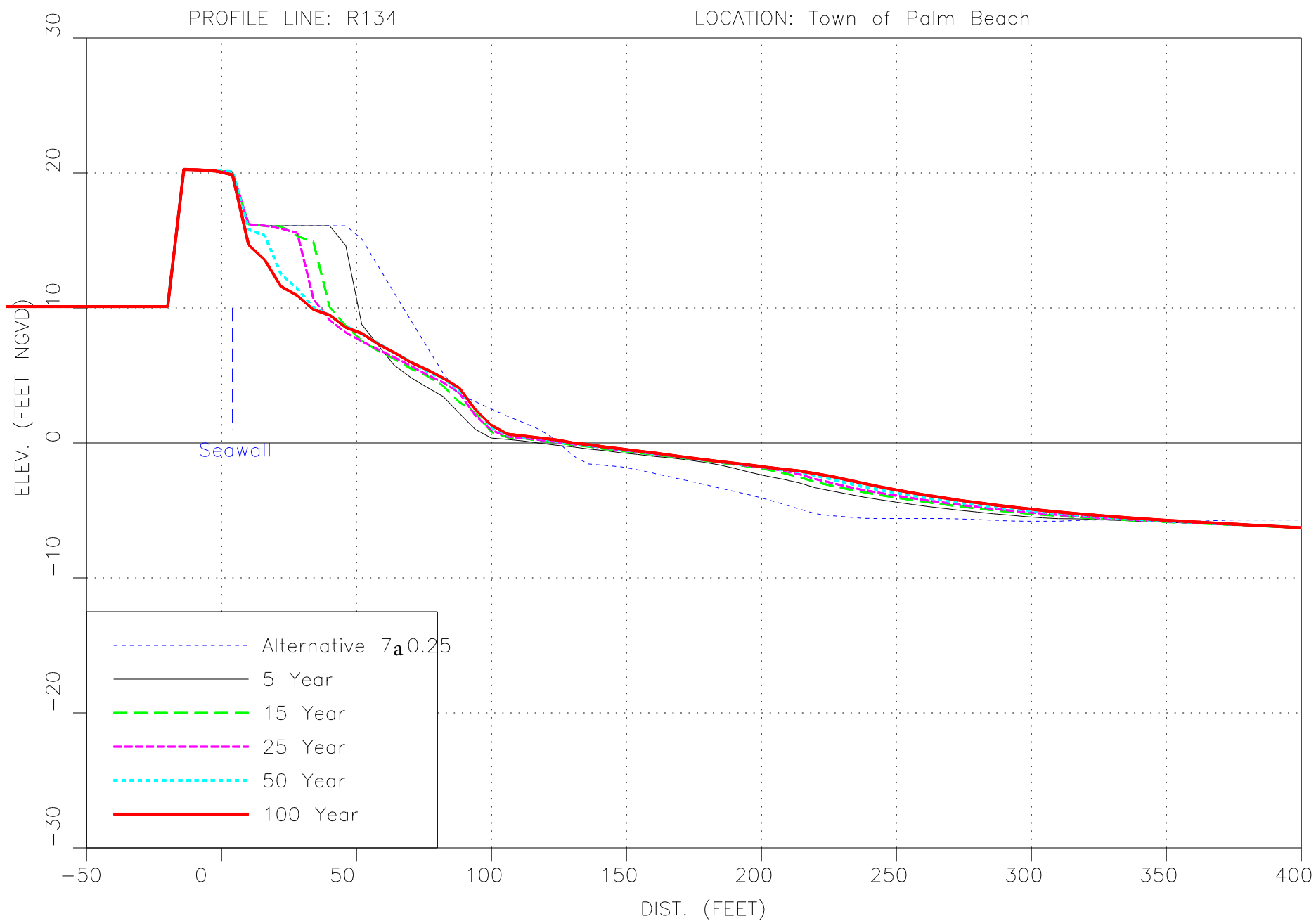






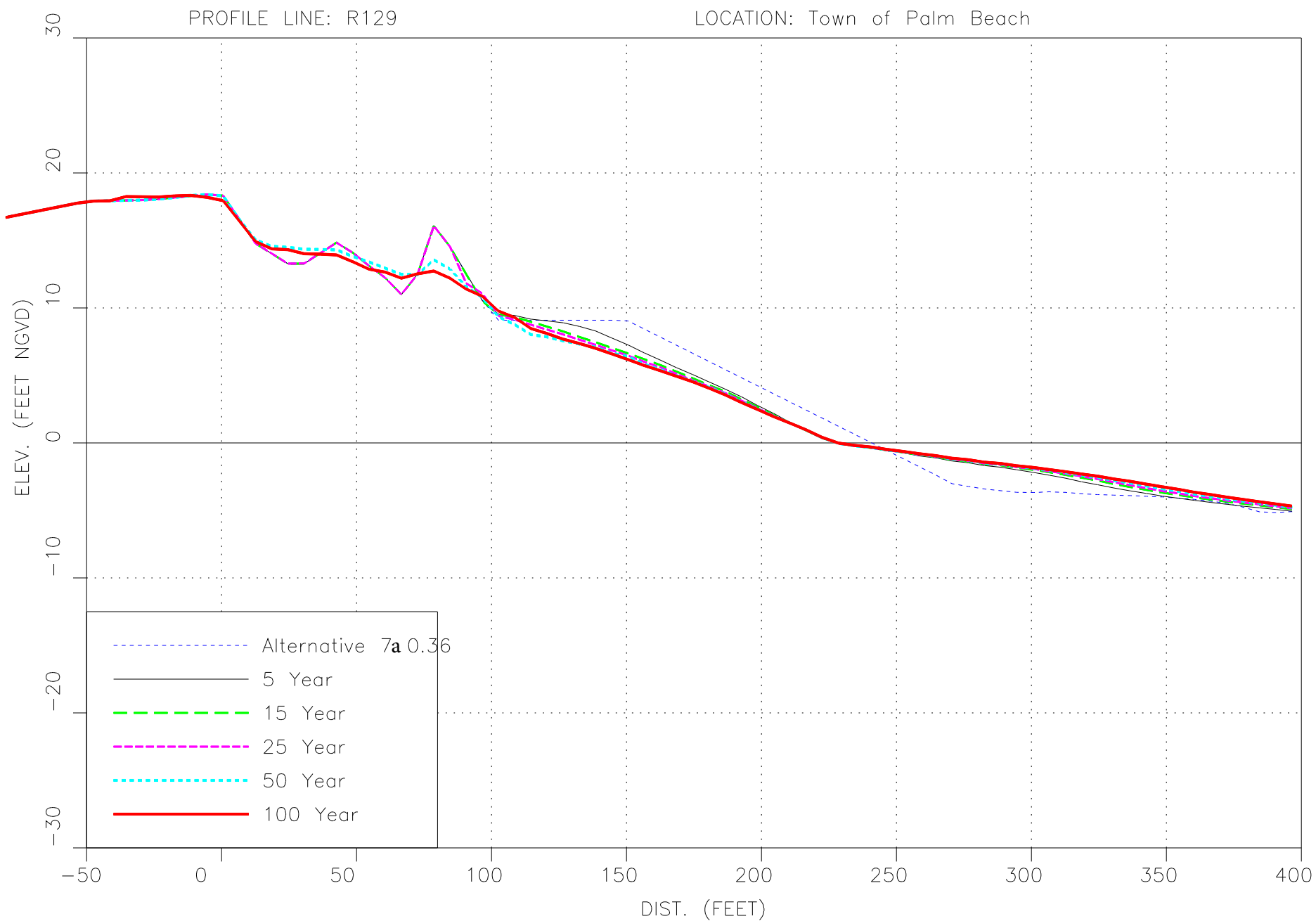


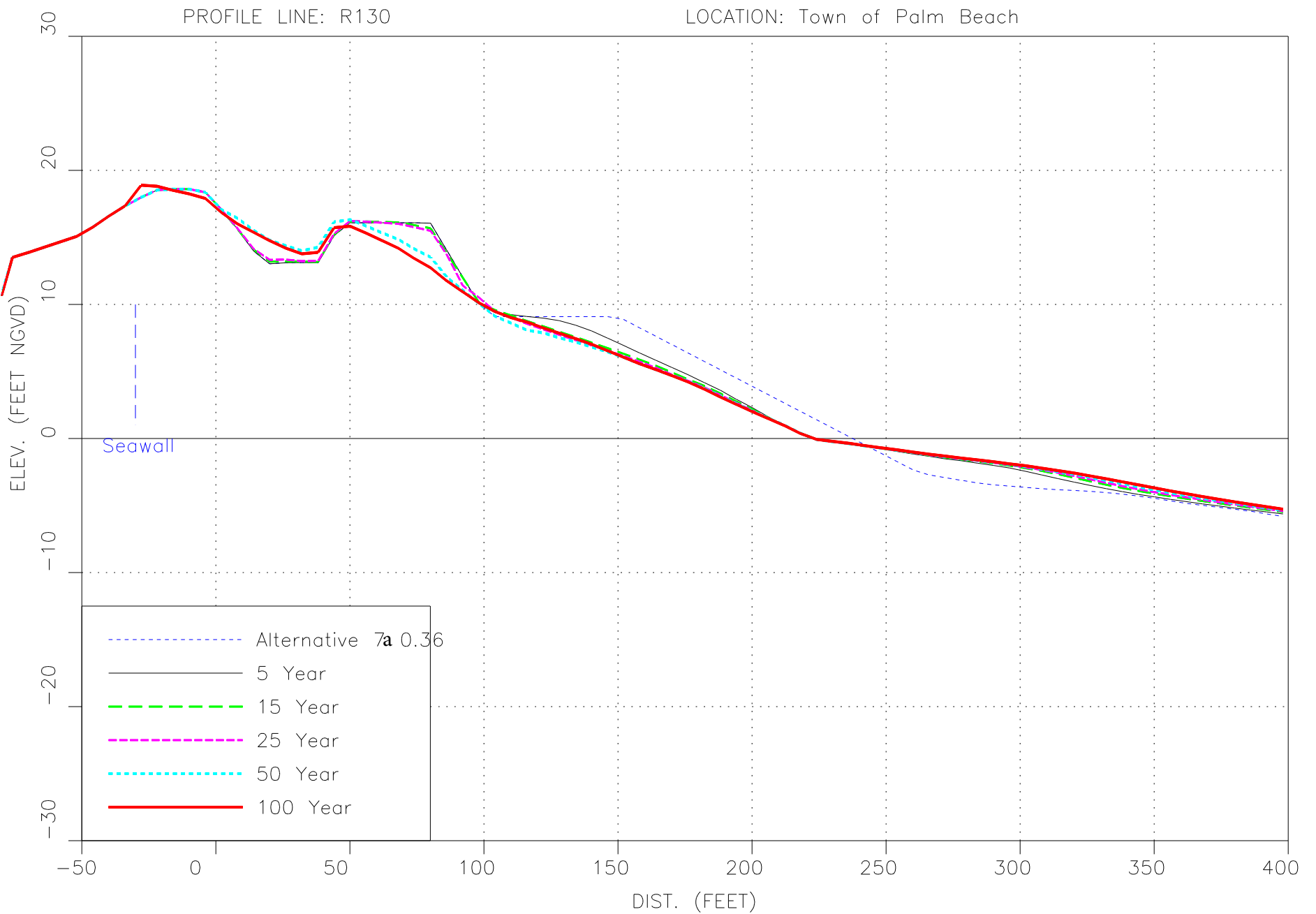


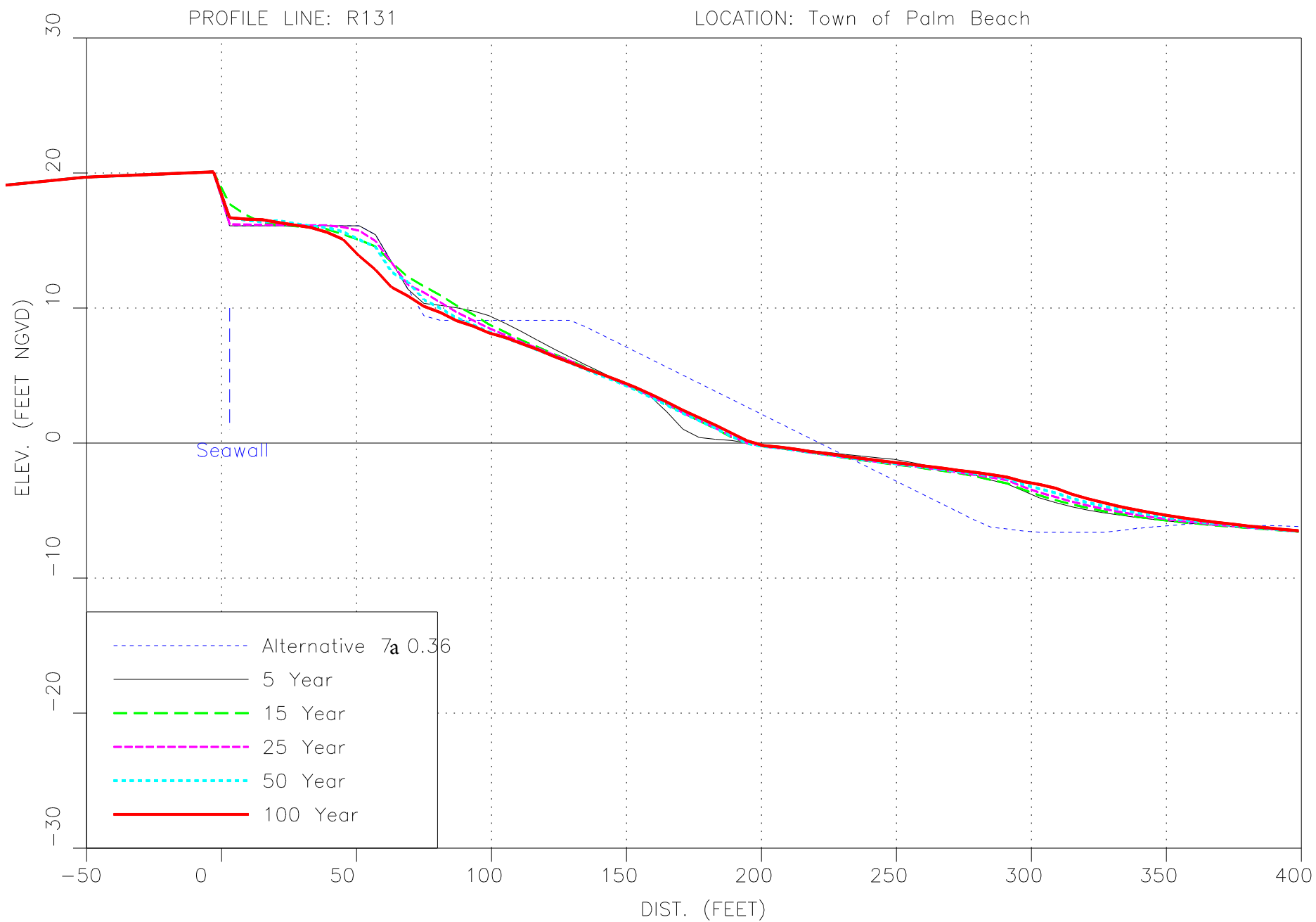


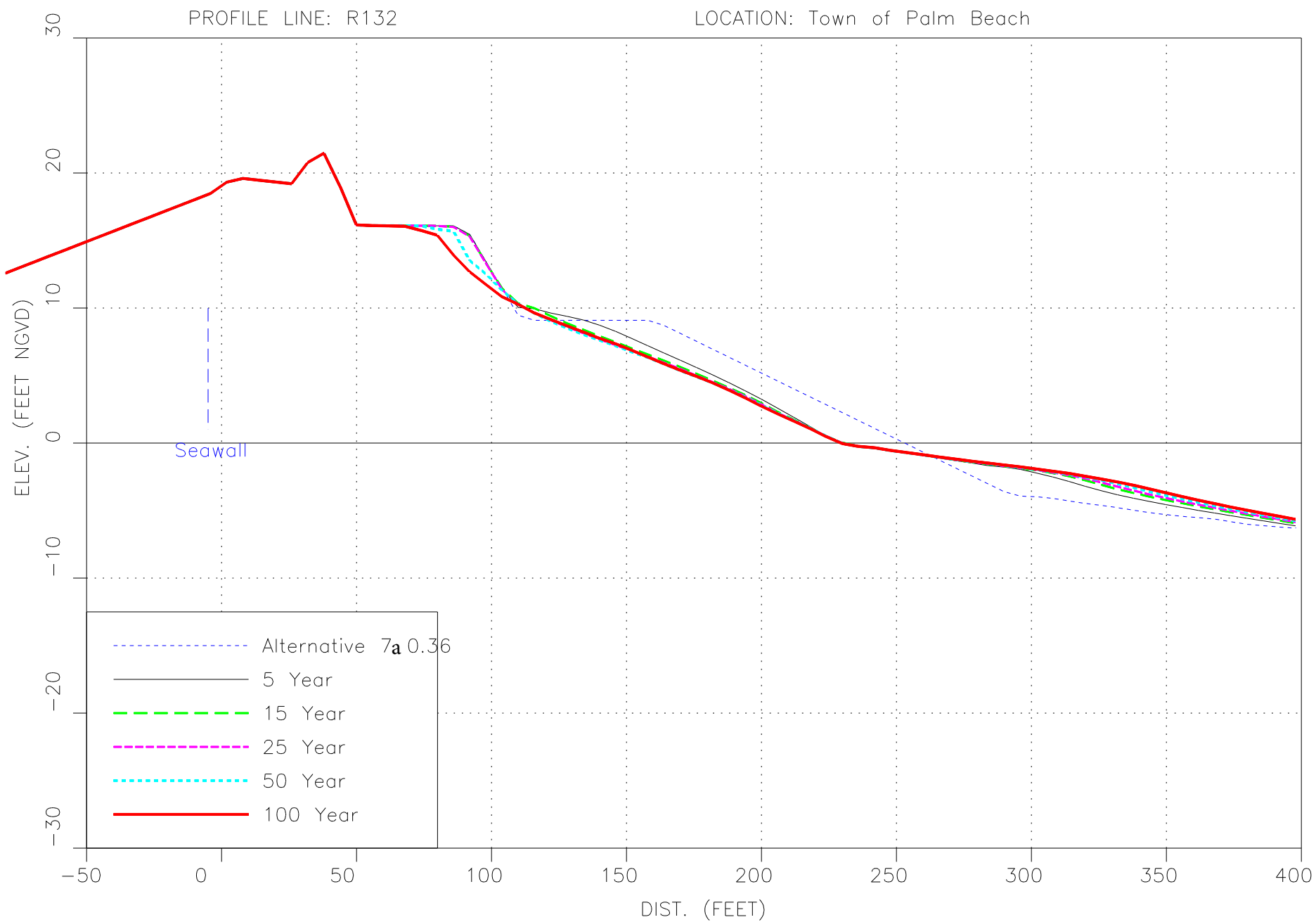
SUB-APPENDIX G-1
SBEACH ANALYSIS REPORT

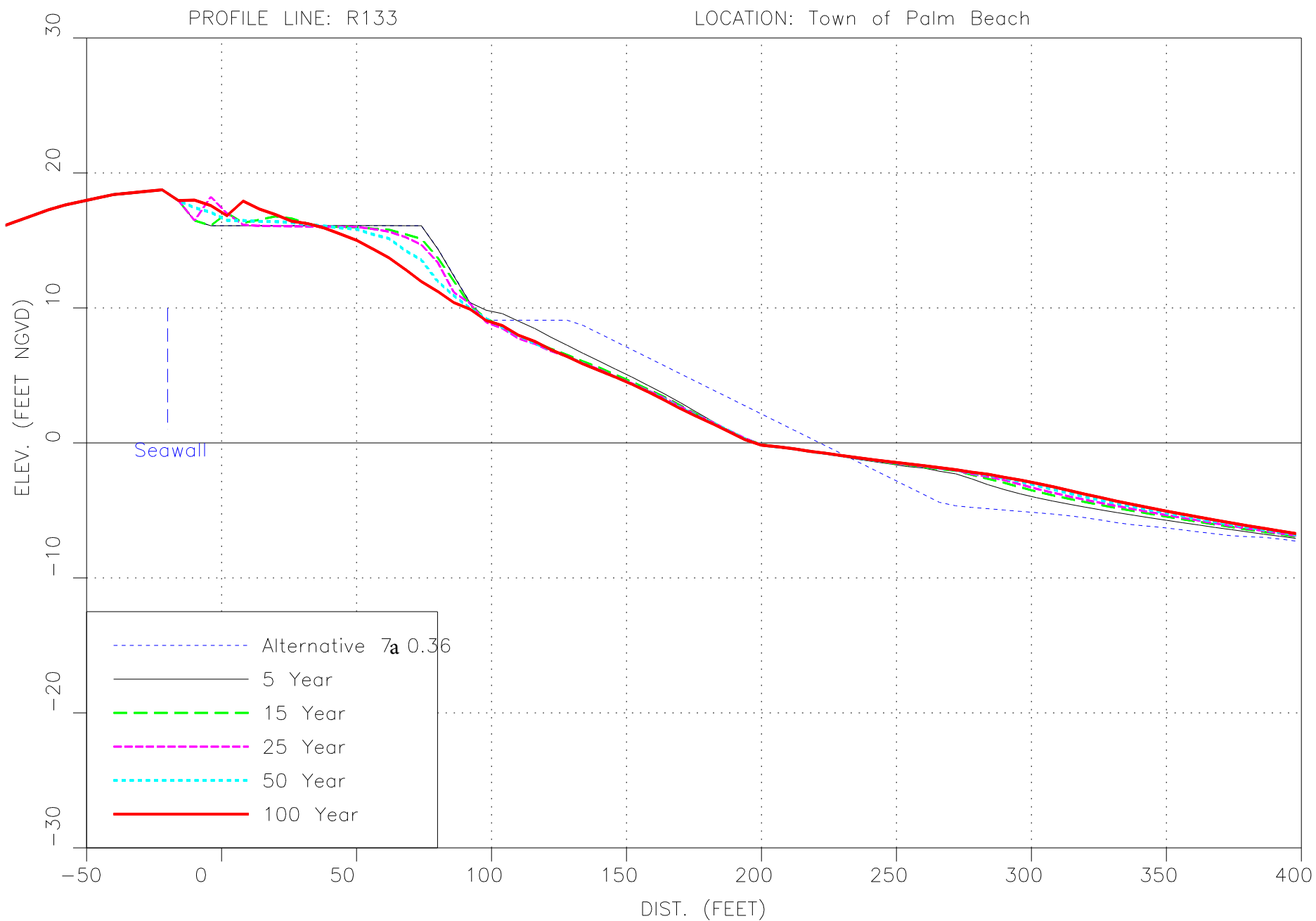
ATTACHMENT F-10
ALTERNATIVE 7a (SOS) GRAIN SIZE 0.36

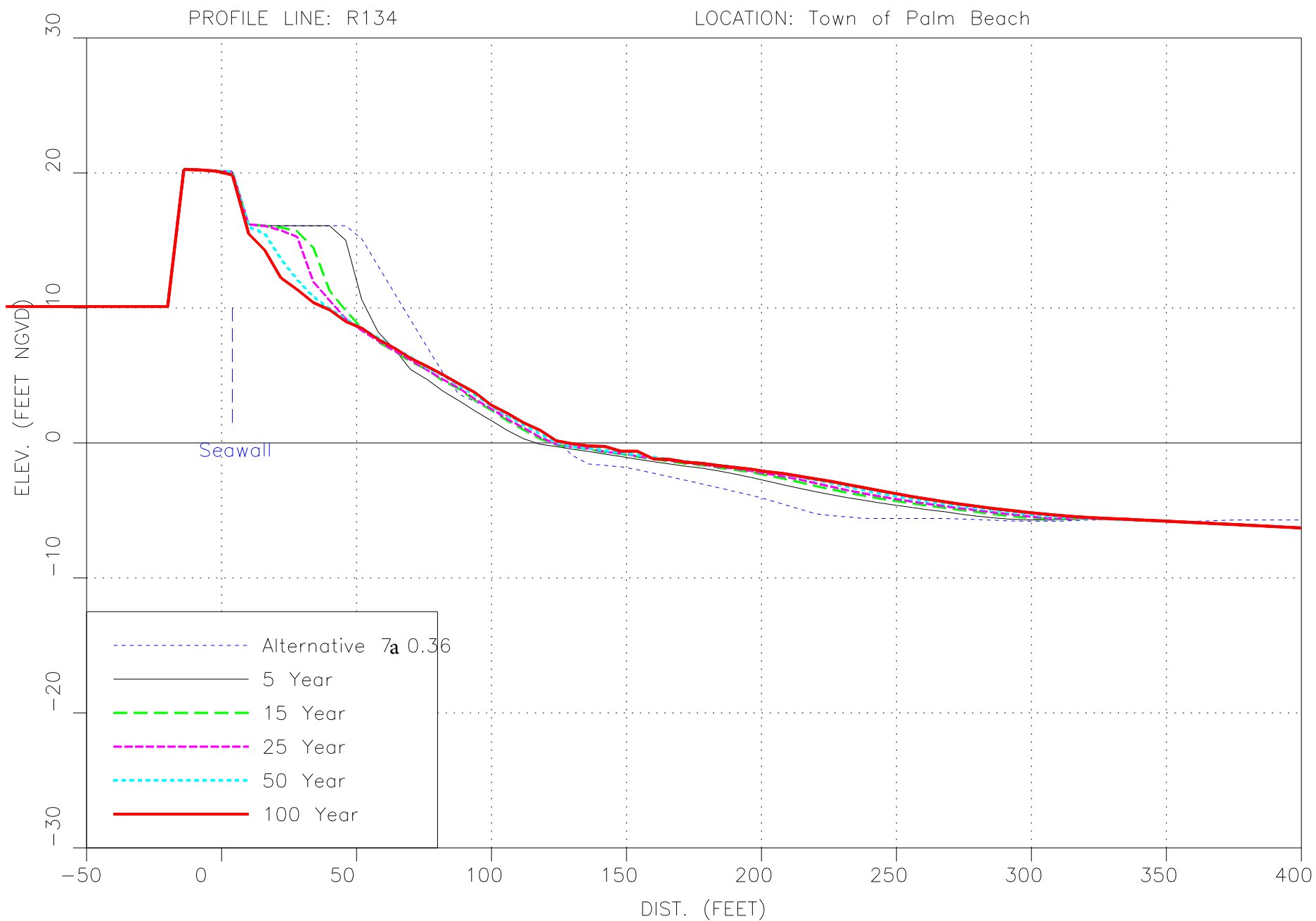






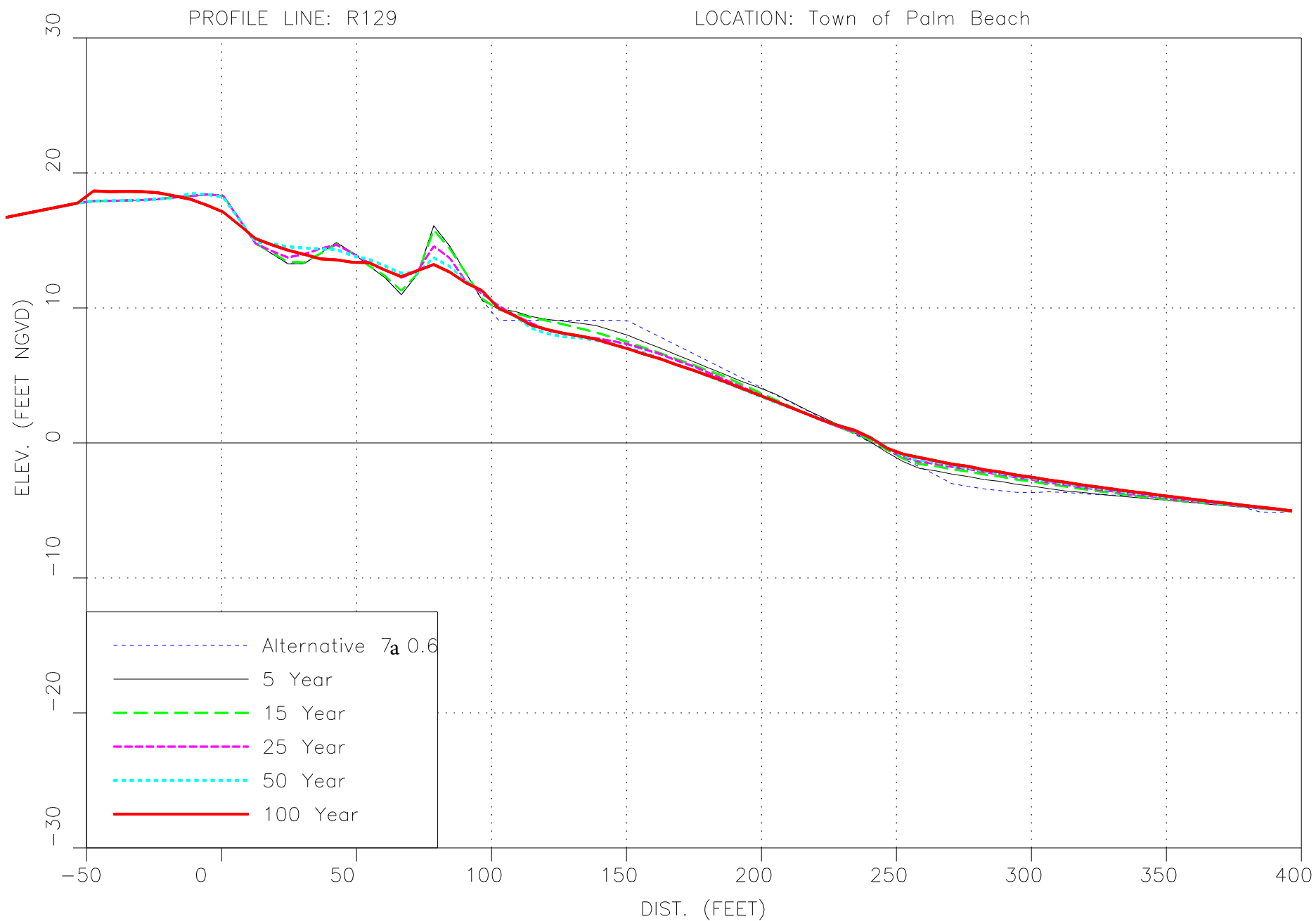


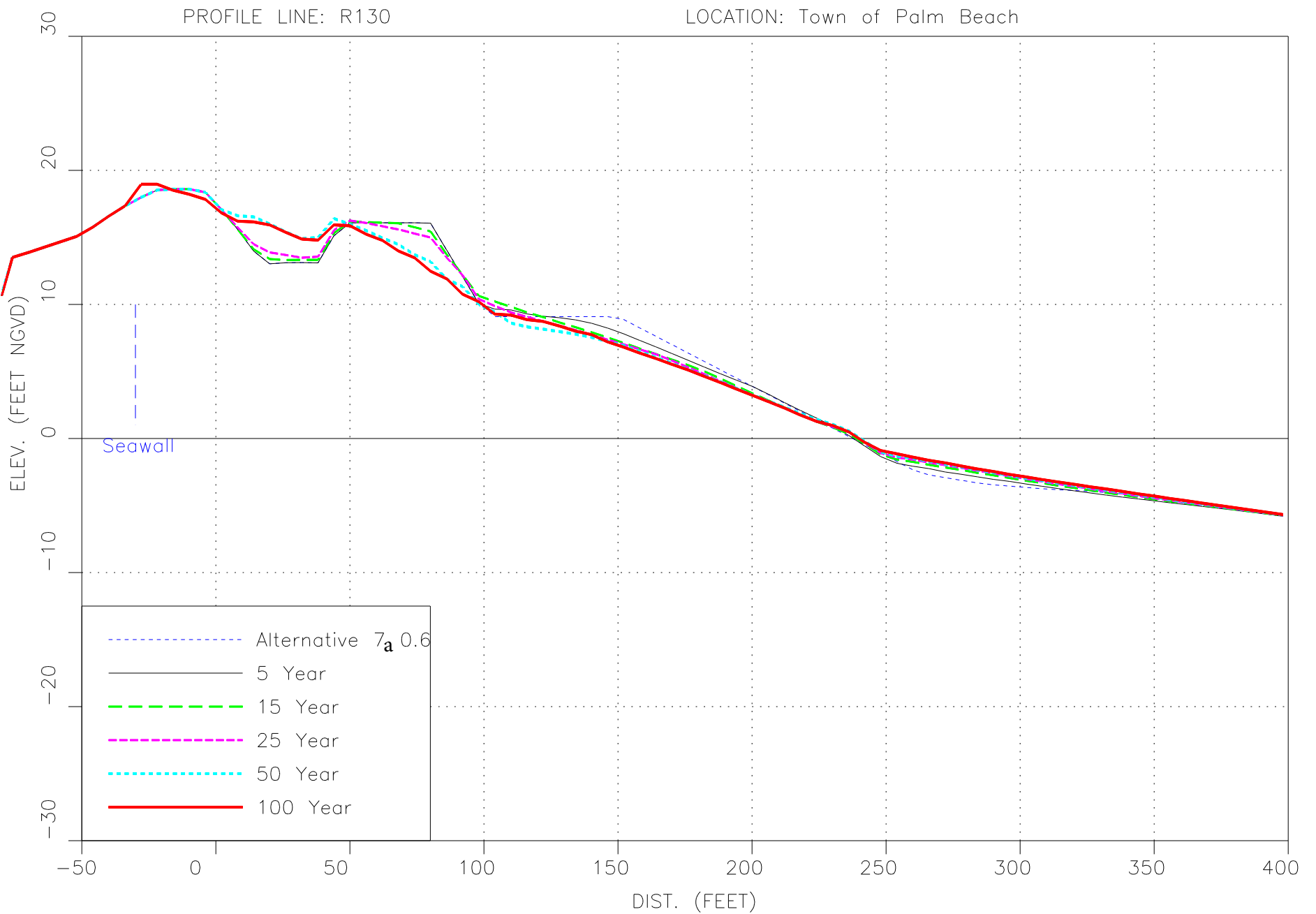


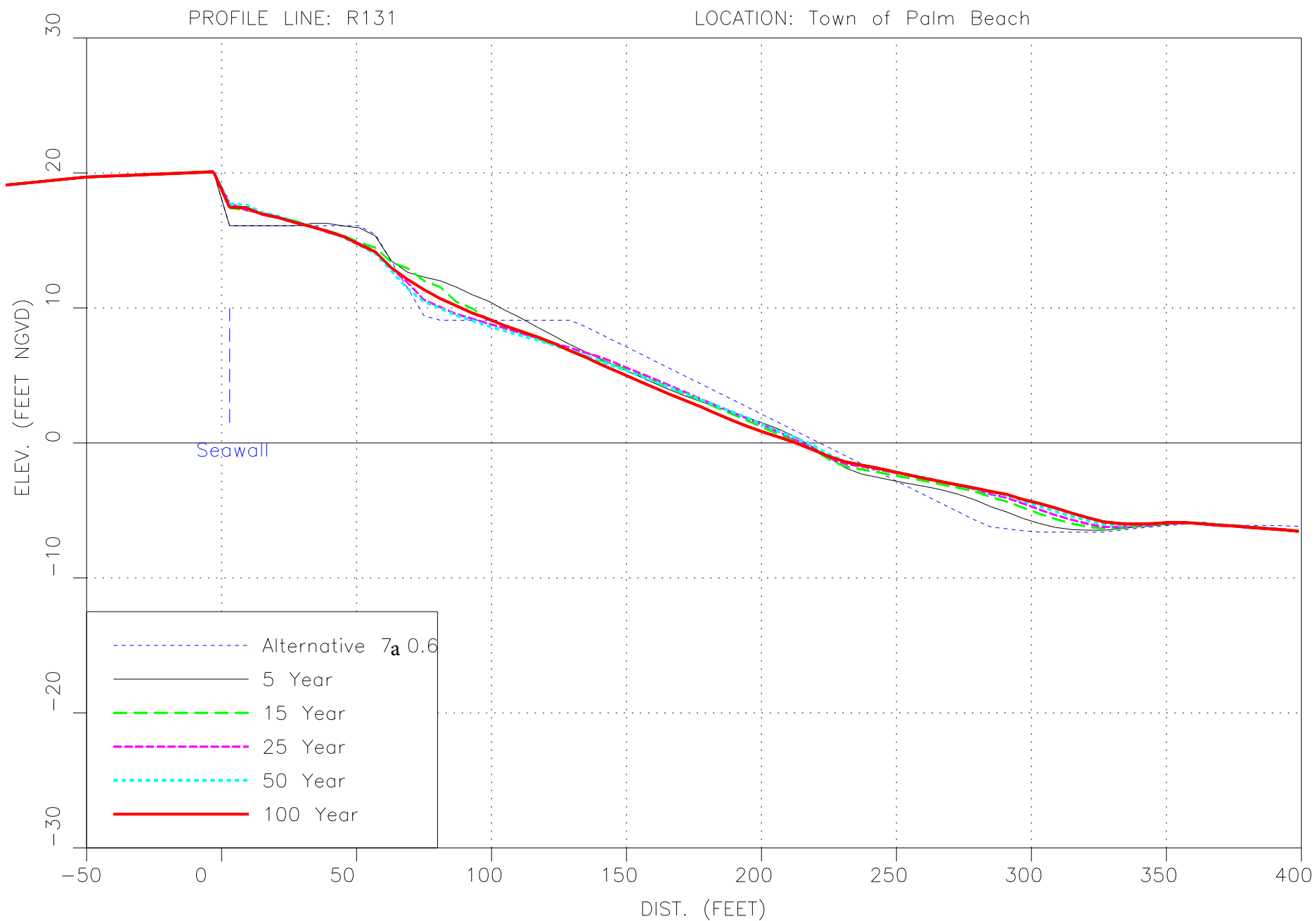


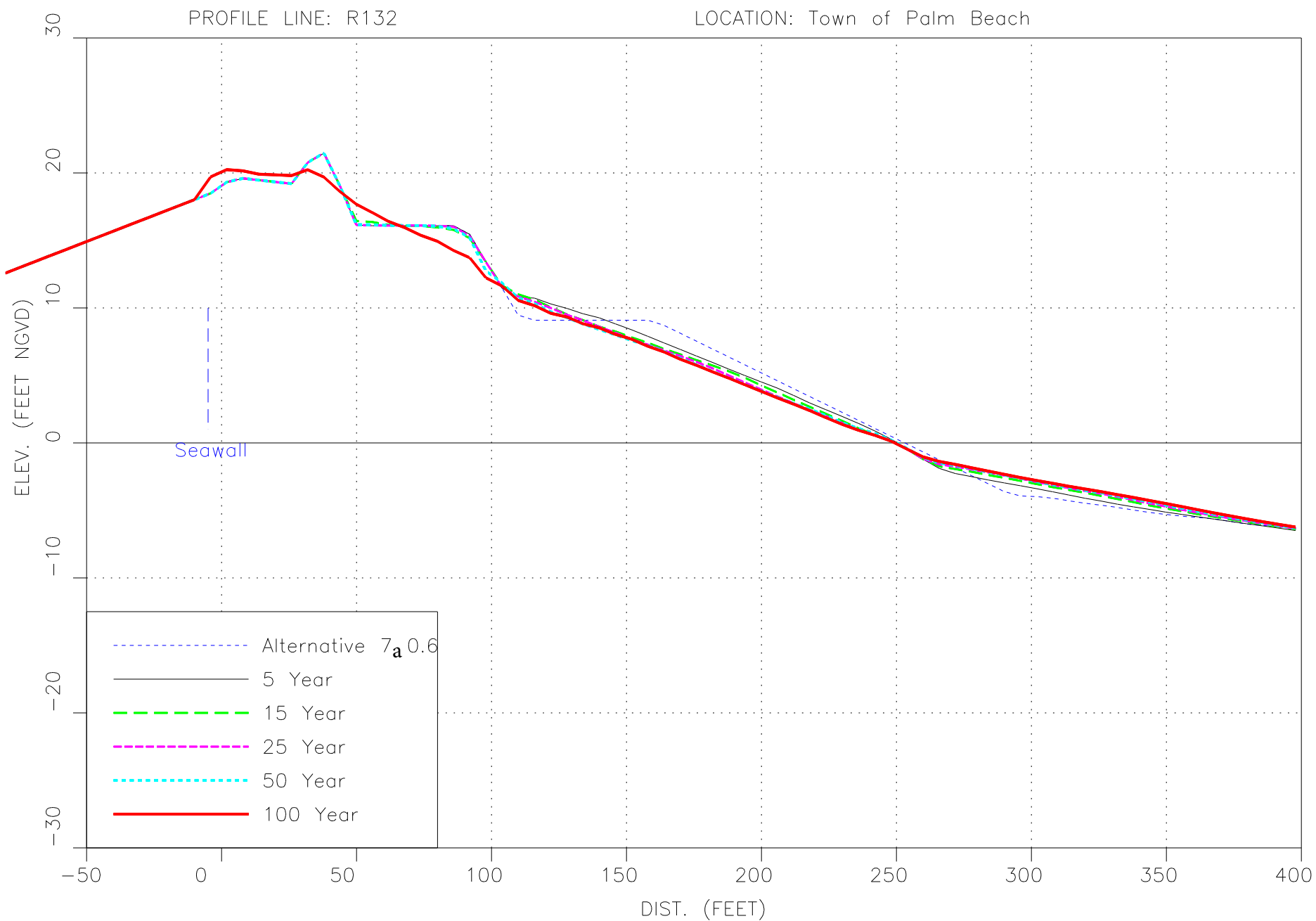
SUB-APPENDIX G-1
SBEACH ANALYSIS REPORT

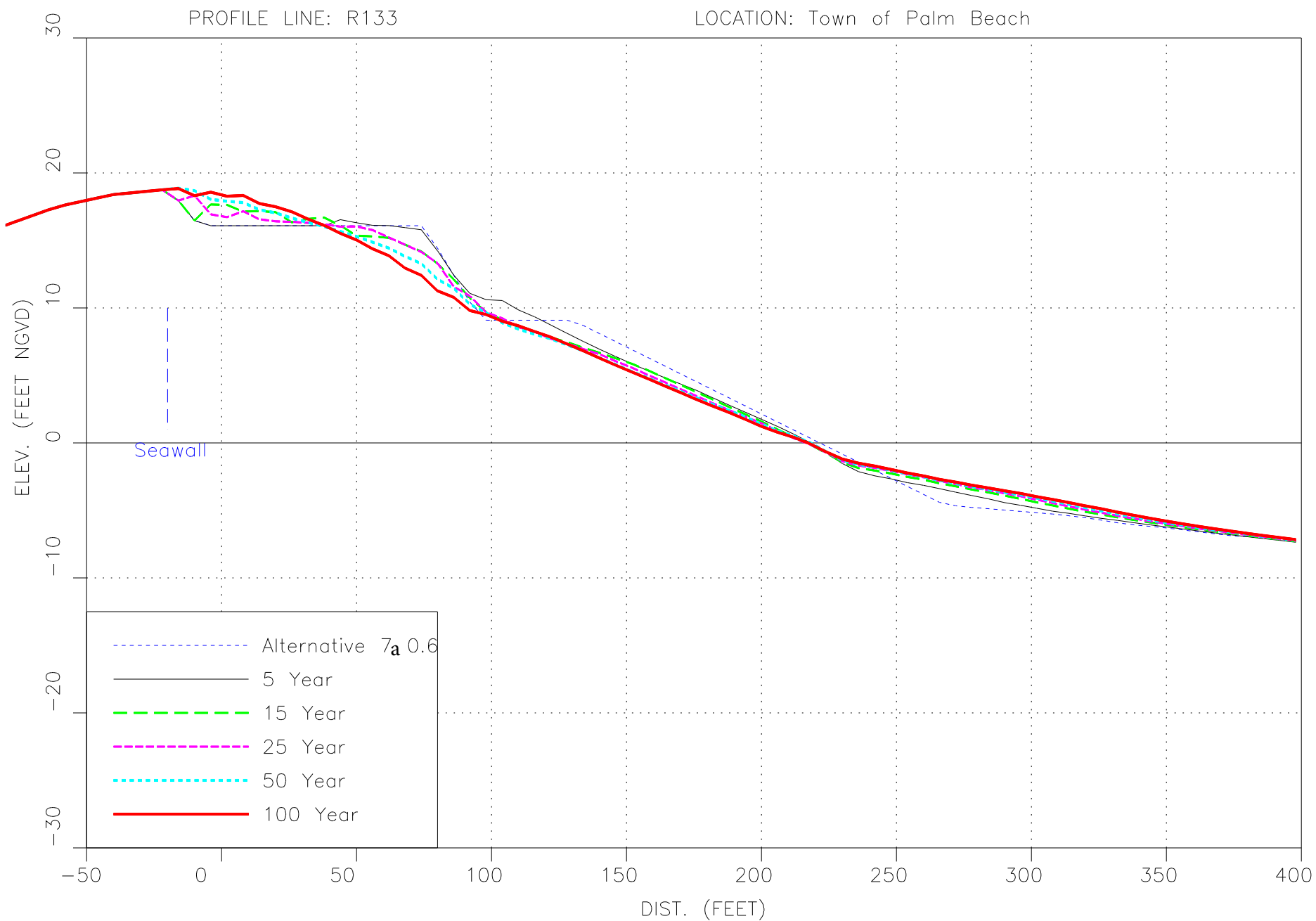
ATTACHMENT F-11
ALTERNATIVE 7a (SOS) GRAIN SIZE 0.6

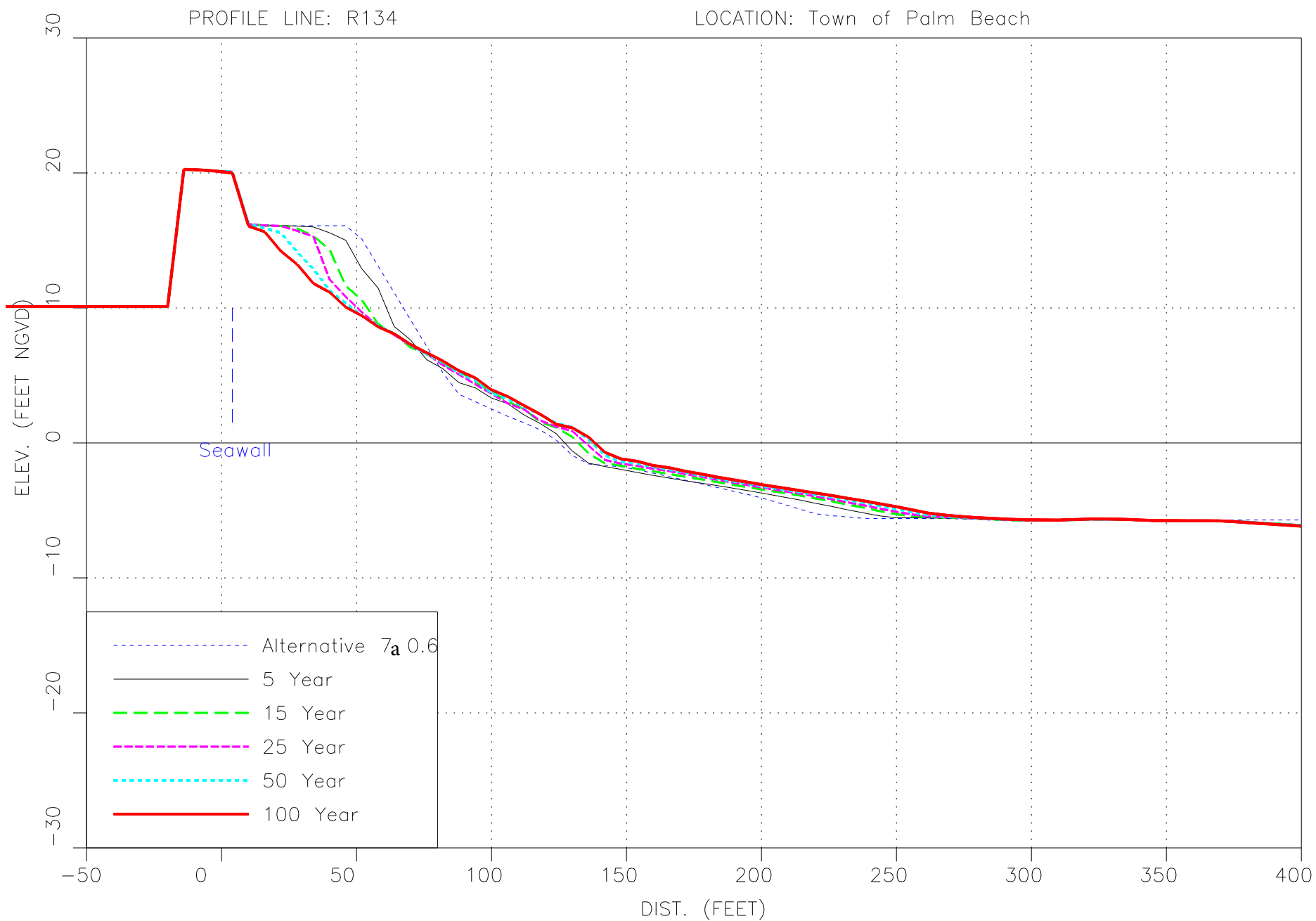










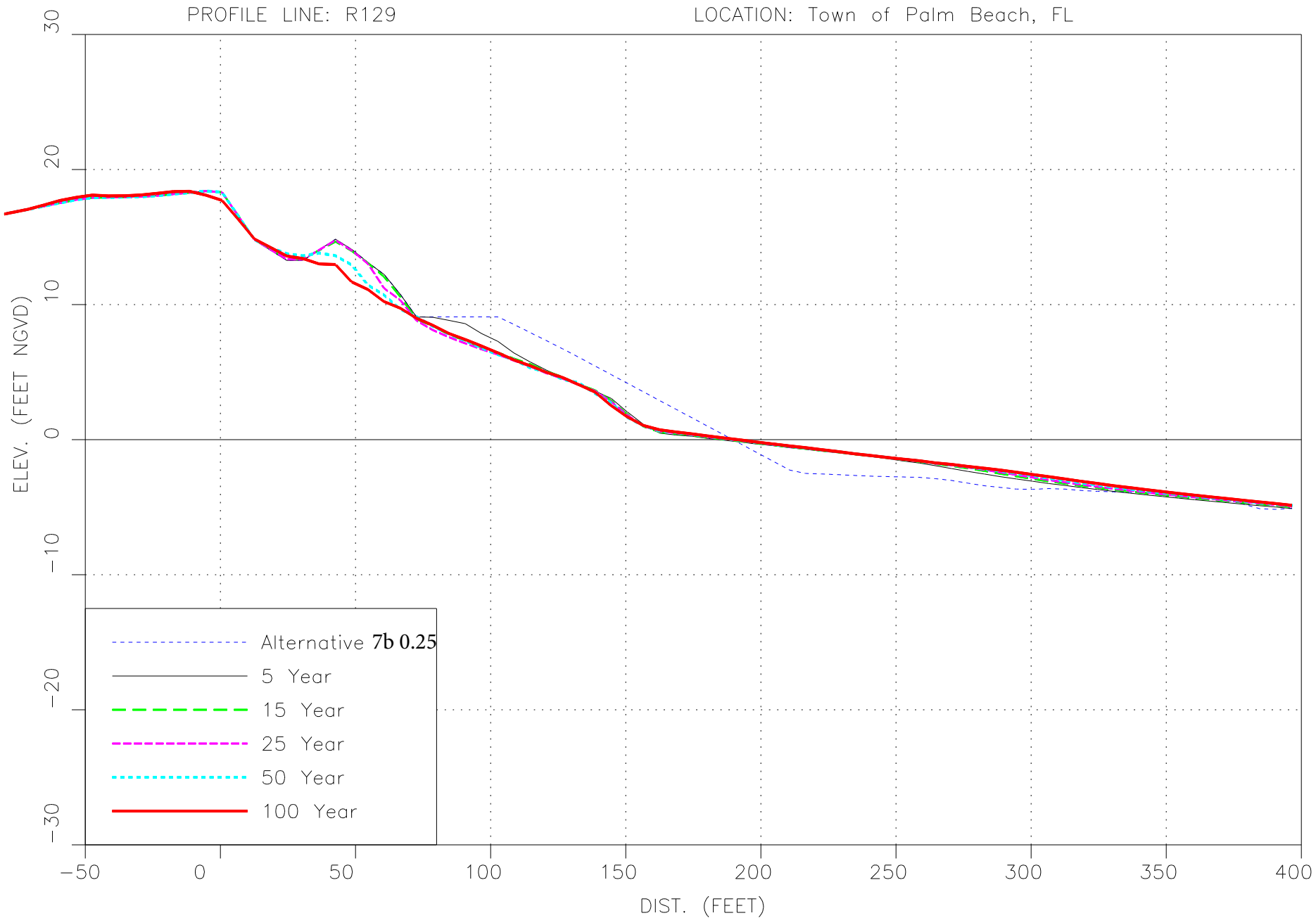


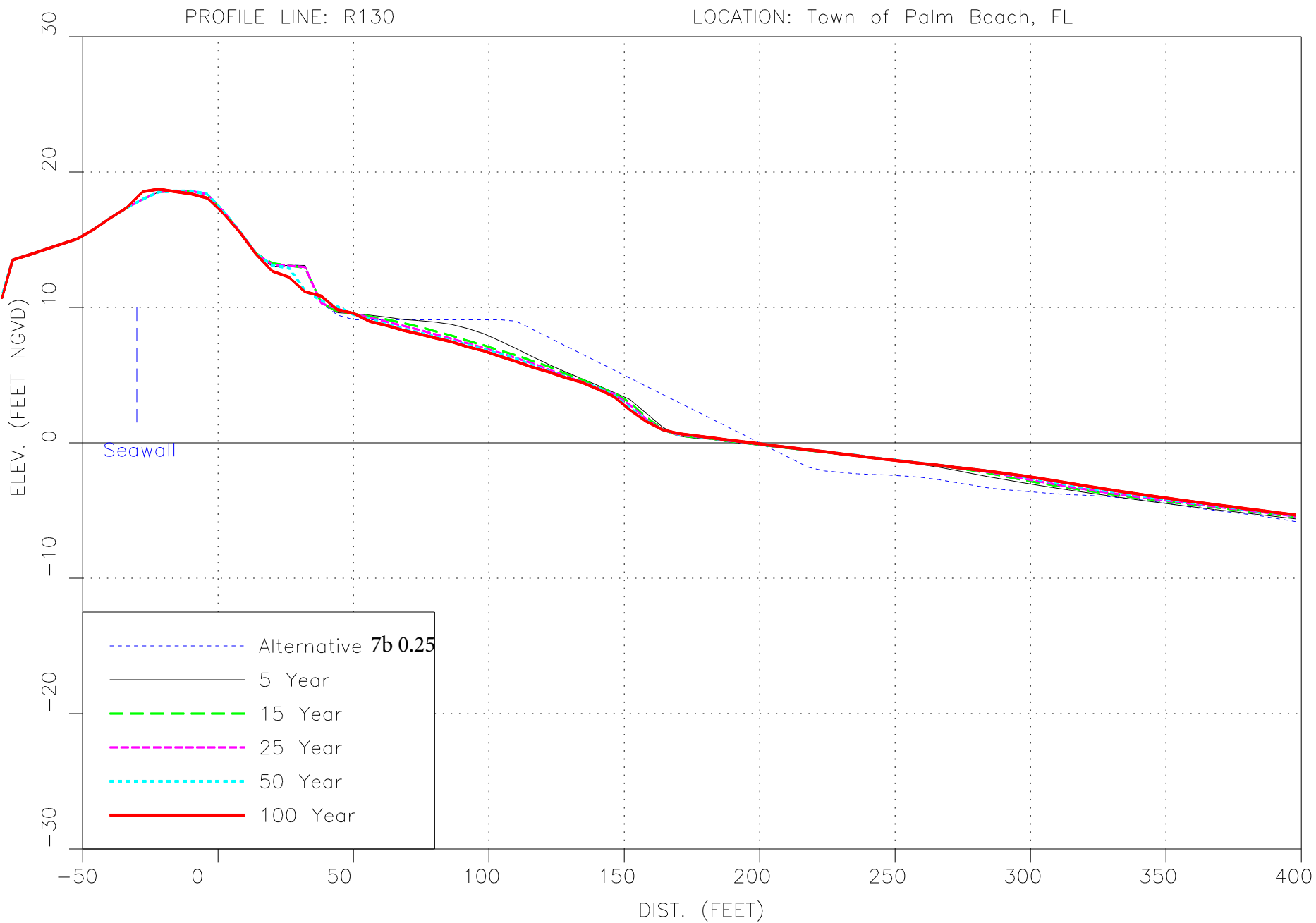
SUB-APPENDIX G-1
SBEACH ANALYSIS REPORT

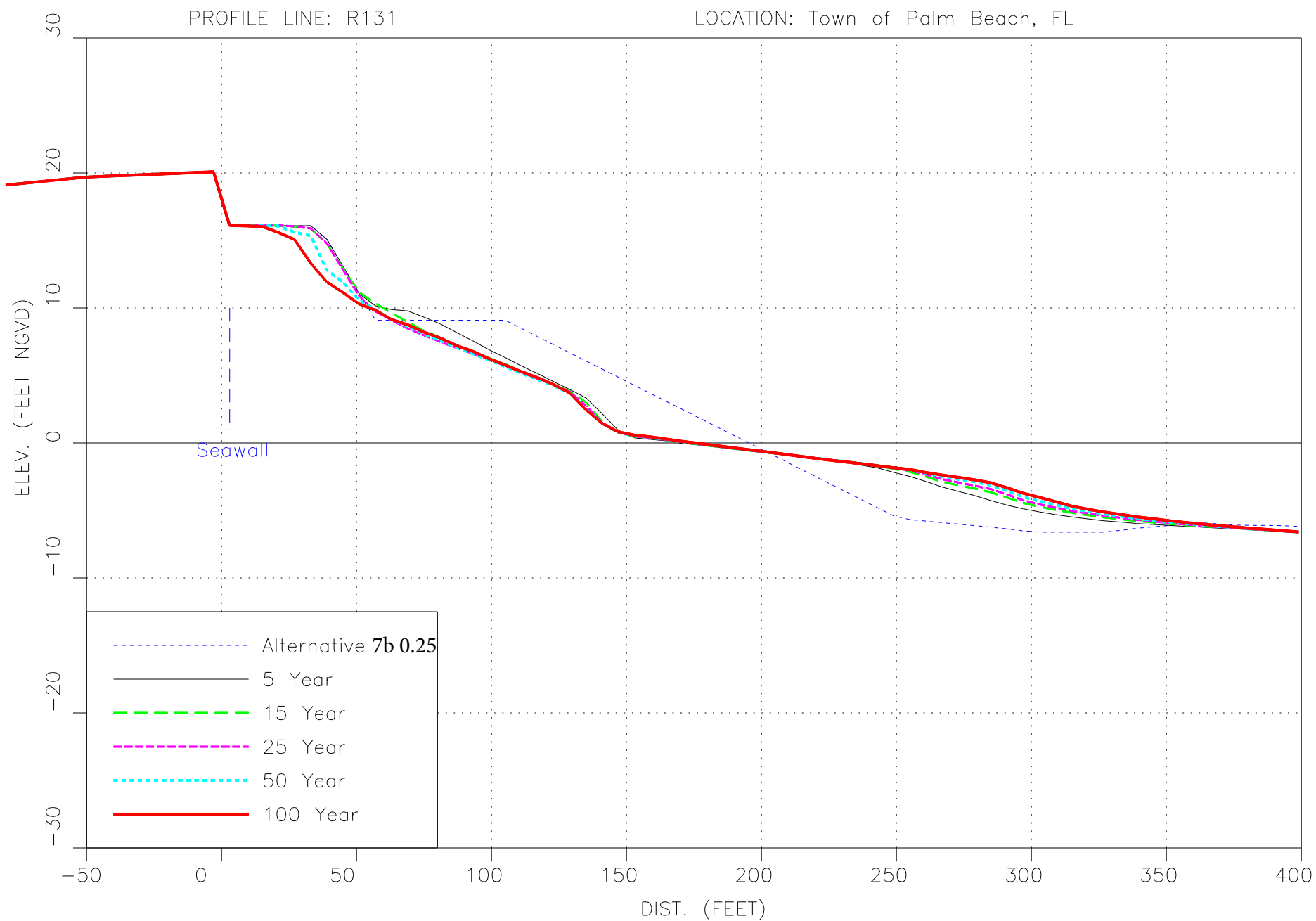
ATTACHMENT F-12
ALTERNATIVE 7b (SOS) GRAIN SIZE 0.25

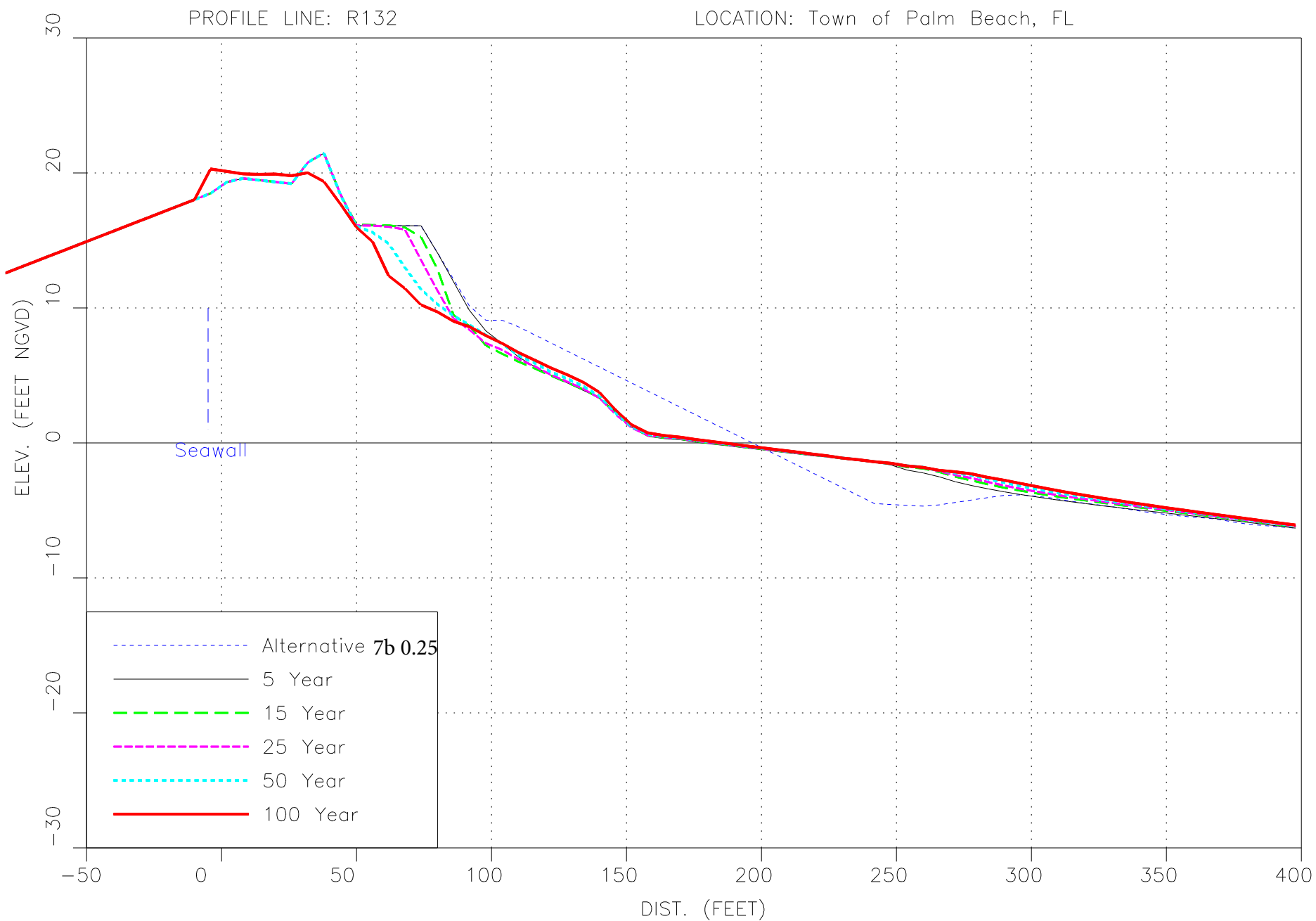
PROFILE LINE: R129

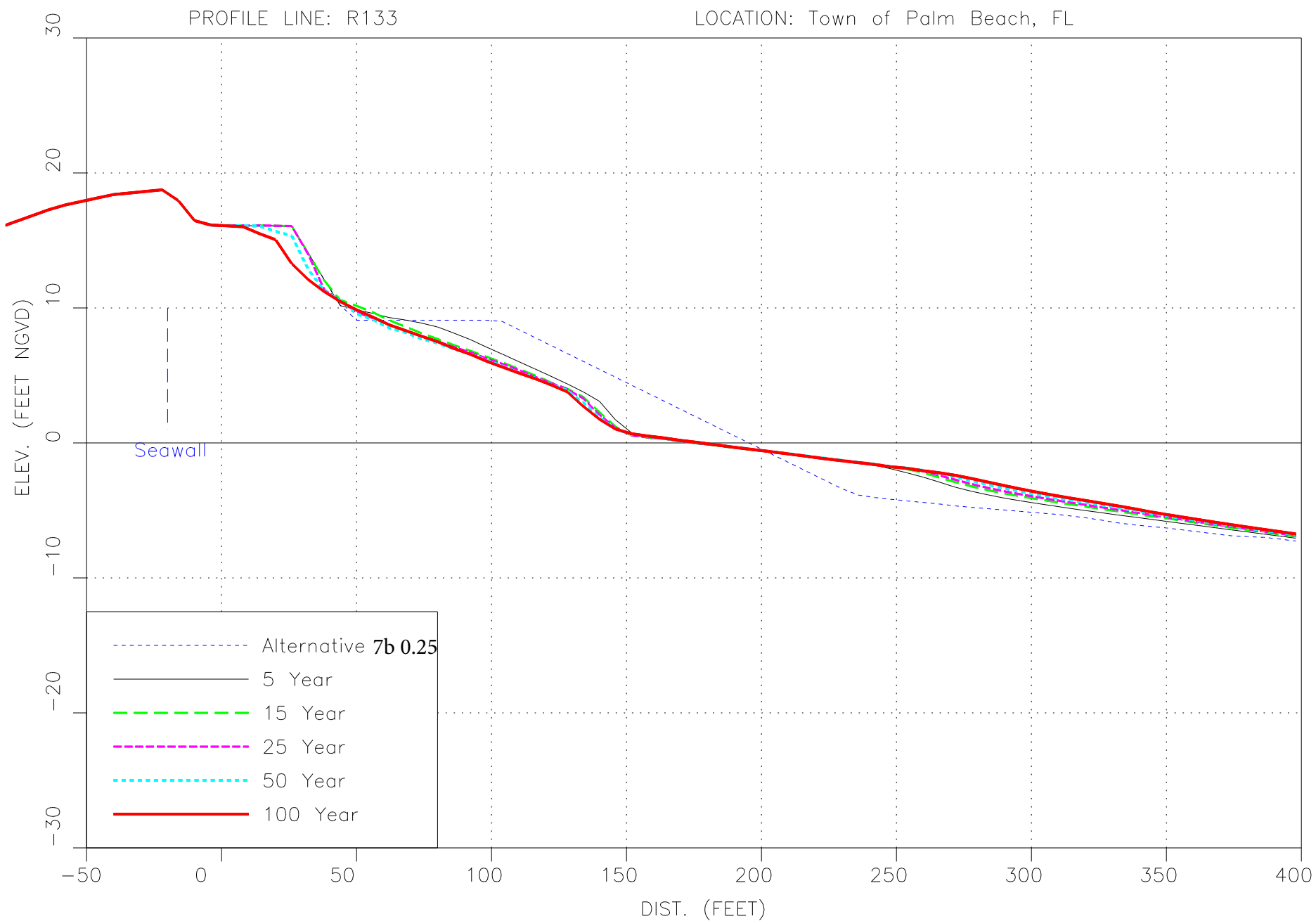
LOCATION: Town of Palm Beach, FL

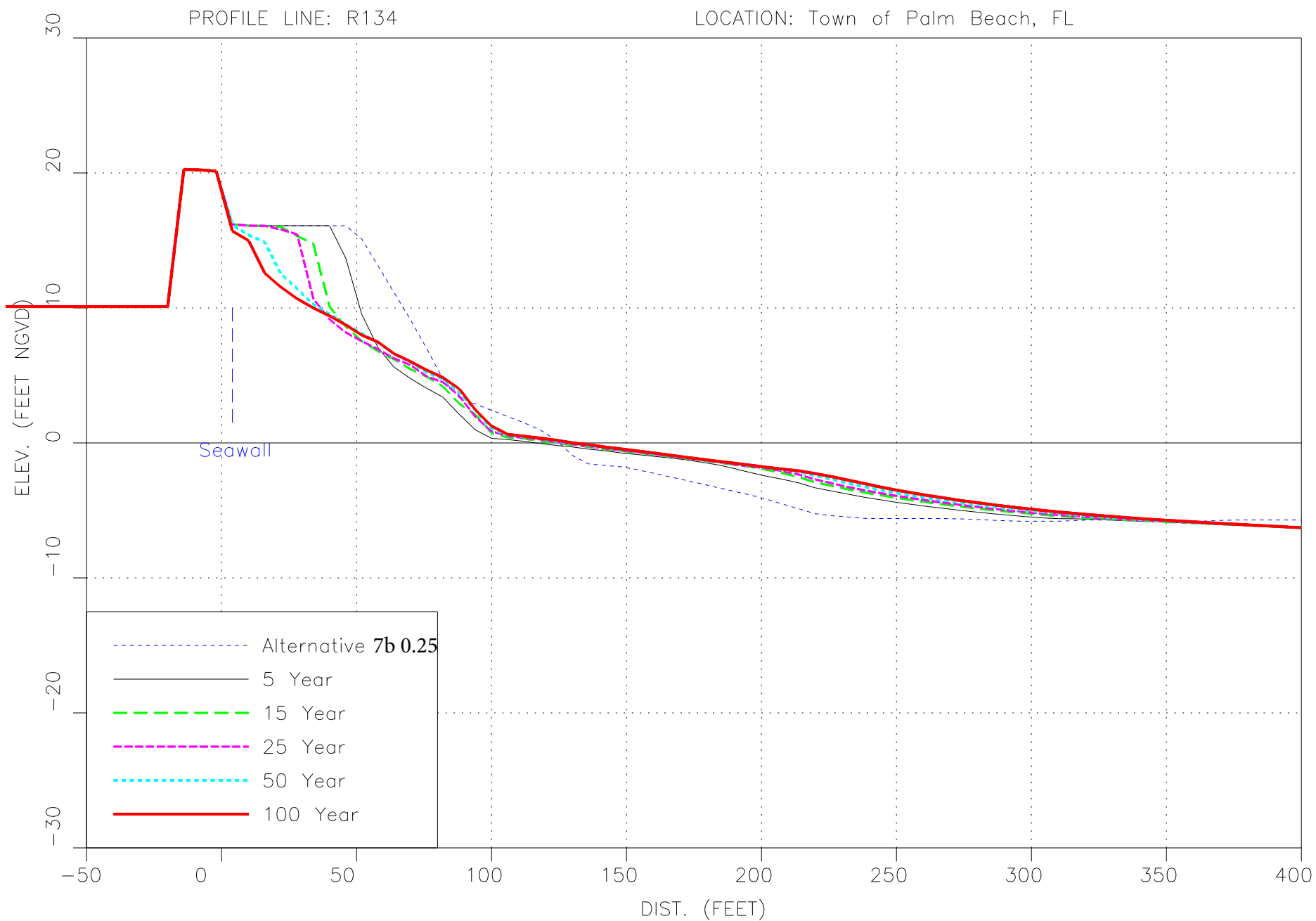










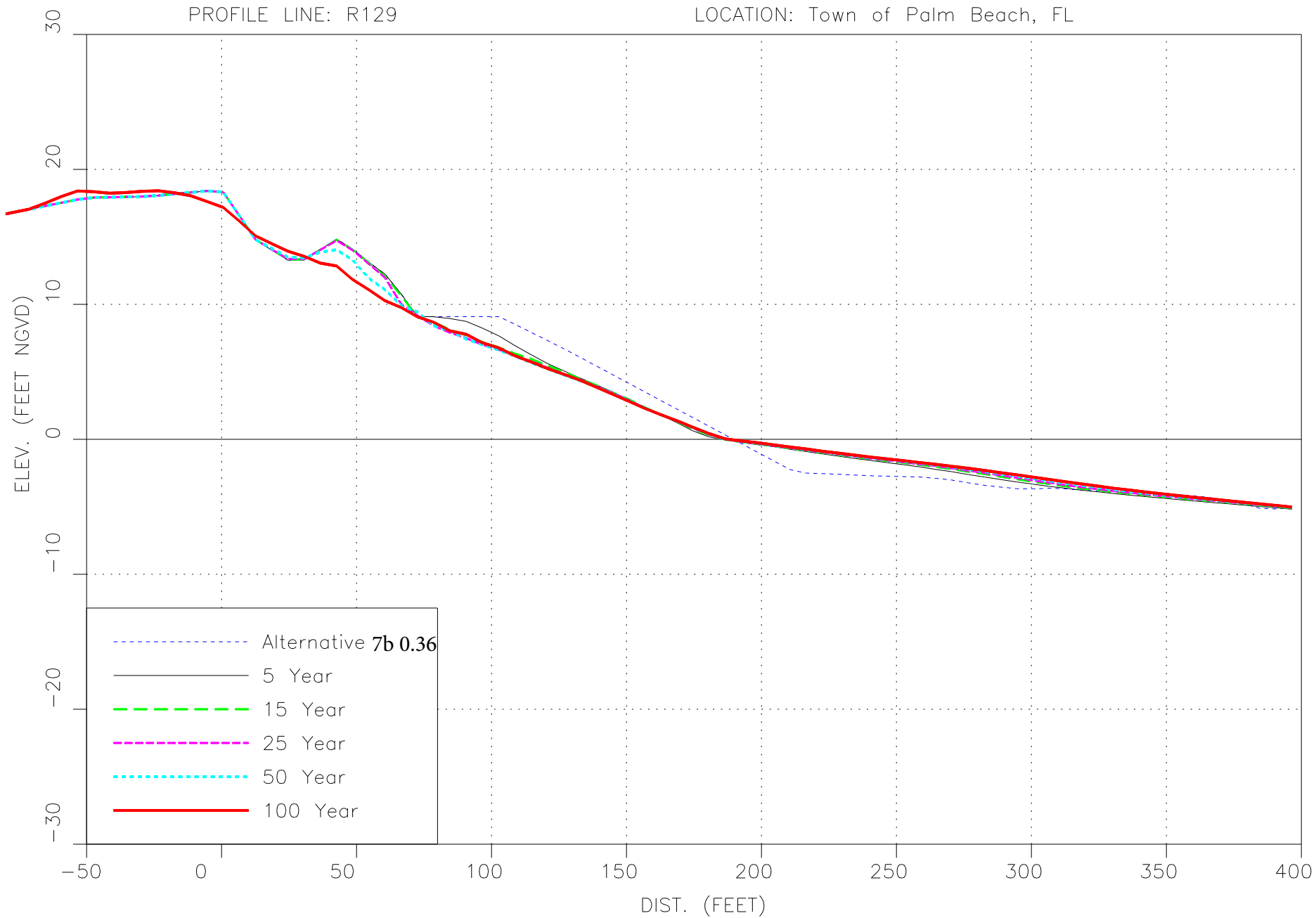


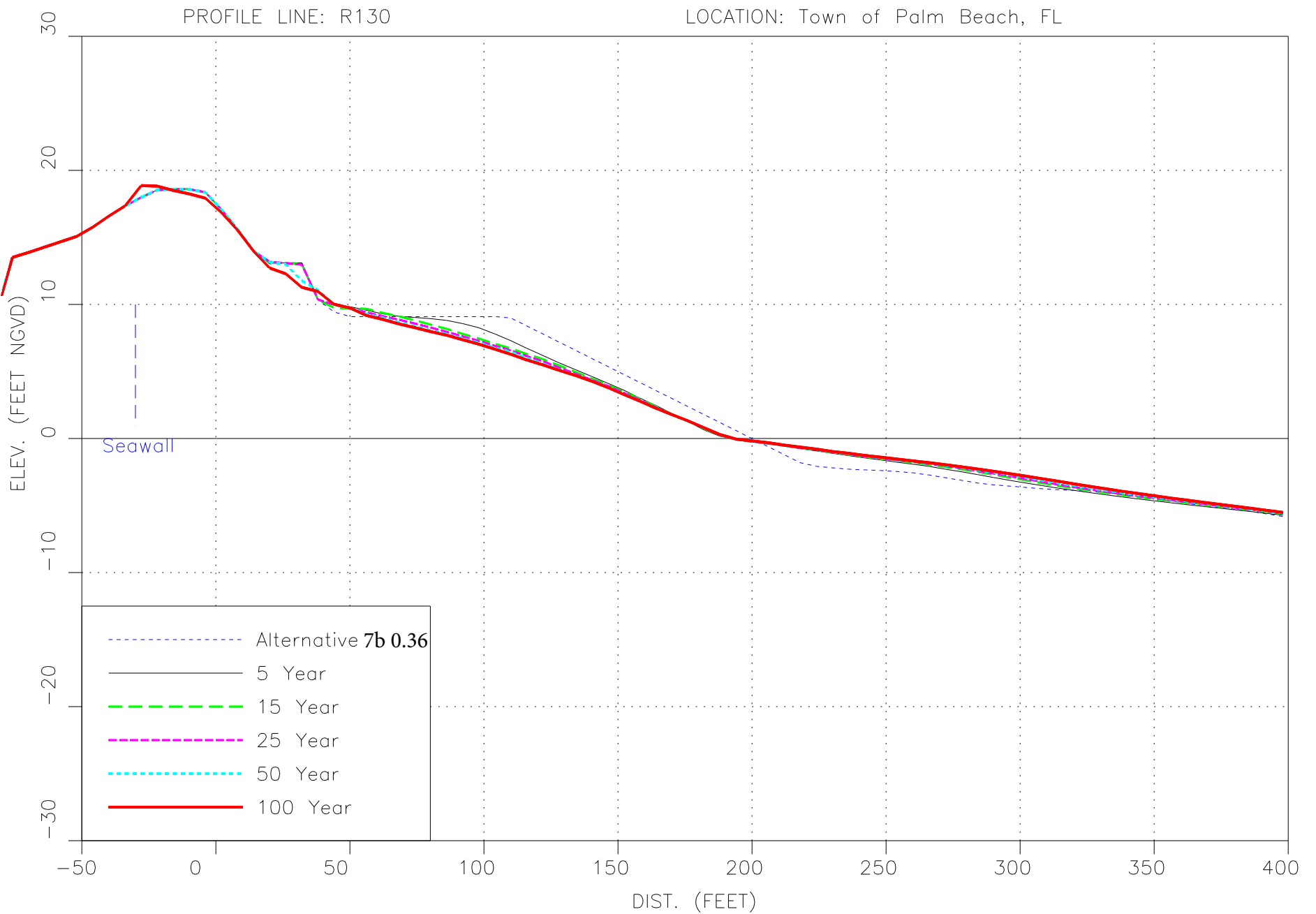
SUB-APPENDIX G-1
SBEACH ANALYSIS REPORT

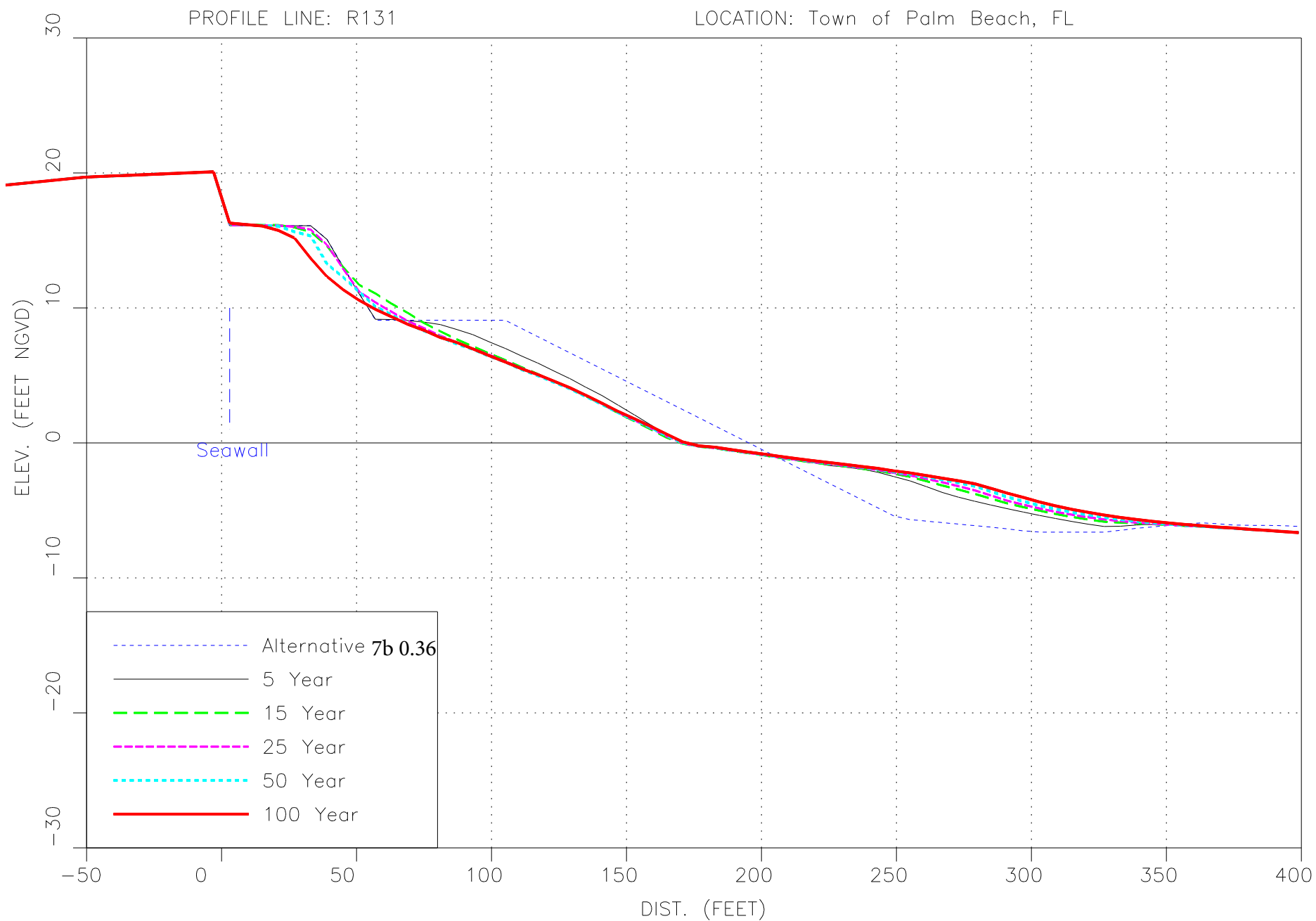
ATTACHMENT F-13
ALTERNATIVE 7b (SOS) GRAIN SIZE 0.36

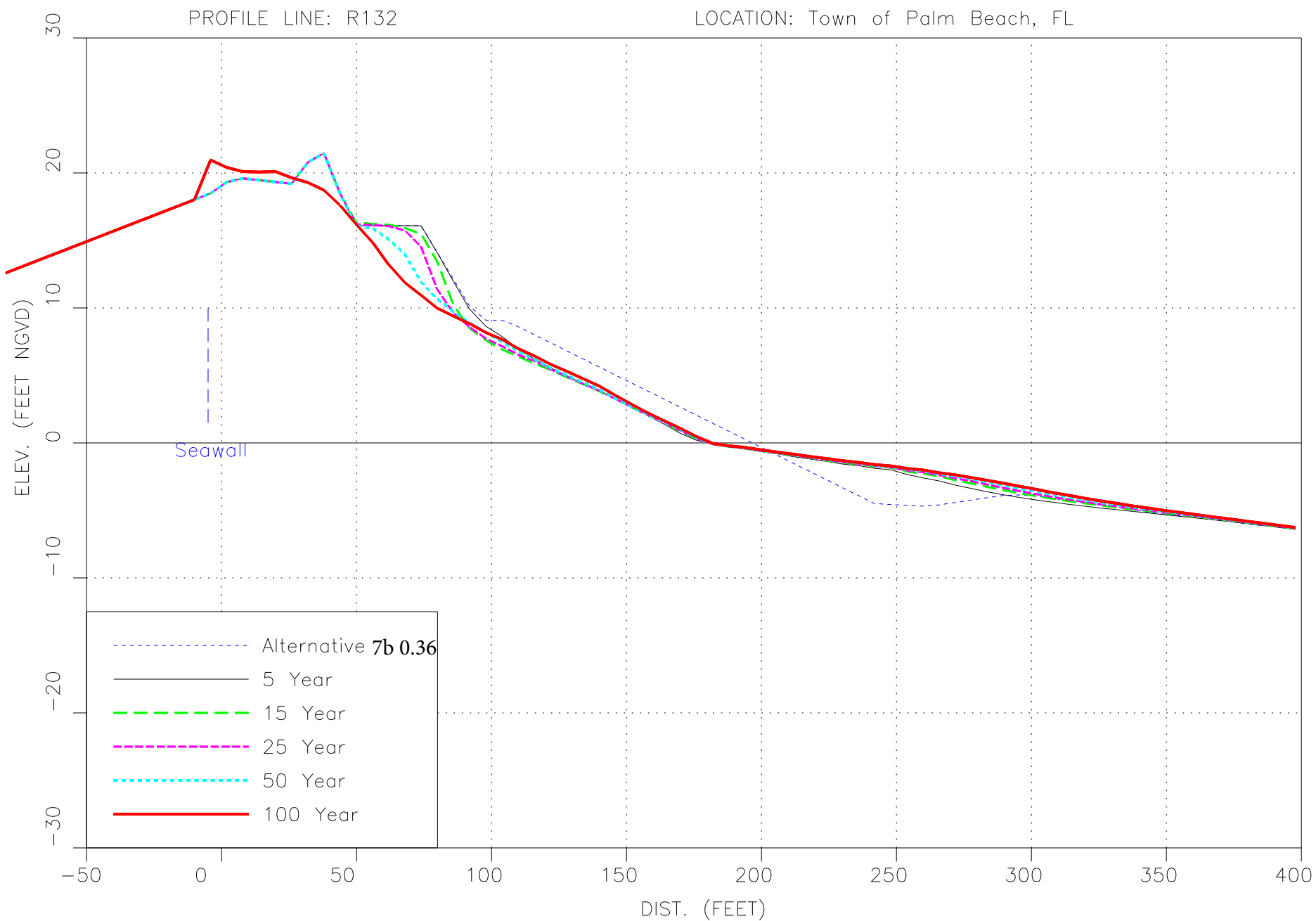
PROFILE LINE: R129

LOCATION: Town of Palm Beach, FL



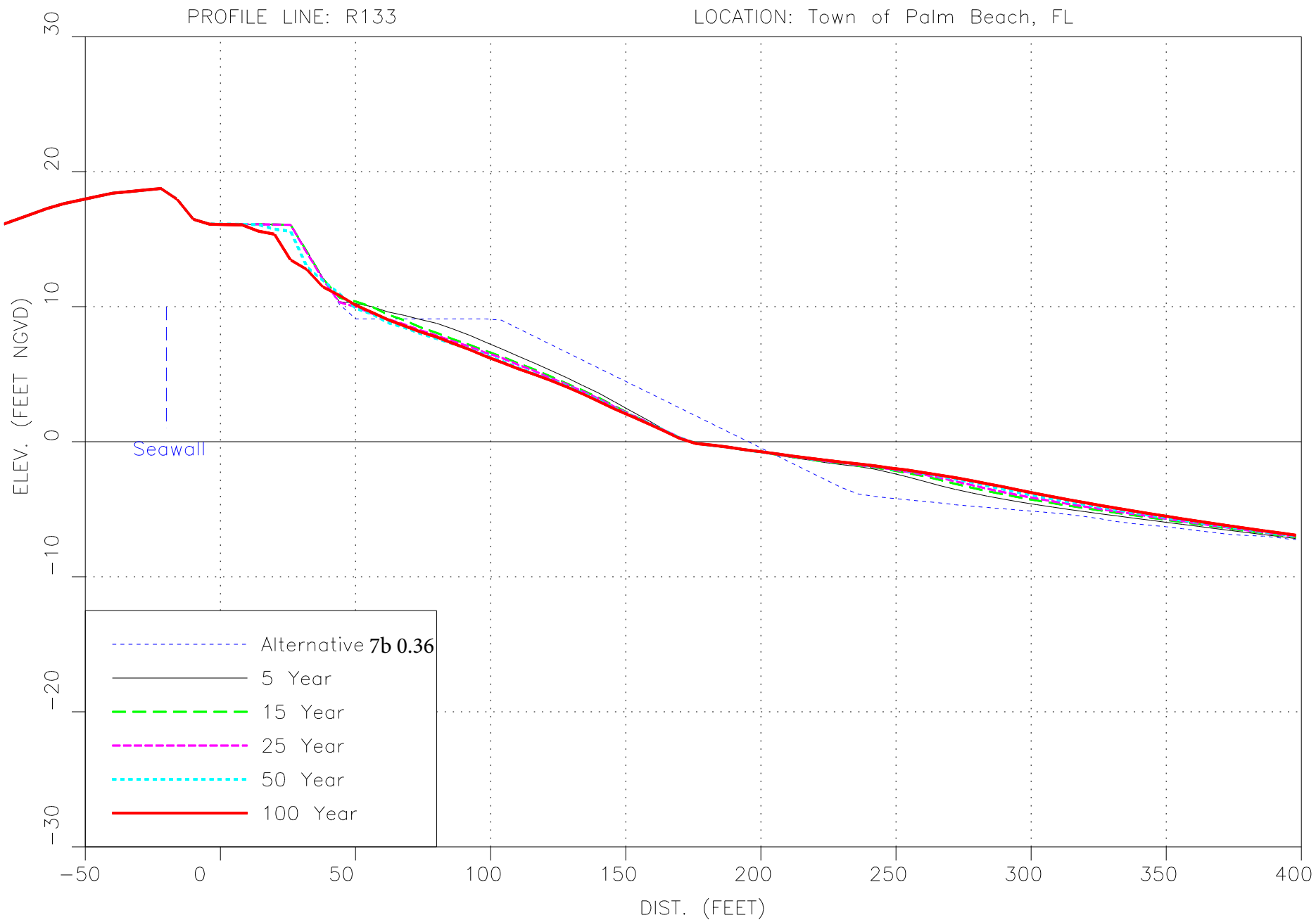


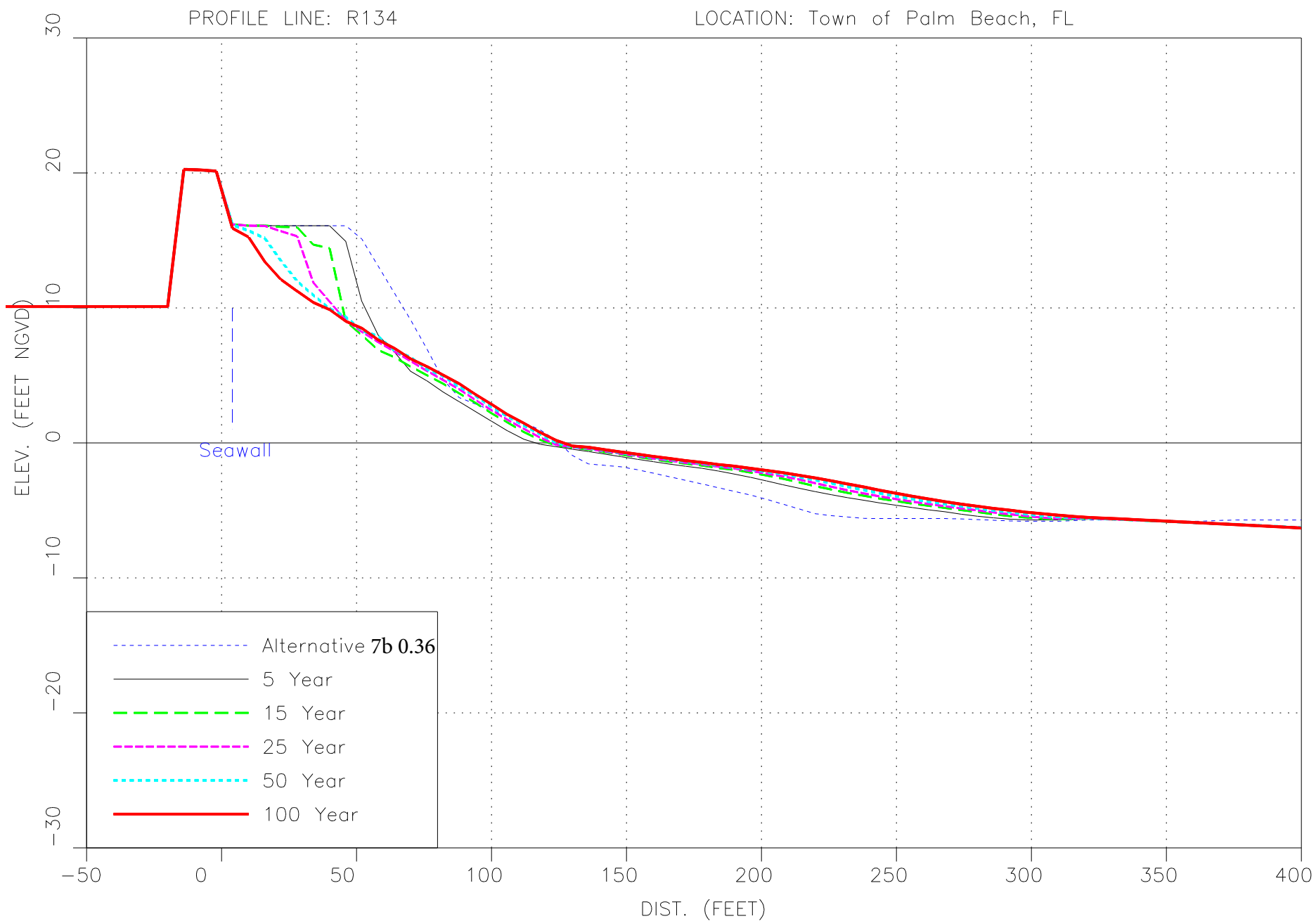




PROFILE LINE: R133

LOCATION: Town of Palm Beach, FL



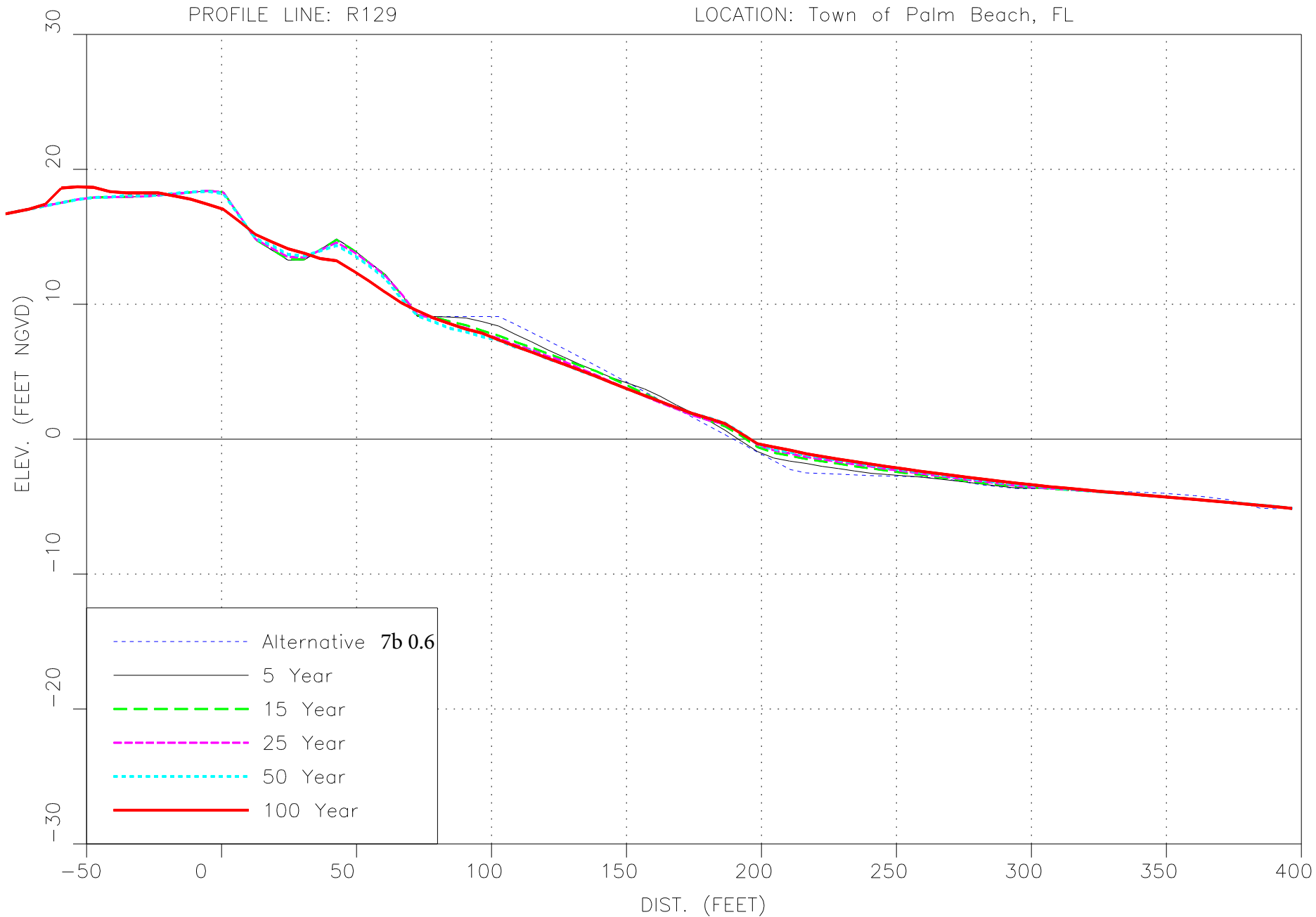


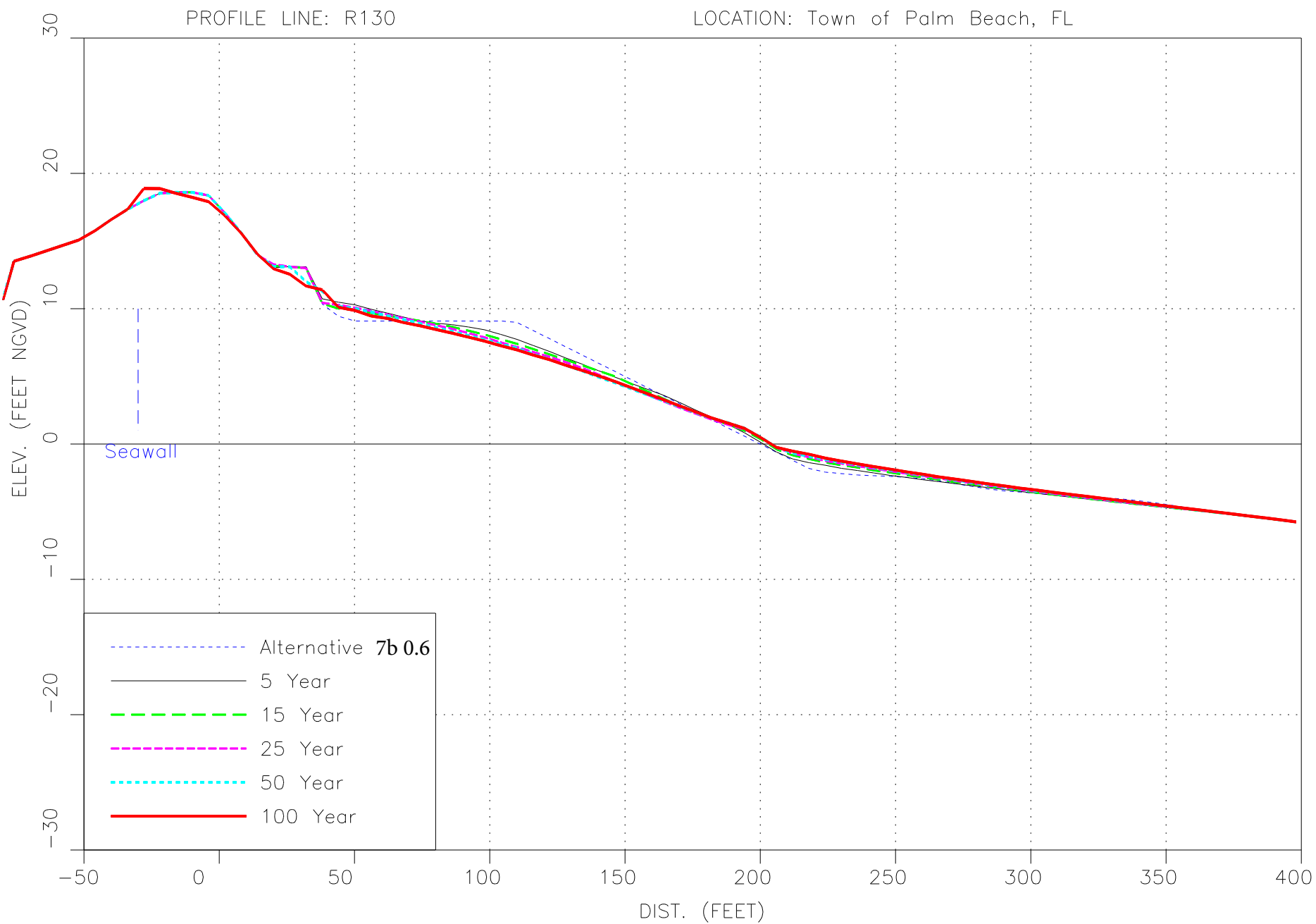
SUB-APPENDIX G-1
SBEACH ANALYSIS REPORT

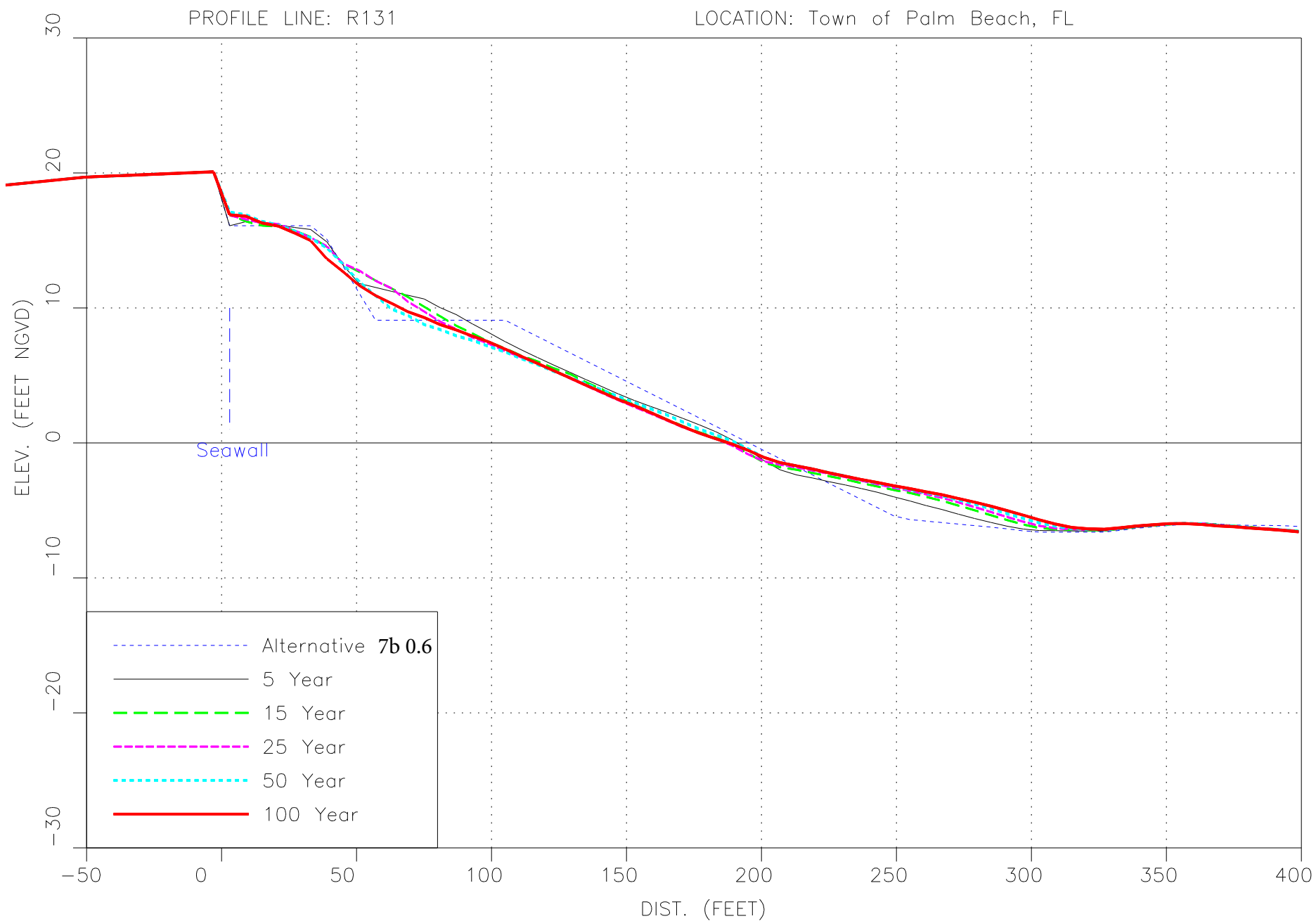
ATTACHMENT F-14
ALTERNATIVE 7a (SOS) GRAIN SIZE 0.6

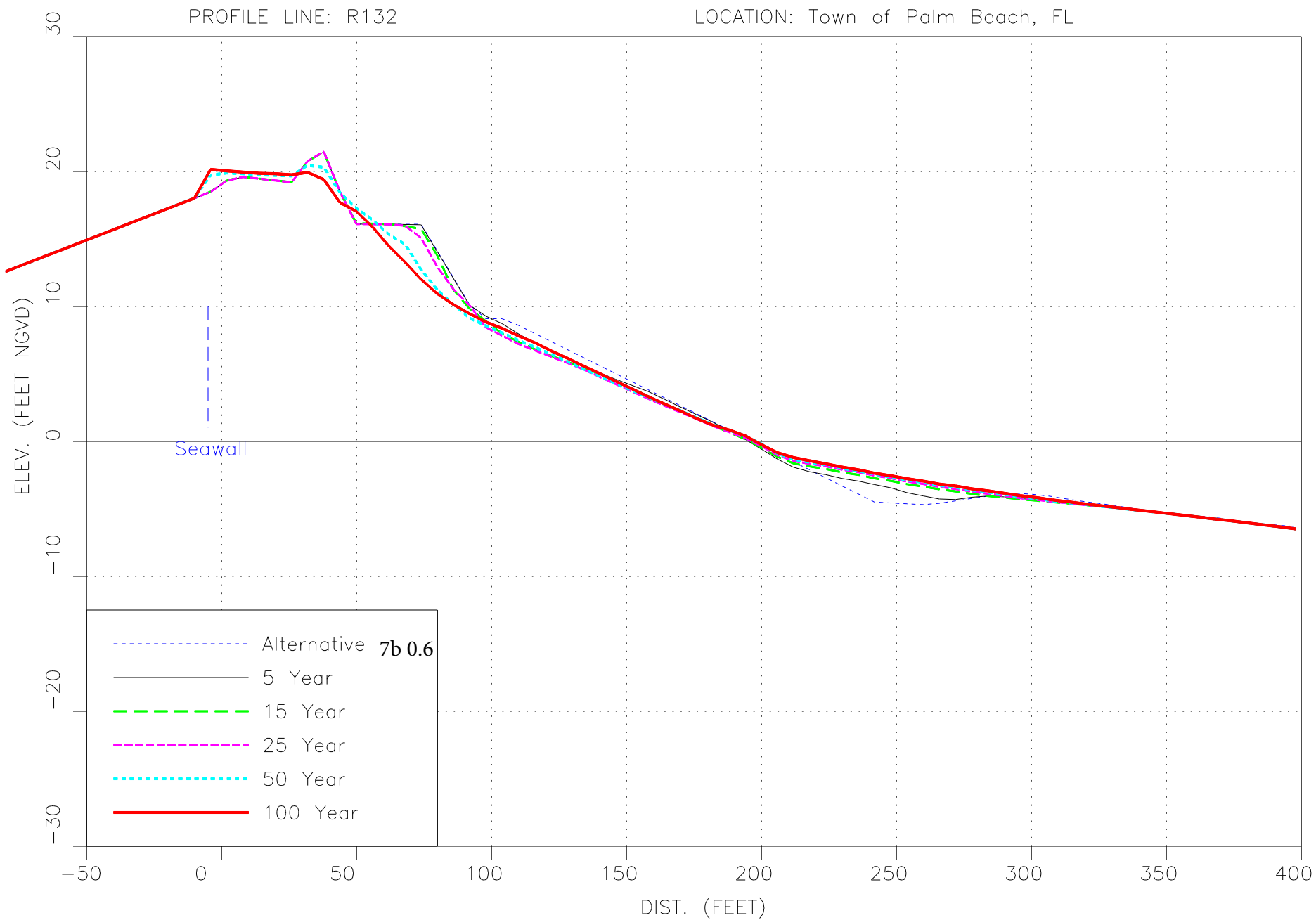
PROFILE LINE: R129

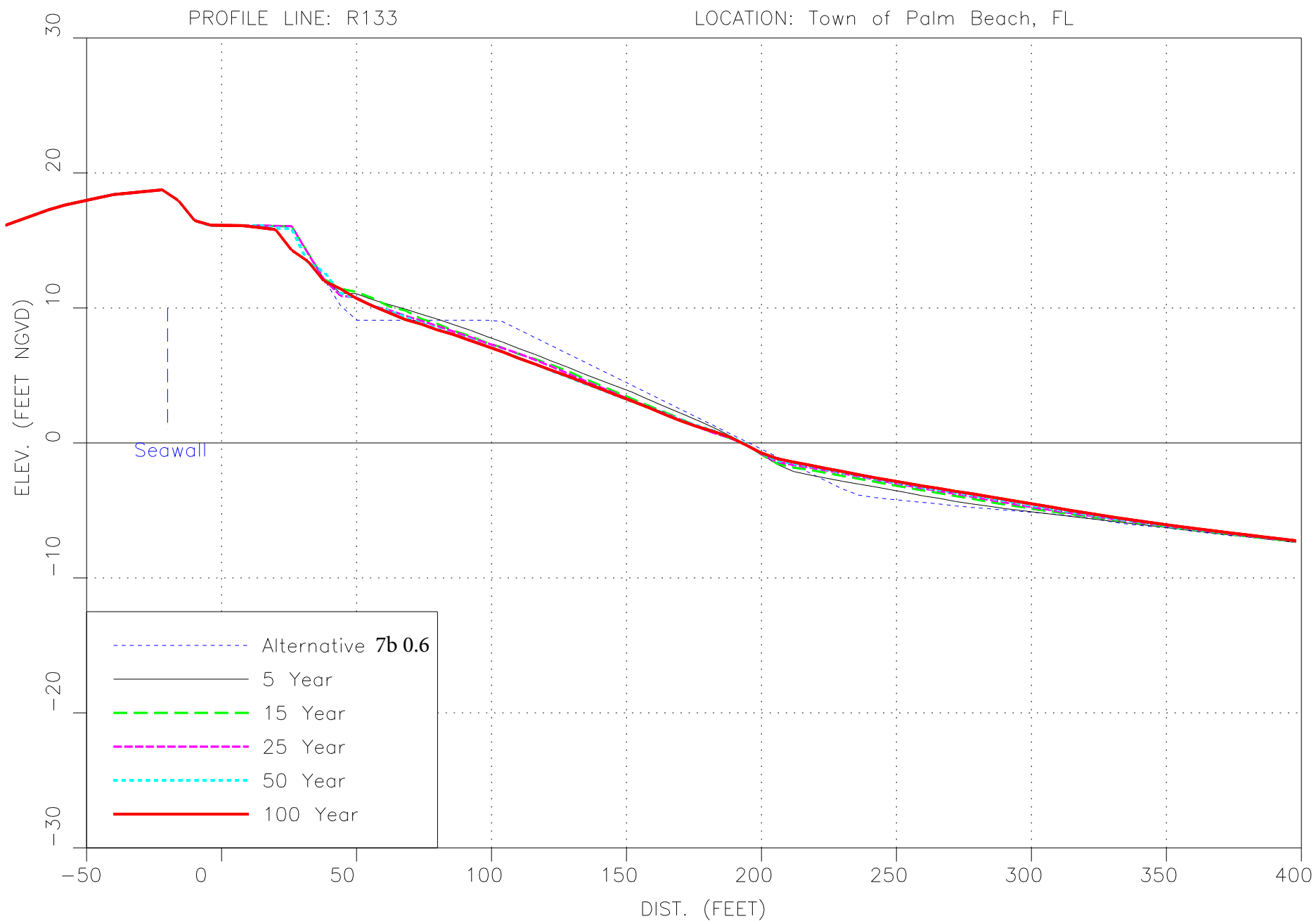
LOCATION: Town of Palm Beach, FL

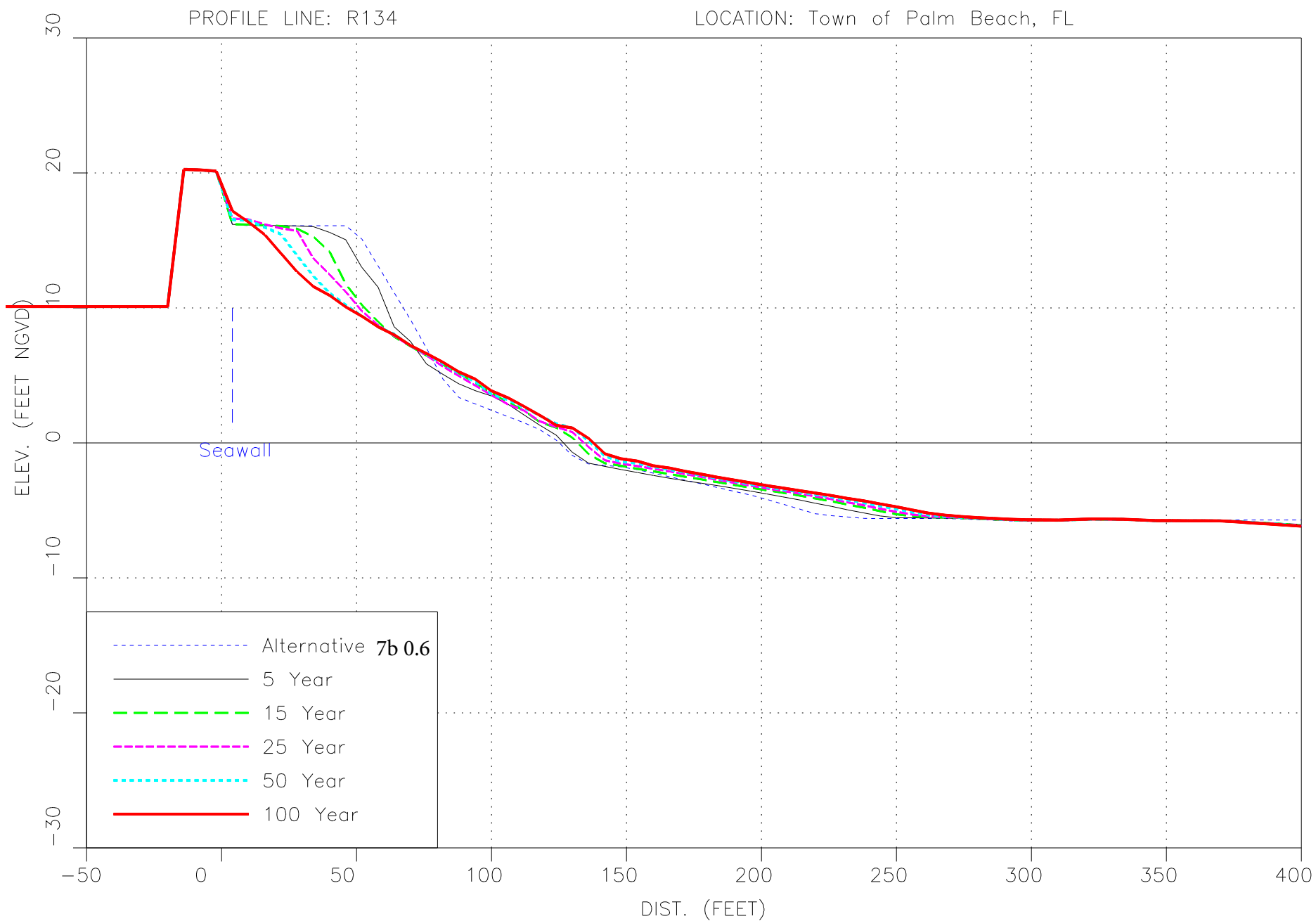












SUB-APPENDIX G-1
SBEACH ANALYSIS REPORT

ATTACHMENT F-15
FUTURE SCENARIO (WITHOUT PROJECT CONDITIONS)

